



~ Geotechnical Evaluations ~ Construction Materials Testing ~ Geosciences ~ Infrastructure Management Services ~

**SOILS EXPLORATIONS AND GEOTECHNICAL
ENGINEERING STUDIES FOR THE PROPOSED
CITY OF MOBILE
CIVIC CENTER ARENA
MOBILE, ALABAMA**

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Geotechnical Engineering-Testing, Inc.

PROFESSIONAL ENGINEERS

Geotechnical Evaluations - Geosciences - Construction Materials - Pavement Management

August 16, 2024

City of Mobile c/o
Mr. Sam Matheny
Volkert Program Management
11 North Water Street, Suite 18290
Mobile, Alabama 36602

Sam.matheny@volkert.com

Re: Soils Explorations and Geotechnical Evaluations for the City of Mobile Civic Center
Arena Facility at 401 Civic Center Drive in Mobile, Alabama

Dear Mr. Matheny:

Geotechnical Engineering-Testing, Inc. (GET) is pleased to submit this report of our soils explorations and geotechnical engineering evaluations for the proposed design and construction of the City of Mobile – Civic Center Area facility in Mobile, Alabama. This report includes the results of the soils explorations program and our recommendations for site preparations, design and construction of building foundations for the currently planned structure, and the design and construction of pavements.

The recommendations provided in the attached report are based in part on the project information provided to GET and only apply to the specific project and site discussed in the report.

Please call Curt Doyle, P.E. if you have any questions regarding this report.

Sincerely,

GEOTECHNICAL ENGINEERING-TESTING, INC.



Curt Doyle, P.E.
Principal Engineer
Alabama License No. 25733
Date: 8/16/2024



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INTRODUCTION

Geotechnical Engineering-Testing, Inc. (GET) has completed the authorized soils explorations and geotechnical engineering studies for the design phase of the new Civic Center Arena building for the City of Mobile. The geotechnical studies included planning and performing the soils exploration and laboratory testing program, evaluation of the soils explorations results, providing general site earthwork recommendations, providing soil parameters for foundation designs, making foundation recommendation for the building structure, providing pavement recommendations for parking and drive areas, and preparation of this report of findings and recommendations. Our professional services for this project have been performed, findings obtained, and recommendations prepared in accordance with generally accepted local geotechnical engineering principles and practices. This warranty is in lieu of all other warranties, either expressed or implied.

This report presents the results of the geotechnical field exploration and laboratory testing programs, provides relevant soil parameters for use by the designers and contractors, and provides recommendations for geotechnical site preparation, building foundation design/construction, and pavement design.

Details of our findings and recommendations are set forth in the following sections of this report.

OBJECTIVES/SCOPE OF WORK

The geotechnical design services for this project have been performed in general accordance with the proposed scope of work dated March 14, 2024, that was developed by GET engineers based upon the information requested by Mr. Sam Matheny of Volkert, Inc and the project design team. Authorization for these services was received on April 9, 2024.

The scope of geotechnical services deemed appropriate for this study, based upon the project requirements, our understanding of the project as summarized herein, and with reliance on our knowledge of the local geology and with similar project parameters, included the following:

- Perform sixteen (16) soil test borings to depths of 90 to 100 ft below the existing ground surface within (or near) the footprint of the proposed civic center building structure.
- Core the existing asphalt pavements and perform four (4) shallow auger borings within the area of proposed drives.
- Visual classifications of recovered soil specimens.
- Laboratory soil testing program.
- Assessment of groundwater conditions.
- Development of recommended design soil parameters.
- Engineering evaluations and recommendations.
- Preparation of this report of findings and recommendations.

PROJECT DESCRIPTION

The new City of Mobile Civic Center Arena structure is to be located at the location of the existing civic center and theater. These structures are to be removed (demolished) in a separate contract. The proposed footprint will encompass approximately the eastern one-half of the existing civic center and all of the existing theater building. The proposed footprint will extend approximately 30 ft east of the existing theater building and approximately 200 ft south of the existing theater building into the existing parking lot. A loading dock will be located on the south side of the proposed structure. The north side of the proposed structure will parallel Civic Center Drive and the front of the new structure will face to the west. Preliminary site development drawings received from Mr. Matheny on June 28, 2024, indicate that a small parking lot will be constructed on the east side of the building, a heavy-duty paved drive area will be constructed on the south side of the building, an approximate 300 ft long drive lane will be constructed along Civic Center Drive and other miscellaneous drive areas will be included in the project. The site development drawings indicate numerous small retaining walls to be constructed around the perimeter of the building. There will also be a concrete plaza area on the west (front) side of the proposed civic center.

The general project location is identified on a Highway Location Map in Figure 1 of this report.

This project generally consists of the construction of a 3-level structure, encompassing an approximate 150,000 square ft footprint and will have approximate dimensions of 345 ft by 430 ft. Based upon preliminary structural drawings received from Mr. Matheny on June 26, 2024, we understand that the first floor level will be the event level. The second level (the main concourse) will be constructed at about 25.5 ft above the ground floor, the third level (suite level) will be constructed at about 38.5 ft above the ground floor and the suite roof will be about 52.5 ft above the ground floor. A catwalk will be constructed at about 60 ft above the event level floor and the civic center roof will be about 80 ft above the ground floor. No preliminary architectural details for the structure have been provided at this time.

Based upon preliminary information received on May 21, 2024 from Mr. Mark Boger, P.E. of MBA Engineers, Inc., the structural engineer responsible for the foundation design, column loads for the structure will range between 30 kips and 1100 kips with the majority of the column loads varying between 300 to 900 kips. Based upon email communication with Ms. Erin Kueht, P.E., of Walter P. Moore, we understand that the proposed finished floor elevation, FFE, of the ground level (event level) slab for the building structure will be essentially the same as the existing structure at about elevation +17.5 ft. Parking areas are assumed to be near existing grade at about elevation +13 ft. We understand that some pits could be as deep as 8 ft (approximately el. +9 ft)

SITE DESCRIPTION

General Site Description

As previously described, approximately one-half of the proposed civic center arena building will be constructed over the footprint of the existing civic center and theater buildings. The remainder will be constructed over the existing walkways between the two buildings, in the grassed lawn area, east of the theater building and in the parking lot to the south of the theater building. The existing building structures will be demolished in a separate contract. As part of the demolition contract, concrete columns, floor slabs, footings and pile caps are to be removed to a minimum of 8 ft below the adjacent finish grade. The existing pile foundations will remain in place. The existing site plan with the proposed structure overlaid on the plan, provided by Mr. Keith Parker of GMC on April 9, 2024, indicates that there are numerous underground utilities beneath the footprint of the proposed structure.

Existing Conditions

Some of the structural sheets from the original design plans for the Municipal Auditorium and Theater dated April 10, 1961 were provided to us by Mr. Matheny. We understand that the as-built plans are unavailable. Based upon the details indicated in the original design, the existing structures are supported on either 12-inch driven displacement piles or cast-in-place concrete piles having a minimum 14-inch butt diameter and 8-inch minimum tip diameter. Piles were designed for a 35-ton capacity. Actual pile type and lengths installed are unknown.

Site Topography

Based upon the topographic data from the site plan, the existing civic center building, the connector structure between the civic center and the theater and the northern one-third of the theater building have a FFE of about elevation +17.5 ft. The southern two-thirds of the theater building has a FFE of about elevation +13.5 ft. The topography of most of the surrounding grassed and paved areas is about elevation +13 ft but there is a small pedestrian walkway area between the existing civic center and theater where the ground surface rises from about elevation +13 ft to about elevation +17 ft.

SOILS EXPLORATIONS PROGRAM

The procedures for the field exploration and laboratory testing programs utilized on this project are summarized in the following sections of this report.

Boring Locations

The locations of the borings performed for this project were selected by GET personnel based upon the preliminary project site plans provided by Mr. Matheny on March 13, 2024. The site plan was overlaid on a Google Earth image and GPS coordinates of the boring locations were selected. In the field, boring locations were established using a network rover submeter GPS surveying instrument. Approximate soil boring locations are shown on the Boring Location Plan sheets included in Appendix A of this report. The state plane coordinates of each location are presented on the Logs of Borings. Some borings may have been offset slightly due to underground utilities or obstructions. The boring elevations were determined by GET personnel using the elevations provided on the existing site survey provided by Mr. Parker.

Field Explorations

The soils explorations for this project included performing 16 deep soil test borings within (or in close proximity to) the planned building footprint. One planned boring (B-12) was eliminated after encountering underground obstacles during several attempts to advance the boring. These borings generally extended to depths of about 90 ft to 100 ft below ground surface. The borings were performed with a Mobile B-37 core-boring rig. The boreholes were advanced using the mud rotary method of drilling. When performing the mud rotary method of drilling, a bentonite drilling fluid was circulated through the drilling rods and boreholes to return the soil cuttings to the ground surface and to provide a positive head on the walls and bottom of the boreholes.

The soil test borings were conducted in general accordance with standard drilling and sampling procedures. Due to encountering numerous active and abandoned utilities within the first couple of borings attempted, the upper 4 ft of the remaining borings were performed with a hand auger to be clear of utilities and other unidentified obstructions. Below this depth, standard penetration tests were performed and split spoon soil samples were collected continuously to a depth of 7.5 ft, and then at the standard 5 ft center-to-center intervals to the boring termination depths. Where cohesive soils were encountered, undisturbed tube samples were collected or attempted with 3-inch diameter thin-walled tube samplers in lieu of or between the SPT sample intervals.

Split spoon soil samples collected during the boring operations were visually described, logged, placed in moisture tight plastic bags and, along with the sealed tube samples, transported to the laboratory. At the laboratory, the samples were visually examined to confirm or adjust field classifications. Selected samples were laboratory tested to determine some engineering properties to aid in analyses.

Within the planned pavement areas, four shallow auger borings were performed. At each boring location the existing asphalt was cored and the borings were advanced with an approximate 3-inch diameter, bucket-type hand auger. Soil borings were advanced to a depth of about 5 ft below ground surface except where auger refusal was met.

Logs of Boring for the soil test borings were prepared using visual classifications of the soils and/or laboratory test results. These Logs of Boring are included in Appendix B of this report.

Seven shallow auger borings (C-1 through C-7) performed at the time of the geotechnical study for the Civic Center parking garage that is currently under construction have also been included in Appendix B of this report.

Laboratory Testing

The laboratory testing program included performing physical laboratory soil mechanics tests on selected soil samples that were recovered from the borings. Tests included moisture content, percent passing the #200 sieve, grain size distribution analysis, Atterberg limits, unit weight, one-point triaxial shear with the sample confined near the overburden pressure, specific gravity and one-dimensional consolidation tests. Tests were performed in general accordance with applicable laboratory soil testing standards. Some classification test results are shown on the Logs of Boring opposite the respective samples tested. A summary of classification test results and other test reports are included in Appendix C of this report.

GENERAL SUBSURFACE CONDITIONS

Site Geology

Based upon a review of the Quaternary Geologic Map of the Mobile Quadrangle dated 1988, the project site lies within the alluvial, coastal, delta and low-terrace deposits of the quaternary system. The delta deposit is light-gray, yellowish-gray, and brownish-gray clay, silt and sand, intermixed and interbedded. The deposit is poorly sorted to well-sorted, weakly bedded; locally includes thin stringers of well-rounded quartz granule gravel, discontinuous lenses of peat and scattered shell debris. Mapped areas include small, younger alluvial, colluvial, and swamp deposits of the Holocene age. The geologic map of the region is included in Figure 2 of this report.

Subsurface Soil Conditions

The subsurface soils at the project site varied somewhat from the surface to a depth of about 25 to 30 ft below existing grade. Below a depth of about 25 to 30 ft, the subsurface soil conditions were relatively uniform. Generally, the upper soils consisted of firm sands from the surface to a depth of 5 ft. These upper sands were underlain by very loose silty sands and clayey sands from 5 ft to about 25 to 30 ft below ground surface. Within this loose sand layer, a 5 to 15 ft thick layer of very soft clay and organic clay was encountered in 10 out of the 16 borings performed. This soft clay layer was generally encountered at 10 to 15 ft below ground surface. Below a depth of 25 to 30 ft, the soils generally consisted of firm and dense sands and silty sands extending to boring termination depths of 90 to 100 ft. There were exceptions to these generalizations, particularly within the upper soils. For instance, six of the soil borings did not encounter the stratum of soft or organic clay within the loose sands.

Within the existing pavement areas, approximately 4 to 5 inches of asphalt was encountered in the borings. Generally, a mix of red, brown and light gray silty sands were encountered below the asphalt surface. There were also some minor organics and gravel encountered.

Details of the soils encountered are presented by the Logs of Boring. The soil boring logs provided with this report are representative of subsurface conditions at their respective locations and for their respective vertical reaches. However, local variations characteristic of the subsurface materials of the region may be encountered during construction. The boring logs and related information are based on the driller's logs and visual examination of soil samples in the laboratory. The delineation between soil types shown on the logs is approximate and the description represents the interpretation of subsurface conditions at the designated boring location on the date drilled.

The generalized subsurface soil conditions are shown by the Subsurface Diagrams included in Appendix D of this report.

Groundwater Conditions

Groundwater was not encountered in the 5-ft deep auger borings that typically extended to about elevation +8 to +9 ft. Groundwater was not measured in most of the deep borings at the time of drilling due to the use of drilling mud to advance the soil borings and most borings were backfilled immediately upon completion due to being a hazard to pedestrian traffic. Two-inch diameter PVC piezometers were installed at a depth of about 15 ft below ground surface adjacent to three of the boreholes (B-3, B-8 and B-14) performed to measure delayed groundwater levels. Groundwater was measured at depths of about 3.5 to 9 ft below ground surface in the piezometers (approximately elevation +7 ft to elevation +10 ft). It should be noted that at the nearby Civic Center Parking Garage project located approximately 300 ft south of the proposed civic center building structure, groundwater would typically seep into construction excavations at about elevation +7 to +8 ft.

Groundwater elevations shown on the boring logs represent the highest groundwater surface measured. Fluctuations in water table levels should be anticipated throughout the year. Absence of groundwater data on certain borings implies that no data is available but does not mean that groundwater will not be encountered at these locations or within the vertical reaches of these borings in the future.

GEOTECHNICAL EVALUATIONS AND RECOMMENDATIONS

The recommendations provided below are based upon the project information described herein, the available subsurface data collected, our engineering evaluations regarding the geotechnical matters, and our experience on projects in close proximity to this site, and the typical climate conditions of the area. If project information or design concepts change, we should be advised of these changes, and should be provided the opportunity to review our recommendations as presented in this geotechnical report taking into consideration the new design information.

Generalized Site Stratigraphy and Engineering Properties

Based on the results of the soils explorations and our engineering experience, we recommend that, where appropriate, the design parameters shown below be assigned to the respective subsurface soils and/or imported soils.

RECOMMENDED DESIGN SOIL PARAMETERS								
Soil Type	Approximate Depth	Approximate Elevation	Total Unit Weight (pcf)	Internal Angle of Friction	Cohesive Shear Strength	Lateral Pressure Coefficients		
						Active (K _a)	Passive (K _p)	At-Rest (K _o)
Firm Sand	0' – 7'	+15 - +8	120 pcf	29°	-	0.35	2.88	0.51
Very Loose Clayey Sand	7' – 15'	+8 – 0	110 pcf	26°	-	0.39	2.56	0.56
Soft Clay	15' – 27'	0 - -12	100 pcf		600 psf			0.71
Firm to Dense Sand	23' – 53'	-12 – 38	125 pcf	35°	-	0.27	3.77	0.42
Dense Sand	53' – 85'	-38 - -70	125 pcf	37°	-	0.25	4.02	0.40
Firm Silty Sand (Backfill)	NA	NA	120 pcf	34°	-	0.28	3.53	0.44
Aggregate Drainage Backfill	NA	NA	130 pcf	34°	-	0.28	3.53	0.44

Seismic Site Class

Seismic site classification according to the International Building Code (2015) is based on the average soil profile to a depth of 100 ft. In accordance with IBC Section 1613.3.2 and ASCE 7-10, this site is characterized as Site Class D.

Settlement Evaluations

Settlement analyses were performed for areas where the floor slab is to be earth supported. These analyses were based upon the quantity of fill soils required to raise a portion of the building footprint to planned FFE, estimated floor loading conditions, the soil conditions encountered in the borings performed, and the results of one-dimensional consolidation tests performed on selected samples in the laboratory. Consolidation tests were performed on 5 selected undisturbed

samples obtained of the very soft clay and organic clay materials encountered within the upper 15 to 27 ft across the site.

Due to the variations between the existing surface elevations and variations in planned finished grades, it is estimated that differential settlement will occur across grade supported slabs. In some areas, especially adjacent existing structures, this differential settlement could be abrupt.

Settlement analyses were performed for several different conditions at this site. Since a key factor in estimating settlement is the thickness of the compressible clay layer, settlement analyses were performed for the varying layer thicknesses identified in the soil borings. For the purposes of our analyses, we made estimates of settlement for widespread fill loading using an average coefficient of consolidation based upon the results of consolidation tests and local experience. We estimate that settlement of the floor slab could range between 1 and 3 inches at the center of the loaded area. Depending on the amount of organics present within the soft clay layer, settlement could be slightly greater in some areas. At the perimeter of a loaded area, settlement is generally about 50 percent of the center settlement. At the corner of the loaded area, settlement is generally about 25 percent of the center settlement.

Utilizing an average coefficient of consolidation obtained from the laboratory consolidation tests, it is estimated that approximately 50 percent of the primary consolidation will occur within about 90 days after loading and 90 percent of the primary consolidation will occur within about 365 days after loading. However, since the layer thickness and consolidation properties are widely variable, this could be significantly less or slightly longer.

In areas where the existing building floor slab is about elevation +17.5 ft, no settlement will occur since the area has been previously loaded. In locations where pile foundations will remain in-place, settlement should not occur. However, between these areas and in areas that will receive up to 4 ft of new fill soils, differential settlement is likely to occur.

It should be noted that the settlement of the soils beneath areas that will receive up to 4 ft of fill will also place drag loads on the deep foundations to be constructed in those areas, thereby reducing the long-term capacity of the pile. Pile foundations are discussed in greater detail later in this report.

Due to potential differential settlement of an earth-supported floor slab and reduction in pile capacities due to drag loads, we recommend one the three following options:

1. The floor slab could be pile supported and a structural slab constructed. This option would probably be the least desirable due to the additional foundation costs.
2. Light weight aggregate fill may be utilized for fill materials beneath the floor slab. The materials typically have a unit weight of about one-third that of soil but can adequately support the slab. Light weight aggregate is a manufactured product and is not locally produced, therefore shipping costs will typically be high.
3. In the areas of the project where the existing grade or FFE of an existing structure is below elevation +17 ft, a preload/surcharge program could be implemented to cause settlement to occur prior to building construction. A preload/surcharge program would also eliminate the drag loads placed on pile foundations.

It is our recommendation that the areas requiring fill be preloaded and surcharged prior to building construction. To provide an adequate load to stress the very soft clay soils greater than the proposed loads and to achieve about 90 percent of the anticipated settlement within 90 days, we recommend that a surcharge load of at least 5 ft of soil fill material be placed above the design FFE (top of surcharge height at approximately elevation +22.5 ft). In areas where required, the top of the surcharge should extend to the perimeter of the building footprint. An outline of the area requiring the preload/surcharge has been indicated in Figure 3 of this report. The contractor must properly stabilize the slopes of the preload and shall be responsible for maintaining erosion control of preload/surcharge materials until removal is permitted. Installation of wick drains will not be required for the above recommended preload/surcharge program at this site. If wick drains are utilized, the surcharge load could be reduced to approximately 3 ft of fill material. Wick drains would have to be installed on a 4 ft triangular spacing to approximately elevation -10 ft.

All preload soil placed below the design FFE should be placed in accordance with the recommendations for *General Geotechnical Site Preparation (Building Area)* found on pages 18 and 19 of this report. There are no material specific requirements for the surcharge soils placed above the FFE other than to have a wet unit weight of 120 pounds per cubic ft, pcf. However, if the surcharge materials will be utilized elsewhere onsite at a later date, these materials should meet the requirements for that application. Surcharge soils should be placed in about 1 ft thick lifts and compacted until firm with the bulldozer.

It is recommended that at least ten (10) settlement plates be installed at locations selected by the geotechnical engineer. For planning purposes, approximate settlement plate locations have been provided in Figure 3. We recommend that settlement plates be constructed in accordance with details of typical settlement plate assembly as shown in the 2022 ALDOT Standard and Special Drawings. It is recommended that the fill materials required below design FFE be placed and compacted prior to installing settlement plates. Settlement plates should be placed prior to placing the 5 ft of surcharge soils. While this will not allow the measurement of the total settlement. It will allow the contractor to place the fill and compact the soils below FFE easier and more rapidly. The construction of settlement plates should be the responsibility of the contractor, and the settlement plates should be installed by the contractor at the direction of representatives of the geotechnical engineer of record. The contractor shall be responsible for ensuring that the settlement plates are not damaged and/or disturbed during construction. The contractor will be responsible for replacing and resetting any disturbed/damaged settlement plates.

After the installation of settlement plates and all preload/surcharge materials, elevation measurements of the settlement plates should be made on a weekly basis by representatives of the geotechnical engineer until settlement essentially ceases as determined by the geotechnical engineer.

GET personnel are available to coordinate with the design team, program manager and demolition contractor to develop a preload/surcharge program package that will reduce impacts to project schedule.

Building Foundation Recommendations

Several options have been evaluated with regards to supporting the building loads on this project. Information regarding the various options and reasons for selection are presented below.

Shallow Foundation Option

Based upon the large column loads and the excessive settlement that would occur from the underlying compressible soils, shallow foundations were not deemed a feasible alternative for the column loads and/or other heavily loaded foundations on this project. Even after the soft soils are preconsolidated by a preload/surcharge program, extensive undercutting and backfilling of the loose sands and soft clay soils between 5 and 15 ft below ground surface would have to be accomplished to provide adequate bearing capacity for shallow foundations. This type of undercut/backfill operation would require extensive dewatering. An extensive site wide dewatering program could pose risks of settlement to nearby structures adjacent the project site supported on shallow foundations unless an adequate recharge program was established.

Lightly loaded walls may be supported on shallow foundations. Shallow continuous strip foundations with a minimum 3 ft width and minimum 2 ft depth below the lowest adjacent final grade will have an allowable bearing capacity of 1500 pounds per square ft, psf. This allowable bearing capacity is based upon a factor of safety of 3.0. Since the bearing capacity of sands is also a function of foundation width and depth, slightly higher bearing capacities may be available if required.

The preliminary drawings indicate that numerous small retaining walls will be constructed around the perimeter of the new structure as part of the site development and stair entrances to the building. Many of these structures are within the existing building footprint scheduled for demolition. The final recommendations for these structures will be provided following the additional explorations to be performed following demolition of the existing structures. For the purposes of preliminary design, it is recommended that retaining wall foundations be designed based upon an allowable bearing capacity of 1500 psf.

Deep Foundation Option

Due to the excessive settlement and insufficient bearing capacity of shallow foundations, we recommend supporting the column and other heavy loads on deep foundations. Typically for a structure such as this and the subsurface soil conditions at the site, we would recommend that foundations be supported by driven displacement piles. Driven displacement piles provide very high end bearing and skin friction capacity within the dense sands as encountered at this site. These type piles are commonly used for projects such as the one at hand. Installation of piles by driving with an impact hammer also provides a record of driving resistance for each individual pile providing assurance of the capacity of each pile installed.

However, since this structure will be constructed approximately 100 ft south of the 160 year old historic Malaga Inn (and another historic building structure) located on the north side of Civic Center Drive, it is our opinion that vibrations from the installation of driven displacement piles could cause or be perceived to cause damage to the structure(s). A vibration study performed at a nearby site, where precast concrete piles were installed in similar soil conditions, indicated that the vibrations measured as the peak particle velocity, PPV, from the installation of prestressed concrete piles was about 0.2 at a distance of 100 ft. Although the piles and associated hammer were larger than would be required for this project, it is anticipated that the PPV would likely be in excess of 0.1 at a distance of 100 ft.

There are numerous differing guidelines regarding maximum PPV values allowed in the areas of various types of structures. However, a PPV of 0.1 has been established as the limiting criteria for vibrations for historic structures or buildings extremely susceptible to vibration damage by several state and national agencies including the American Association of State Highway and Transportation Officials (AASHTO), Louisiana DOT, California DOT, and Federal Transit Authority. Based upon this information and discussions with a local pile driving contractor, it is our recommendation that driven displacement piles not be utilized for this project. Although the risk of structural damage to the nearby structures is minimal, the vibrations could cause architectural damage (i.e. cracking of plaster walls and minor settlement). Therefore, we do not recommend the use of driven displacement piles for this project.

We have evaluated several different types of drilled foundations that might be utilized for supporting the column loads. Drilled deep foundations typically produce essentially no ground vibrations and the noise level is significantly less than for driven piles. These type foundations include drilled shafts (piers), continuous flight auger (CFA), and drilled displacement piles.

Since the diameter and depths of drilled shaft (pier) foundations can be up to 8 ft and 200 ft, respectively, this type foundation is certainly capable of supporting the column loads planned for this structure. In fact, often the column is constructed directly on the shaft foundation eliminating the requirement for pile caps. However, due to the high groundwater present at this site, a drilling slurry would be required to construct the shafts via the “wet” method. This increases the cost of the shaft construction. Although various sizes and depth of shafts could be utilized for the different column loads, the preliminary drawings indicate that the number of shafts that would be required for this project will likely make this alternate economically unfeasible.

Based upon the laboratory testing performed on samples collected at this site and experience in the vicinity of the project, continuous flight auger (CFA) or auger cast piles were not considered a viable deep foundation alternative for this site. The firm sand layer from which the piles will obtain the required friction and end bearing capacity of the pile consists of relatively “clean” sands. Laboratory testing of these materials indicates that these materials generally had less than 10 % passing the number 200 sieve. Clean sands beneath the groundwater surface are highly susceptible to “mining” during pile installation. “Mining” is the condition when the surrounding soils are pulled to the auger, thereby loosening the surrounding soils. While this may only minimally reduce the capacity of the pile being installed, it could greatly reduce the capacity of adjacent piles that have previously been installed.

Therefore, it is our opinion that a drilled displacement pile foundation will be the most appropriate for this project given the conditions. Drilled displacement piles are similar to CFA piles but due to modifications in tooling and the equipment utilized, the soils are displaced to the side and very little spoil is produced, thereby essentially negating the potential for “mining” the clean sands. The disadvantage of a drilled displacement pile is that the depth of penetration into dense sands and load carrying capacity can be somewhat limited.

Drilled displacement piles are proprietary systems. Therefore, the contractor typically provides the size and length of pile required to achieve the design capacity based upon his equipment and methodology. Since this is a contractor designed and constructed foundation system, we recommend that a performance specification be established, with some minimum requirements, for the use of drilled displacement piles. For the purposes of design and the basis of the performance specification, we recommend the use of an 18-inch diameter drilled displacement pile and maximum design loads of 200 kips (compression) and 40 kips (tension) per pile based upon a factor of safety of 2.0. Based upon conversations with a drilled displacement pile contractor and past experience installing this type foundation system for the Civic Center Parking Garage project, it is our opinion that an 18-inch pile will achieve this capacity at approximately 35 to 40 ft below ground surface.

Our evaluations of the axial capacities of the piles only considered pile-soil interaction and not the structural characteristics of the pile material or the weight of the pile. *We recommend that the project structural engineer review and/or design the pile layout and lateral bracing for the piles.* Recommended design soil parameters for use by the project structural engineer in determining the lateral load capacity of the piles were provided in the table of Design Soil Parameters above. The contractor will be responsible for interpreting the results of the soils explorations (or obtaining additional information) when designing the drilled displacement pile foundation.

Our tension capacity analyses did not include the weight of the pile.

The piling contractor may elect to utilize a larger or smaller diameter pile, but we recommend that the pile diameter be a minimum of 16 inches. The drilled displacement piles should be installed with a minimum pile center-to-center spacing to pile diameter ratio (s/d) of 3. Maintaining this recommended s/d ratio should result in negligible single pile capacity reduction due to pile group effect.

Since a performance specification will be developed for the use of drilled displacement piles and the contractor will have some ability to vary the pile size, length and structural reinforcement, we recommend that pre-production and post-production pile load testing be performed. While the installation of each production pile will be monitored by a representative of the geotechnical engineer of record to ensure that all piles are constructed in the same manner as the test piles, the ability to verify the actual capacity of each pile installed on the project is not possible.

As part of the testing program, we recommend that five pre-production compression test piles be installed, and load tested. One tension load test shall be performed on a pre-production pile. We recommend that compression verification load tests be performed on 12 production piles. Pre-production test pile locations and production piles to be tested shall be selected by the geotechnical engineer of record.

Static pile load tests (compression) shall be conducted in general accord with procedures in ASTM D1143 and the static pile load test (tension) shall be conducted in general accord with procedures in ASTM D3689. All static load tests performed on each of the pre-production test piles shall be performed or monitored by representatives of the geotechnical engineer of record.

All pre-production load tests shall be performed to geotechnical failure, which is defined as excessive pile movement or plunge. This can enable a determination of the actual factor of safety of the test piles, along with providing settlement or pile top deformation data under various test loads up to failure. Ultimate capacity of the pile will be determined by any of the following methods: Davisson Offset Limit, Brinc-Hansen 90-percent Criterion or Butler-Hoy Criterion but will be limited to the load that produces a maximum of 1.0 inch of pile head deflection. All pre-production test piles must achieve 200 percent of design load to be considered passing. Consideration could be given to reducing the number of load tests if the first 3 pre-production tests exceed 250 percent of design load.

Load testing for verification testing of production piles may be performed using static load testing, rapid load tests (RLT) (usually Statnamic™) or dynamic load test (DLT) methods in accordance with ASTM D 1143 or ASTM D7383. A third-party engineer should be retained by either the owner or contractor to perform and evaluate the RLT or DLT tests. All verification tests performed on production test piles must achieve 175 percent of design load to be considered passing. Production piles to be load tested shall be selected by the geotechnical engineer of record. Additional testing shall be performed if verification tests fail to meet desired load. Consideration could be given to reducing the number of verification tests provided pre-production tests exceed 250 percent of the design load and pile integrity tests indicate no anomalies.

We recommend that pile integrity testing be performed by the geotechnical engineer of record or his subconsultant on all pre-production test piles, verification test piles, and 20 percent of production piles.

More detailed pile installation and testing requirements will be provided in a performance specification to be included in the project documents.

General Geotechnical Site Preparation (Building Area)

Below are some general guidelines and recommendations for site preparations in the building area. The means and methods of construction will be the responsibility of the contractor.

- Clear the proposed building construction area; these operations are anticipated to remove all deleterious materials at and near the surface such as organics, debris, underground utilities, old foundations, asphalt, concrete, etc. Clearing should include all remains of material from demolition activities. The clearing activities should extend a minimum of 10 ft outside the building footprint.
- During demolition of the existing buildings, we recommend that the existing pile caps be removed. The existing piles may remain in-place unless in conflict with planned pile foundations.
- Existing piles within the footprint of a planned pile cap but not in conflict with a planned pile may remain in-place if cut off below the pile cap elevation.

- A qualified soils technician should inspect and probe the insitu soils for zones of soft or very loose soils. Additional undercutting, if deemed necessary, should be directed by the project geotechnical engineer of record.
- The top 12 inches of the insitu soils should be compacted to at least 98 percent standard Proctor density (SPD) in all areas.
- Where offsite borrow soils will be required, these materials should be granular soils that are free of organics or deleterious materials with no more than about 25 percent passing a #200 sieve and that have a plasticity index of no more than six.
- Fill soils should be placed in loose lifts no thicker than 8 inches and each lift should be compacted to at least 100 percent SPD.
- Representative samples of the backfill/fill soils and insitu subgrade soils should be collected for classification and laboratory Proctor density testing. The maximum dry density, optimum moisture content, gradation, and plasticity should be determined. These tests are needed for quality control of compacted soils. Field density tests should be performed on the compacted insitu subgrade and fill soils. One test should be performed for each 2500 square feet of general fill area per lift of backfill/fill soils.

Floor Slab

We recommend that the subgrade soils beneath the floor slab be prepared as described in the *General Geotechnical Site Preparation* section above. Following the preparation of the subgrade soils as recommended, we recommend a modulus of subgrade reaction of 200 pci may be utilized for the floor slab design.

After the subgrade soils and back/fill soils are compacted and placed as noted above in the General Geotechnical Site Preparation section, we recommend that at least 6 inches of free-draining materials be placed as a capillary moisture break immediately beneath the concrete floor slab in the support building. Free-draining materials should consist of aggregate graded between 3/4 inch and No. 200 sieve, with no more than 75% passing the #4 sieve and no more than 5% material passing the No. 200 sieve. These materials should be compacted until firm. We also recommend a vapor barrier be placed between the free-draining materials and the concrete floor slab.

General Geotechnical Site Preparation (Pavement Areas)

- We recommend the removal of the existing asphalt pavements, concrete curbing and any deleterious material in the proposed roadway areas.
- After clearing, and cutting to design subgrade elevation, if necessary, the top 8 inches of the insitu soils should be compacted to at least 98 percent SPD or to the satisfaction of the geotechnical engineer.
- The parking areas shall be proof rolled with a loaded tandem axle dump truck to identify any weak spots that may be present. Proof rolling should be observed by a representative of the geotechnical engineer.
- Any required backfill/fill soils should be placed in maximum 8-inch thick loose layers, and each layer should be compacted and tested before placement of the succeeding lift.
- Where offsite borrow soils will be required, these materials should be granular soils that are free of organics or deleterious materials with no more than about 25 percent passing a #200 sieve and that have a plasticity index of no more than six.
- Each layer of backfill/fill soils should be compacted to at least 100 percent SPD.
- The top 8 inches of subgrade materials, whether insitu soils or fill soils, shall be compacted to at least 100 percent SPD in all areas.
- Representative samples of the backfill/fill soils and insitu subgrade soils should be collected for classification and laboratory Proctor density testing. The maximum dry density, optimum moisture content, gradation, and plasticity should be determined. Field density tests should be performed on the compacted insitu subgrade and fill soils. One test should be performed for each 2500 square feet of fill area per lift of backfill/fill soils. Where crushed aggregate will be required for this project, we recommend crushed aggregate base (ALDOT 825 B) be placed in lifts no thicker than 6 inches. Each lift should be compacted to at least 100 percent modified Proctor density (MPD).
- Pavement material properties should meet, and construction practices should be in accord with, the current ALDOT Standard Specifications. The minimum compaction requirements outlined in this report supersede the minimum requirements in the ALDOT specifications.

Pavement Recommendations

Currently, we have received no design traffic data. The preliminary site development drawings indicate that there will be a paved parking area for automobiles located on the east side of the building, heavy duty pavement on the south side of the building for truck loading/unloading, a lane addition to Civic Center Drive and other miscellaneous drives. After the pavement subgrade soils have been prepared as described previously, we recommend the following pavement sections:

Passenger Vehicle Parking Areas

Pay Item 424A-340: Superpave Bituminous Concrete Wearing Surface Layer, 1/2” Maximum Aggregate Size Mix, ESAL Range A/B (220 Lb/SY)

Pay Item 301A-012: Crushed Aggregate Base Course, Type B, Plant Mixed, 6” Compacted Thickness

New Roadway & Heavy Duty Areas

Pay Item 424A-340: Superpave Bituminous Concrete Wearing Surface Layer, 1/2” Maximum Aggregate Size Mix, ESAL Range A/B (220 Lb/SY)

Pay Item 424B-636: Superpave Bituminous Concrete Upper Binder Layer, 1” Maximum Aggregate Size Mix, ESAL Range A/B (275 Lb/SY)

Pay Item 301A-012: Crushed Aggregate Base Course, Type B, Plant Mixed, 6” Compacted Thickness

If additional loading information is provided for the roadways, the above recommendations can be modified if warranted.

Utility Trenches

Backfilling of storm drain and utility trenches must be performed in a controlled manner to reduce settlement of the backfill and potential cracking of overlying floor slabs and pavements. We recommend that utility trenches be backfilled with acceptable borrow or dense-graded crushed stone in 4 to 6-inch loose lifts compacted with mechanical piston tampers. We recommend that utility trenches be compacted to at least 95% SPD outside of pavement or building areas and at least 98% SPD beneath pavement or building areas. Should seepage occur in utility trenches, it may be necessary to “floor” the trench with dense-graded aggregate to provide a working surface.

Required Special Inspections

We recommend that the geotechnical engineer of record be allowed to review the project plans and specifications after further development to determine the appropriate Special Inspections and inspection frequency related to the soils and foundations.

Additional Investigations

We recommend that additional investigations be performed in areas of the site where structures currently exist following the demolition phase of the project. This will enable us to address foundations for some of the periphery structures and provide additional information to the foundation contractors that will be bidding the project.

CONSTRUCTION CONSIDERATIONS

Excavation Slopes

Due to the potential for high groundwater and the sandy materials encountered in the upper 5 to 10 ft we recommend that excavation slopes be constructed no steeper than 3:1 horizontal:vertical (H:V) or in accordance with OSHA regulations whichever is most stringent. The contractor will be responsible for maintaining safe excavation slopes during construction.

Site Grading

The upper 5 to 10 ft of the soils at this site are typically silty sand materials. Difficult excavation is not anticipated. These materials are generally suitable for backfill, however, the insitu materials may be more silty than off site borrow and therefore more moisture sensitive. Proper moisture conditioning will be required for reuse.

Upon completion of grading, care should be taken to maintain the subgrade moisture content near optimum prior to construction of pavements. Construction traffic over the completed subgrade should be avoided to the extent practical. The site should also be graded to prevent ponding of surface water on the prepared subgrades or in excavations. If the subgrade should become frozen, desiccated, saturated, or disturbed, the affected material should be removed or these materials should be scarified, moisture conditioned and densified prior to pavement construction.

Dewatering

Based upon the information provided at this time, it is not anticipated that a sitewide permanent dewatering system will be required for this project. In some of the deeper excavations extending to about elevation +7 to +8 ft, temporary dewatering may be required.

The contractor shall provide dewatering systems as required for excavations. The dewatering systems should be of sufficient size and capacity to prevent ground and surface water flow into the excavation and to allow work to be installed in a dry condition. The contractor shall control groundwater in a manner that preserves strength of foundation soils, does not cause instability or raveling of excavation slopes, and does not result in damage to existing structures. The contractor shall be responsible for the disposal of the water. Grading shall be maintained around excavations to prevent surface water from flowing into excavation areas.

Engineering Services During Construction

The engineering recommendations provided in this report are based on the information obtained from the soils explorations, laboratory testing program, and experience on similar projects. Regardless of the thoroughness of a geotechnical exploration program, there is always a possibility that conditions at locations remote from borings will be different from those at specific boring locations and that conditions will not be as anticipated by the designers or constructors. In addition, the construction process may itself alter soil conditions. Therefore, we recommend that a representative of the geotechnical engineer of record observe and document the construction procedures used, and the conditions encountered. Unanticipated conditions and inadequate procedures should be reported to the design team along with timely recommendations to address such conditions.

CONCLUSION

This report concludes the authorized design phase geotechnical engineering services for the City of Mobile Civic Center Arena in Mobile, Alabama. This geotechnical report has been prepared for the exclusive use of City of Mobile and members of the design/construction team for the specific project discussed in this document. In the event that any changes in the design or location or elevation of any of the project elements as outlined in this geotechnical report are planned, or if any structures are included or added that are not discussed in this document, the conclusions and recommendations contained herein shall not be considered valid unless the changes are reviewed and the conclusions and recommendations modified or validated by GET.

Our evaluations for the project were based upon the preliminary project plans and data received to date from the members of the program management and project design team that have been previously indicated. Additional assumptions have been outlined in the discussions contained in previous sections of this report.

This geotechnical report presents an evaluation of site conditions on the basis of conventional geotechnical procedures for site characterization.

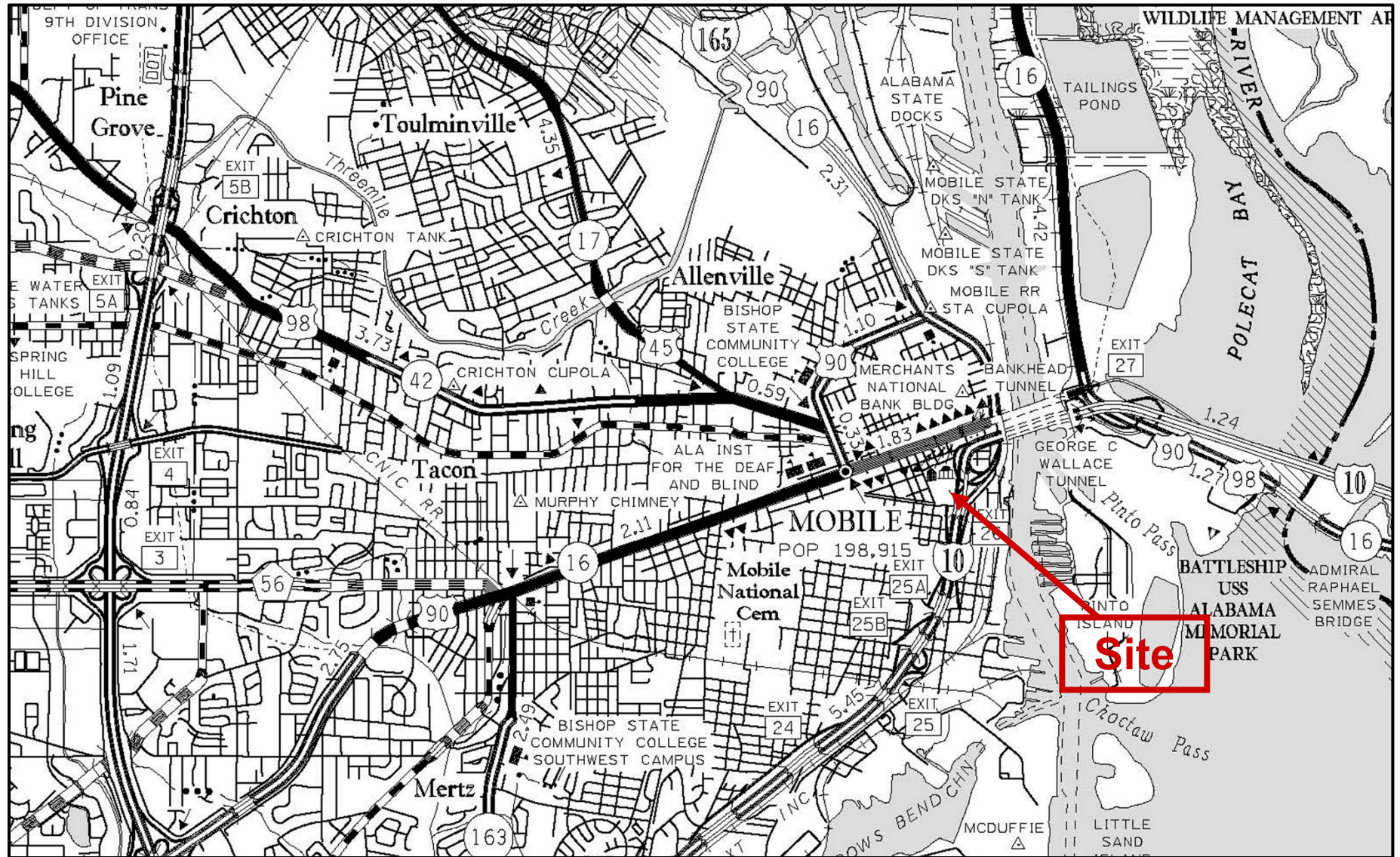
LIMITATIONS

The evaluations and recommendations presented by this report are based on the data obtained from the soil borings drilled at the locations shown on the boring location plan and the laboratory testing program performed. Additional assumptions may have been outlined in the discussions contained in previous sections of this report.

We prepared this report to aid in the evaluation of this site and to assist in the design of the project. The recommendations provided are based in part on the project information provided to GET and only apply to the specific project and site discussed in this report. If the project description or stated assumptions are incorrect or if additional information is available, correct or additional information should be conveyed to GET for review. Recommendations can then be modified if warranted.

Our professional services for this project have been performed, findings obtained, and recommendations prepared in accordance with generally accepted engineering principles and practices. The services identified herein were completed in a manner consistent with the level of care and skill ordinarily exercised by members of the profession currently practicing in the same locality and under similar conditions as this project. No other representation, expressed or implied, is included or intended, and no warranty or guarantee is included or intended in this report or any other instrument of service.

FIGURES



Source – General Highway Map Mobile County, Alabama, Alabama Dept. of Transportation, 2011



**City of Mobile
Civic Center
Mobile, AL**

**Highway Location Map
Figure 1**

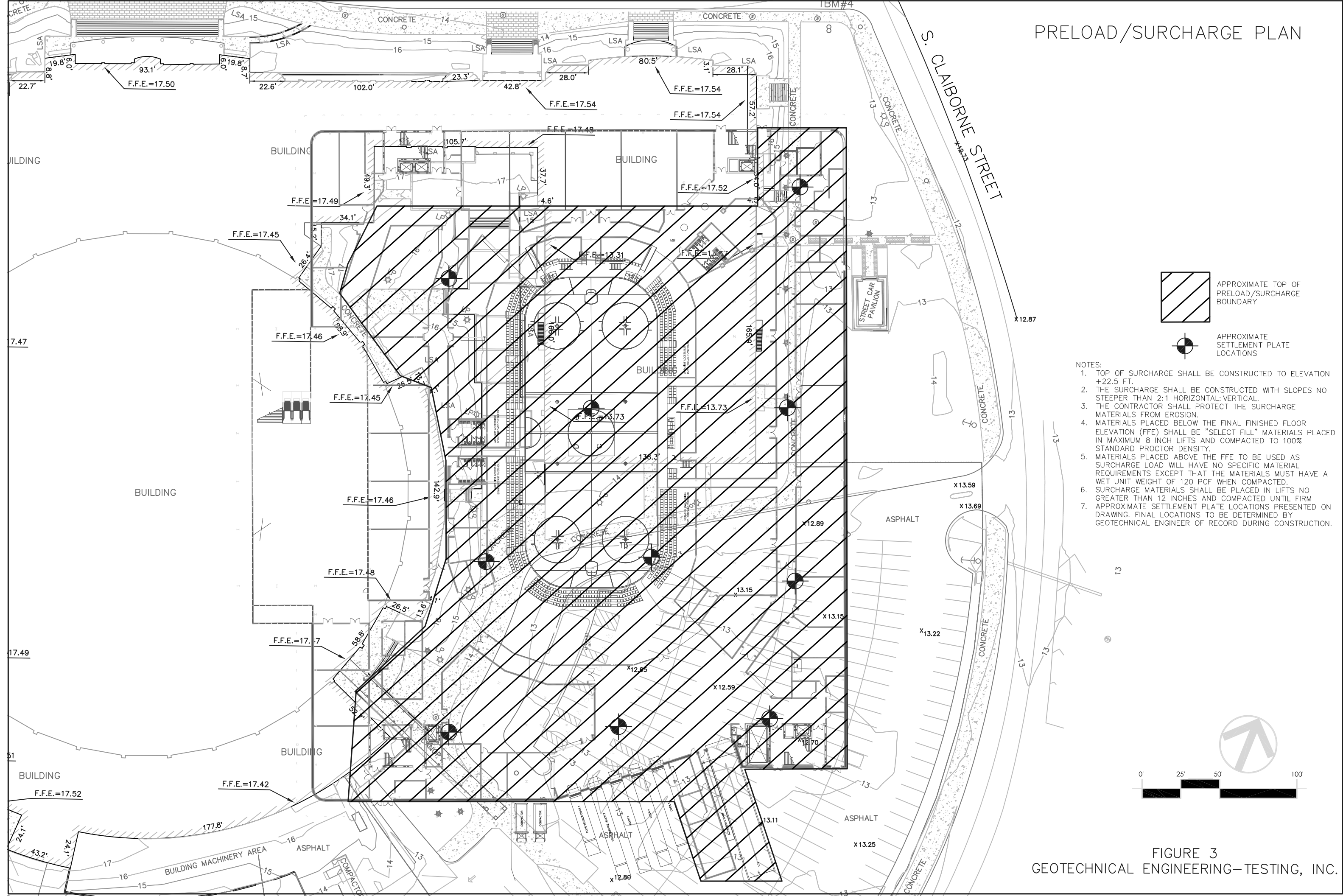


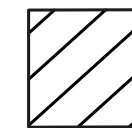
Source – Quaternary Geologic Map of the Mobile 4° x 6° Quadrangle, United States, USGS, 1988



DELTA DEPOSIT - Light-gray, yellowish-gray, and brownish-gray clay, silt and sand, intermixed and interbedded. Deposit is poorly sorted to well-sorted, weakly bedded; locally includes thin stringers of well-rounded quartz granule gravel, discontinuous lenses of peat and scattered shell debris. Mapped areas include small, younger alluvial, colluvial, and swamp deposits of the Holocene age.

PRELOAD/SURCHARGE PLAN



 APPROXIMATE TOP OF PRELOAD/SURCHARGE BOUNDARY

 APPROXIMATE SETTLEMENT PLATE LOCATIONS

- NOTES:
1. TOP OF SURCHARGE SHALL BE CONSTRUCTED TO ELEVATION +22.5 FT.
 2. THE SURCHARGE SHALL BE CONSTRUCTED WITH SLOPES NO STEEPER THAN 2:1 HORIZONTAL:VERTICAL.
 3. THE CONTRACTOR SHALL PROTECT THE SURCHARGE MATERIALS FROM EROSION.
 4. MATERIALS PLACED BELOW THE FINAL FINISHED FLOOR ELEVATION (F.F.E.) SHALL BE "SELECT FILL" MATERIALS PLACED IN MAXIMUM 8 INCH LIFTS AND COMPACTED TO 100% STANDARD PROCTOR DENSITY.
 5. MATERIALS PLACED ABOVE THE F.F.E. TO BE USED AS SURCHARGE LOAD WILL HAVE NO SPECIFIC MATERIAL REQUIREMENTS EXCEPT THAT THE MATERIALS MUST HAVE A WET UNIT WEIGHT OF 120 PCF WHEN COMPACTED.
 6. SURCHARGE MATERIALS SHALL BE PLACED IN LIFTS NO GREATER THAN 12 INCHES AND COMPACTED UNTIL FIRM
 7. APPROXIMATE SETTLEMENT PLATE LOCATIONS PRESENTED ON DRAWING. FINAL LOCATIONS TO BE DETERMINED BY GEOTECHNICAL ENGINEER OF RECORD DURING CONSTRUCTION.

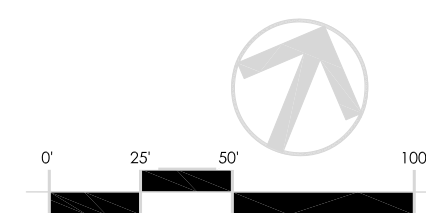
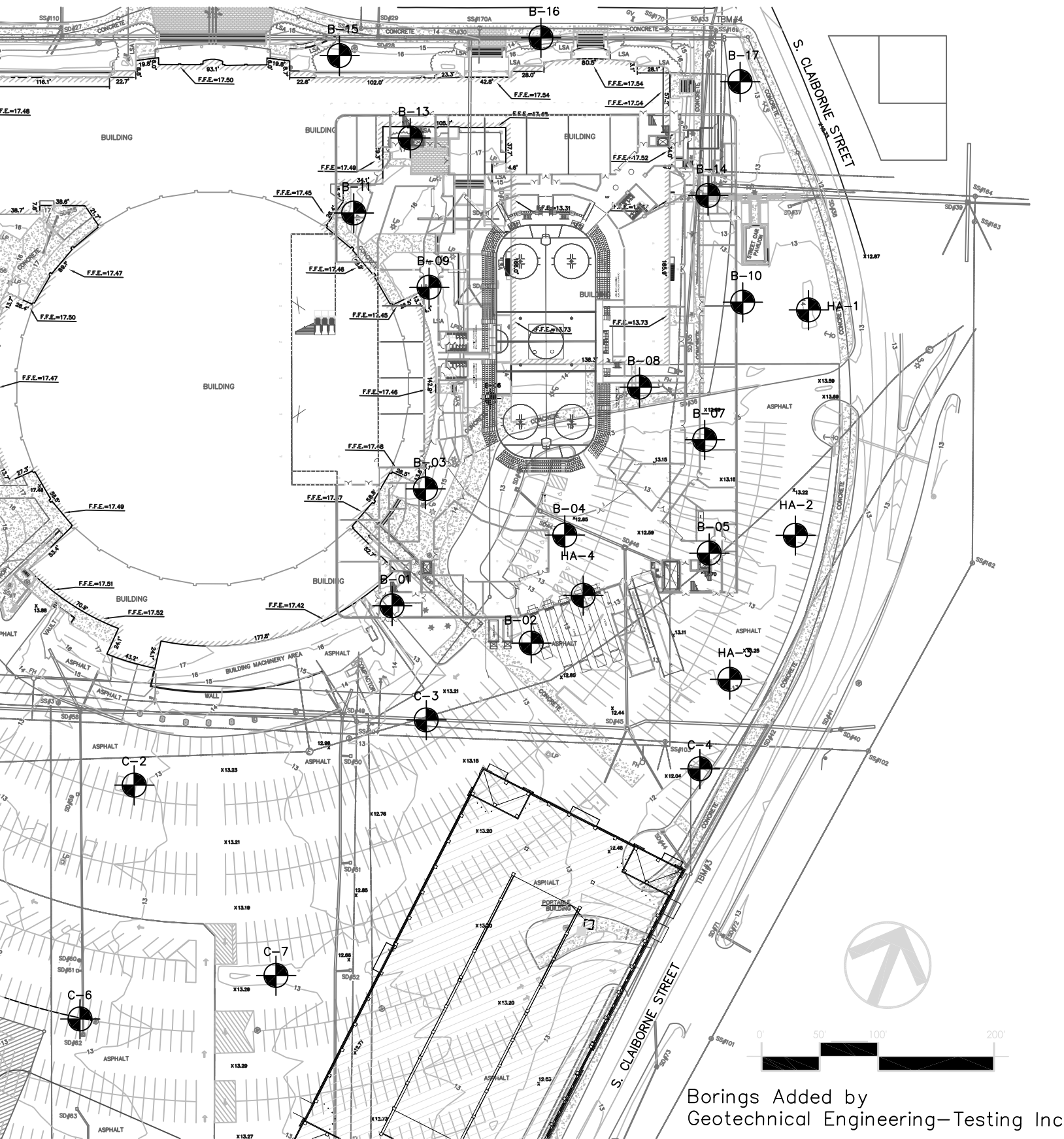


FIGURE 3
GEOTECHNICAL ENGINEERING—TESTING, INC.

APPENDIX A

BORING LOCATION PLAN SHEET



Borings Added by
Geotechnical Engineering-Testing Inc

APPENDIX B

LOGS OF BORING

PROJECT NAME:

DATE DRILLED:

G.E.T. PROJ. NUMBER:

BORING DEPTH: 0 FT.

PROJECT LOCATION:

BORING ELEV.:

DRILL RIG:

DATUM:

WATER DEPTH:



BORING NUMBER: LEGEND

DRILL METHOD:

REMARKS:

BORING LOCATION:

DRILL CREW:

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	INSITU CBR (DCP)	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
0		TOPSOIL										
5		SAND										
10		CLAY										
15		SILT										
20		GRAVEL										
25		ORGANICS										
30		PEAT										
35		SILTY SAND (EXAMPLE OF A SOIL MIXTURE)										
40		SPLIT-SPOON SAMPLE (STANDARD PENETRATION TEST)										
45		UNDISTURBED TUBE SAMPLE										
50		SAMPLE NOT RECOVERED										
55		VANE SHEAR										
		B.T. @ 0 FT										
60												
65												
70												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/3/24

BORING DEPTH: 100 FT.

BORING ELEV.: 13.4 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-1

BORING LOCATION:

N: 249796 **E:** 1797464

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
0			1									
		Brown silty sand	2									
			3									
5			4	12								
		Firm grayish brown silty sand w/ trace clay	5	3		29	23	5		15.0		SC-SM
			6	5								
10												
		Very soft dark gray clayey sand	7	WOH		29	29	16		28.8		SC
15		Very soft dark gray organic clay	T-1			51	85	62	72	55.9	c=0.28	CH
		Very loose gray silty sand w/ trace clay	8	3								
20												
		Soft light brown sandy clay	9	4		47	27	8		54.4		CL
25												
			10	25								
30		Firm to dense light brown sand										
			11	27								
35												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

REMARKS:

DRILL CREW: ES,BT,
PH(LOGGER)



BORING LOCATION:

N: 249796 **E:** 1797464

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _f	N _c		L.L.	P.I.				
35												
40		Firm to dense light brown sand	12	33		24	NP	NP		7.3		SP-SM
45			13	35								
50		Dense to firm light brown & gray sand	14	25								
55			15	30		22	NP	NP		5.1		SP-SM
60		Dense light brown sand	16	41								
65			17	44								
70			18	44								

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL.GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/3/24

BORING DEPTH: 100 FT.

BORING ELEV.: 13.4 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-1

BORING LOCATION:

N: 249796 E: 1797464

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
70												
75			19	35								
80			20	35		22	NP	NP		3.8		SP
85		Dense light brown sand	21	38								
90			22	35								
95			23	44								
100		B.T. @ 100 FT	24	44								
105												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS: ENCOUNTERED TREE AT 11'

DATE DRILLED: 5/2/24

BORING DEPTH: 100 FT.

BORING ELEV.: 13.0 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-2

BORING LOCATION:

N: 249811 E: 1797587

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
0		Asphalt	1	16								
		Firm red and brown silty sand	2	14								
		Firm light brown & light gray silty sand	3	16								
5		Firm light brown & light gray silty sand w/ trace clay	4	15								
			5	2		27	25	6		20.3		SC-SM
		Very loose gray silty clayey sand	6	3								
10												
		Large amount of organics	7	WOH								
15			T-1									
		Very loose gray clayey sand	8	3		26	28	12		40.2		SC
20												
			9	27								
25												
		Firm light brown & light gray sand w/ silt	10	23								
30												
			11	21		25	NP	NP		7.8		SP-SM
35												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS: ENCOUNTERED TREE AT 11'

DATE DRILLED: 5/2/24

BORING DEPTH: 100 FT.

BORING ELEV.: 13.0 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-2

BORING LOCATION:

N: 249811 **E:** 1797587

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
35												
40		Firm light brown & light gray sand w/ silt	12	22								
45		Firm brownish yellow sand	13	22		21	NP	NP		4.3		SP
50			14	31								
55		Dense light brown & light gray sand w/ trace silt	15	45								
60			16	39		21	NP	NP		6.2		SP-SM
65		Dense light brown sand w/ organics	17	39								
70		Dense and very dense light brown sand w/ silt	18	43								

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS: ENCOUNTERED TREE AT 11'

DATE DRILLED: 5/2/24

BORING DEPTH: 100 FT.

BORING ELEV.: 13.0 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-2

BORING LOCATION:

N: 249811 E: 1797587

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
70												
75			19	41		22	NP	NP		11.4		SP-SM
80			20	48								
85		Dense and very dense light brown sand w/ silt	21	48								
90			22	50								
95			23	52								
100		B.T. @ 100 FT	24	46								
105												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS: PIEZOMETER INSTALLED AT 15 FT

DATE DRILLED: 5/6/24

BORING DEPTH: 100 FT.

BORING ELEV.: 16.0 FT.

DATUM:

WATER DEPTH: 8.7 FT.

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-3

BORING LOCATION:

N: 249900 **E:** 1797453

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
0			1									
		Silty sand	2									
			3									
5		Very loose brown silty sand	4	3		15				25.1		
			5	7								
		Loose light gray & light brown silty sand	6	8								
10												
			7	WOH		27	28	12		25.6		SC
15		Very loose gray clayey sand										
			8	WOH		28	28	16		22.7		SC
20												
		Very loose light brown & light red silty sand	9	4								
25												
			10	27								
30		Firm light brown sand										
			11	25								
35												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS: PIEZOMETER INSTALLED AT 15 FT

DATE DRILLED: 5/6/24

BORING DEPTH: 100 FT.

BORING ELEV.: 16.0 FT.

DATUM:

WATER DEPTH: 8.7 FT.

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-3

BORING LOCATION:

N: 249900 **E:** 1797453

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
35												
40			12	19		21	NP	NP		5.0		SP-SM
45			13	20								
50			14	29								
55		Firm light brown sand	15	27		22	NP	NP		5.1		SP-SM
60			16	26								
65			17	29								
70			18	25								

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS: PIEZOMETER INSTALLED AT 15 FT

DATE DRILLED: 5/6/24

BORING DEPTH: 100 FT.

BORING ELEV.: 16.0 FT.

DATUM:

WATER DEPTH: 8.7 FT.

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-3

BORING LOCATION:

N: 249900 **E:** 1797453

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
70												
75			19	22								
		Firm light brown sand										
80			20	25		34	NP	NP		5.5		SP-SM
85			21	24								
90			22	25								
		Firm reddish yellow sand										
95			23	27								
		Firm brown sand										
100		B.T. @ 100 FT	24	30								
105												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/16/24

BORING DEPTH: 100 FT.

BORING ELEV.: 12.6 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-4

BORING LOCATION:

N: 249908 **E:** 1797579

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
0												
5		Very loose reddish brown silty sand	4	2								
			5	2		28	20	7		26.2		SC-SM
10		Very loose gray silty sand w/ some clay	6	3								
15		Very loose gray clayey sand	7	0		28	21	14		27.9	c*=0.13	SC
20		Soft dark gray organic clay	8	3		97				87.3		
25		Stiff light gray sandy clay	9	13		29						
30			10	30								
		Dense & firm light brown & gray sand w/ silt										
35			11	25		24	NP	NP		5.3		SP-SM

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/16/24

BORING DEPTH: 100 FT.

BORING ELEV.: 12.6 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-4

BORING LOCATION:

N: 249908 **E:** 1797579

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
35												
40			12	32								
45		Dense & firm light brown & gray sand w/ silt	13	28		23	NP	NP		10.3		SP-SM
50			14	31								
55			15	40								
60			16	40		13	NP	NP		8.5		SW-SM
65		Dense grayish brown silty sand w/ gravel	17	44								
70			18	44								

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/16/24

BORING DEPTH: 100 FT.

BORING ELEV.: 12.6 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-4

BORING LOCATION:

N: 249908 **E:** 1797579

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
70		Dense grayish brown silty sand w/ gravel										
75		Dense light brown sand w/ silt	19	47								
80		Dense light brown and gray sand w/ gravel	20	49								
85			21	44								
90		Dense grayish brown sand	22	50								
95			23	49								
100		B.T. @ 100 FT	24	50								
105												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/23/24

BORING DEPTH: 90 FT.

BORING ELEV.: 12.8 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-5

BORING LOCATION:

N: 249940 **E:** 1797700

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
0		Firm light brown silty sand										
5			4	20								
		Very loose dark gray clayey sand	5	0		36	31	10		34.6		SC
10		Very loose dark gray silty sand w/ clay	6	2		32	NP	NP		22.5		SM
15		Very soft dark brown organic clay	7	0		82					c*=0.25	
			T-1									
20			8	8								
25		Dense light brown & gray silty sand	9	35								
30			10	36								
35		Firm brownish gray sand w/ silt	11	25		24	NP	NP		6.4		SP-SM

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/23/24

BORING DEPTH: 90 FT.

BORING ELEV.: 12.8 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-5

BORING LOCATION:

N: 249940 **E:** 1797700

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
35												
		Firm brownish gray sand w/ silt	12	25								
40												
			13	17								
45												
		Firm to dense yellowish brown sand										
			14	33		20	NP	NP		4.3		SP
50												
			15	30								
55												
			16	39								
60												
		Dense light gray sand with silt										
			17	37								
65												
			18	38		19	NP	NP		6.6		SP-SM
70												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/23/24

BORING DEPTH: 90 FT.

BORING ELEV.: 12.8 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-5

BORING LOCATION:

N: 249940 **E:** 1797700

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
70		Dense light gray sand with silt										
75			19	31								
		Dense brownish gray silty sand										
80			20	32								
85		Firm to dense reddish yellow silty sand	21	25								
90		B.T. @ 90 FT	22	39								
95												
100												
105												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/9/24

BORING DEPTH: 100 FT.

BORING ELEV.: 14.3 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-6

BORING LOCATION:

N: 249993 **E:** 1797476

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
0			1									
		Loose brown silty sand	2									
			3									
5			4	7		19				22.6		
		Loose light brown & gray silty sand w/ clay lenses	5	5								
			6	8								
10												
		Loose to very loose brownish yellow silty sand w/ clay										
			7	2		28	22	7		25.2		SC-SM
15												
		Very loose brownish yellow & light gray clayey sand	8	3								
20												
			9	19								
25												
		Firm light brown sand	10	23		24				4.2		SP
30												
			11	26								
35												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/9/24

BORING DEPTH: 100 FT.

BORING ELEV.: 14.3 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-6

BORING LOCATION:

N: 249993 **E:** 1797476

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
35												
40			12	29								
45			13	28								
50			14	26		22	NP	NP		3.1		SP
55			15	28								
60			16	33								
65			17	23								
70			18	24		17				4.2		SP

Firm light brown sand

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/9/24

BORING DEPTH: 100 FT.

BORING ELEV.: 14.3 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-6

BORING LOCATION:

N: 249993 **E:** 1797476

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _r	N _c		L.L.	P.I.				
70		Firm light brown sand										
75			19	28								
80			20	33								
85		Firm to dense reddish yellow & brown sand	21	34								
90			22	30								
95			23	38								
100		Dense brown sand	24	35								
		B.T. @ 100 FT										
105												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/22/24

BORING DEPTH: 100 FT.

BORING ELEV.: 13.2 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-7

BORING LOCATION:

N: 250029 **E:** 1797660

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _r	N _c		L.L.	P.I.				
0												
		Loose brownish gray silty sand										
5			4	7								
		Very loose dark gray silty sand	5	2		31				17.2		
			6	2								
10		Very soft gray sandy clay										
			7	0		73					c*=0.13	
15		Very soft dark brown & black organic clay										
			T-1			41	43	31				
			8	6								
20		Loose light gray sand w/ silt										
			9	32								
25												
			10	33								
30		Dense light gray & light brown sand w/ silt										
			11	38		26	NP	NP		7.3		SP-SM
35												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/22/24

BORING DEPTH: 100 FT.

BORING ELEV.: 13.2 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-7

BORING LOCATION:

N: 250029 **E:** 1797660

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
35												
40			12	36								
45			13	31								
50		Dense light gray & light brown sand w/ silt	14	35		22	NP	NP		5.8		SP-SM
55			15	30								
60			16	27								
65		Firm light brown & gray sand w/ silt	17	22								
70			18	23		20	NP	NP		5.8		SP-SM

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

DATE DRILLED: 5/22/24

BORING DEPTH: 100 FT.

BORING ELEV.: 13.2 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-7

BORING LOCATION:

REMARKS:

N: 250029 E: 1797660

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
70		Firm light brown & gray sand w/ silt										
75		Firm yellowish brown sand w/ silt	19	22								
80		Firm yellowish red silty sand w/ gravel	20	26								
85			21	30								
90		Dense reddish brown silty sand	22	33								
95		Dense brownish gray gravely sand w/ silt	23	36								
100		B.T. @ 100 FT	24	40								
105												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS: PIEZOMETER INSTALLED AT 15 FT

DATE DRILLED: 5/22/24

BORING DEPTH: 100 FT.

BORING ELEV.: 13.7 FT.

DATUM:

WATER DEPTH: 3.6 FT.

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-8

BORING LOCATION:

N: 250050 **E:** 1797591

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
0		Firm light brown silty sand										
5			4	16								
			5	1								
10		Very loose dark gray clayey sand	6	2		37	34	13		37.8		SC
15			7	0							c*=0.13	
20			8	1		105						
		Very soft dark brown & gray organic clay	T-1			99	157	114	46	96.4	c=0.56	CH
25			9	0		54					c*=0.13	
30		Firm light gray sand w/ silt	10	21								
35			11	19								

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

REMARKS: PIEZOMETER INSTALLED AT 15 FT

DRILL CREW: ES,BT,
PH(LOGGER)



BORING LOCATION:

N: 250050 **E:** 1797591

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _f	N _c		L.L.	P.I.				
35												
40		Firm light gray sand w/ silt	12	25			NP	NP		5.7		SP-SM
45			13	34								
50			14	34								
55		Dense brownish gray silty sand	15	35		21	NP	NP		12.5		SM
60			16	30								
65			17	22								
70		Firm yellowish red silty sand w/ gravel	18	24		23	NP	NP		10.8		SP-SM

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL.GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS: PIEZOMETER INSTALLED AT 15 FT

DATE DRILLED: 5/22/24

BORING DEPTH: 100 FT.

BORING ELEV.: 13.7 FT.

DATUM:

WATER DEPTH: 3.6 FT.

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-8

BORING LOCATION:

N: 250050 **E:** 1797591

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
70												
75			19	28								
		Firm yellowish red silty sand w/ gravel										
80			20	30								
85			21	32								
90			22	40								
		Dense light yellowish brown sand										
95			23	41								
100			24	43								
		B.T. @ 100 FT										
105												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/14/24

BORING DEPTH: 100 FT.

BORING ELEV.: 16.9 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-9

BORING LOCATION:

N: 250062 **E:** 1797391

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
0												
		Loose bown silty sand										
5			4	0								
			5	17		21	NP	NP		14.9		SM
		Firm light brown silty sand	6	17								
10												
		Loose yellowish brown silty clayey sand	7	6								
15												
		Very loose dark gray silty clayey sand	8	1		26	22	5		24.0		SC-SM
20												
		Stiff gray clay w/ organics	9	12							c*=0.75	
25												
		Firm to dense light brown sand	10	26		29	NP	NP		5.9		SP-SM
30												
			11	29								
35												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/14/24

BORING DEPTH: 100 FT.

BORING ELEV.: 16.9 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-9

BORING LOCATION:

N: 250062 E: 1797391

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
35												
40			12	26		26	NP	NP		4.3		SP
45		Firm to dense light brown sand	13	32								
50			14	38								
55			15	25		21	NP	NP		14.1		SM
60		Dense light brown & white silty sand	16	42								
65			17	40								
70		Dense brown sand w/ silt	18	48								

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/14/24

BORING DEPTH: 100 FT.

BORING ELEV.: 16.9 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-9

BORING LOCATION:

N: 250062 E: 1797391

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
70												
75			19	47		22	NP	NP		11.1		SP-SM
80			20	46								
85		Dense brown sand w/ silt	21	48								
90			22	44								
95			23	42								
100		B.T. @ 100 FT	24	44								
105												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/1/24

BORING DEPTH: 100 FT.

BORING ELEV.: 13.5 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-10

BORING LOCATION:

N: 250151 E: 1797646

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
0			1									
		Gray silty sand	2									
			3									
5		Firm gray sand w/ silt	4	12								
			5	2		30	NP	NP		37.3		SM
			6	WOH								
10		Very loose light gray silty sand w/ trace clay										
			7	2		28				23.5		
15												
			8	14								
20												
			9	18		24	NP	NP		3.8		SP
25		Firm light gray sand										
			10	23								
30												
			11	24		25	NP	NP		4.7		SP
35												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

REMARKS:

DRILL CREW: ES,BT,
PH(LOGGER)



BORING LOCATION:

N: 250151 **E:** 1797646

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _f	N _c		L.L.	P.I.				
35												
40		Firm light gray sand	12	25								
45			13	28		21	NP	NP		4.7		SP
50			14	40								
55		Firm to dense light brown & brown sand	15	42		22	NP	NP		6.8		SP-SM
60			16	34								
65			17	39		20	NP	NP		5.9		SP-SM
70			18	47								

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL.GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/1/24

BORING DEPTH: 100 FT.

BORING ELEV.: 13.5 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-10

BORING LOCATION:

N: 250151 **E:** 1797646

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
70												
75			19	47								
80		Firm to dense light brown & brown sand	20	48								
85			21	43								
90			22	43		31	NP	NP		5.7		SP-SM
95		Dense yellowish brown sand	23	50								
100		B.T. @ 100 FT	24	48								
105												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/17/24

BORING DEPTH: 100 FT.

BORING ELEV.: 16.8 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-11

BORING LOCATION:

N: 250097 **E:** 1797307

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
0												
5		Loose brownish gray silty sand w/ gravel	4	11		10	NP	NP		14.5		SM
			5	9								
10			6	7								
		Loose to very loose brownish gray silty clayey sand										
15			7	2		27	25	8		24.7		SC
20		Very loose gray clayey sand	8	0		27	28	11		22.5		SC
25			9	0		73						
		Very soft gray clay	T-1			120	150	82	41	66.6	c=0.47	MH
30			10	0		49						
35		Dense light brown sand	11	37								

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/17/24

BORING DEPTH: 100 FT.

BORING ELEV.: 16.8 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-11

BORING LOCATION:

N: 250097 **E:** 1797307

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
35		Dense light brown sand										
40		Dense brownish gray silty sand	12	38								
45			13	24								
50		Firm yellowish brown silty sand	14	24								
55			15	24								
60		Firm to dense dark yellowish brown sand w/ silt	16	37		21	NP	NP		7.4		SP-SM
65		Dense brownish gray sand with gravel	17	38								
70		Dense brownish gray sand w/ silt	18	42								

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/17/24

BORING DEPTH: 100 FT.

BORING ELEV.: 16.8 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-11

BORING LOCATION:

N: 250097 E: 1797307

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
70												
75			19	43								
		Dense brownish gray sand w/ silt										
80			20	37		22	NP	NP		8.6		SP-SM
85			21	41								
90			22	50								
		Dense to very dense light brownish gray sand with gravel										
95			23	48								
100			24	51								
		B.T. @ 100 FT										
105												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/15/24

BORING DEPTH: 100 FT.

BORING ELEV.: 17.2 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-13

BORING LOCATION:

N: 250174 E: 1797328

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
0		Very loose dark reddish brown clayey sand										
5			4	0								
			5	8								
10		Loose dark brown silty sand w/ gravel	6	11								
15		Very loose yellowish brown & red clayey sand	7	2								
20		Very loose gray clayey sand	8	0		27	26	9		22.2		SC
25		Very soft gray clay	9	0		71	76	55		88.8		CH
			T-1			78	78	57	56		c=0.46	
30			10	30								
35		Dense yellowish red sand w/ silt	11	32								

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/15/24

BORING DEPTH: 100 FT.

BORING ELEV.: 17.2 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-13

BORING LOCATION:

N: 250174 **E:** 1797328

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
35												
40		Dense yellowish red sand w/ silt	12	34		19	NP	NP		11.9		SP-SM
45			13	27								
50		Dense yellowish red & gray silty sand	14	30								
55		Dense to firm dark grayish brown silty sand	15	31		22	NP	NP		15.3		SM
60			16	24								
65		Dense brownish gray sand w/ gravel	17	36								
70			18	41								

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/15/24

BORING DEPTH: 100 FT.

BORING ELEV.: 17.2 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-13

BORING LOCATION:

N: 250174 E: 1797328

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _r	N _c		L.L.	P.I.				
70		Dense brownish gray sand w/ gravel										
75			19	42		19	NP	NP		10.2		SP-SM
80			20	44								
85			21	48								
		Dense dark brown sand w/ silt										
90			22	40		20	NP	NP		5.7		SP-SM
95			23	41								
100		B.T. @ 100 FT	24	48								
105												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS: PIEZOMETER INSTALLED AT 15 FT

DATE DRILLED: 5/21/24

BORING DEPTH: 100 FT.

BORING ELEV.: 13.0 FT.

DATUM:

WATER DEPTH: 3.6 FT.

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-14

BORING LOCATION:

N: 250225 **E:** 1797584

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
0		Very loose light grayish brown & gray silty sand										
5			4	5		25	NP	NP		11.4		SP-SM
		Very loose gray silty clayey sand	5	2								
10			6	2		29	NP	NP		21.5		SM
		Very loose gray silty sand w/ trace clay										
15			7	WOH		126						
		Very soft dark brown & black organic clay										
			T-1			32	50	33	89		c=0.25	
20			8	11								
		Firm light gray sand w/ silt										
25			9	30								
		Dense brownish yellow silty sand										
30			10	32		24	NP	NP		4.7		SP
		Dense grayish brown to gray sand										
35			11	40								

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS: PIEZOMETER INSTALLED AT 15 FT

DATE DRILLED: 5/21/24

BORING DEPTH: 100 FT.

BORING ELEV.: 13.0 FT.

DATUM:

WATER DEPTH: 3.6 FT.

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-14

BORING LOCATION:

N: 250225 **E:** 1797584

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
35												
40		Dense grayish brown to gray sand	12	31								
45			13	42								
50			14	45		20	NP	NP		5.6		SP-SM
55		Dense light brown sand w/ trace silt	15	32								
60			16	28								
65			17	37		20	NP	NP		6.9		SP-SM
70		Dense to firm gray silty sand	18	36								

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS: PIEZOMETER INSTALLED AT 15 FT

DATE DRILLED: 5/21/24

BORING DEPTH: 100 FT.

BORING ELEV.: 13.0 FT.

DATUM:

WATER DEPTH: 3.6 FT.

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-14

BORING LOCATION:

N: 250225 **E:** 1797584

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
70												
75			19	20								
		Dense to firm gray silty sand										
80			20	25								
85			21	30								
90			22	35								
		Dense gray & red silty sand										
95			23	32								
100			24	32								
		B.T. @ 100 FT										
105												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/20/24

BORING DEPTH: 100 FT.

BORING ELEV.: 16.9 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-15

BORING LOCATION:

N: 250218 **E:** 1797245

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
0												
5		Firm dark reddish brown silty sand	4	10								
			5	9								
10		Loose light grayish brown silty sand	6	11		19				17.7		
15		Very loose light yellowish brown silty sand	7	4								
20		Very loose dark gray silty sand	8	0		27	NP	NP		21.4		SM
25		Very soft dark gray clay	9	0		81						
			T-1			64	96	61	60	85.8	c=0.93	CH
30		Very stiff yellowish brown sandy clay	10	11								
35		Dense light brownish gray sand w/ trace silt	11	37								

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/20/24

BORING DEPTH: 100 FT.

BORING ELEV.: 16.9 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-15

BORING LOCATION:

N: 250218 E: 1797245

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
35												
40			12	37		21	NP	NP		6.2		SP-SM
		Dense light brownish gray sand w/ trace silt										
45			13	43								
50			14	43								
55			15	38								
60			16	46		22	NP	NP		6.7		SP-SM
		Dense to firm light brown sand w/ trace silt										
65			17	40								
70			18	46								

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/20/24

BORING DEPTH: 100 FT.

BORING ELEV.: 16.9 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-15

BORING LOCATION:

N: 250218 E: 1797245

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _r	N _c		L.L.	P.I.				
70												
75			19	28								
		Dense to firm light brown sand w/ trace silt										
80			20	21								
85			21	28								
		Firm brownish gray sand										
90			22	32								
95			23	33								
		Dense light brownish gray sand w/ silt										
100			24	33								
		B.T. @ 100 FT										
105												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/24/24

BORING DEPTH: 90 FT.

BORING ELEV.: 13.8 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-16

BORING LOCATION:

N: 250297 E: 1797400

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
0		2" Concrete										
		Loose grayish brown silty sand										
5			4	6								
			5	2		24	20	4		24.5		SC-SM
		Very loose light gray silty clayey sand	6	4								
10												
		Soft dark grayish brown organic clay w/ sand seams	7	4		156						
15			T-1									
		Firm to dense light gray sand with silt	8	26		21				9.0		
20												
			9	31								
25												
		Firm light brown and gray silty sand	10	25								
30												
			11	20								
35												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/24/24

BORING DEPTH: 90 FT.

BORING ELEV.: 13.8 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-16

BORING LOCATION:

N: 250297 **E:** 1797400

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
35		Firm light brown and gray silty sand										
40			12	20		23	NP	NP		6.0		SP-SM
45			13	25								
50			14	26								
55		Firm to dense light brown sand	15	30		22	NP	NP		4.0		SP
60			16	36								
65			17	38								
70			18	41								

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 5/24/24

BORING DEPTH: 90 FT.

BORING ELEV.: 13.8 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-16

BORING LOCATION:

N: 250297 E: 1797400

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _r	N _c		L.L.	P.I.				
70		Firm to dense light brown sand										
75			19	35								
80		Dense grayish brown sand	20	44		21	NP	NP		4.9		SP
85			21	31								
90		B.T. @ 90 FT	22	44								
95												
100												
105												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 4/25/24

BORING DEPTH: 100 FT.

BORING ELEV.: 13.2 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-17

BORING LOCATION:

N: 250326 **E:** 1797573

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
0			1									
		Silty sand	2									
			3									
5		Firm gray silty sand	4	13								
			5	2		28	NP	NP		21.7		SM
		Very loose light gray silty sand	6	4								
10												
		Very loose dark gray clayey sand	7	1		29	30	13		33.7		SC
15												
		Medium consistency brownish yellow sandy clay w/ gravel	8	5								
20												
			9	14								
25												
		Firm light brown sand w/ trace silt	10	30		22	NP	NP		6.8		SP-SM
30												
			11	25								
35												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG: MOBILE B37

DRILL METHOD: MUD ROTARY

REMARKS:

DATE DRILLED: 4/25/24

BORING DEPTH: 100 FT.

BORING ELEV.: 13.2 FT.

DATUM:

WATER DEPTH: NOT
MEASURED

DRILL CREW: ES,BT,
PH(LOGGER)



BORING NUMBER: B-17

BORING LOCATION:

N: 250326 **E:** 1797573

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _t	N _c		L.L.	P.I.				
35												
40			12	27		19	NP	NP		5.0		SP-SM
45		Firm light brown sand w/ trace silt	13	20								
50			14	26		22	NP	NP		6.3		SP-SM
55			15	31								
60		Dense light gray sand w/ trace silt	16	44		19	NP	NP		5.9		SP-SM
65			17	45								
70		Dense light brown sand	18	38								

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

REMARKS:

DRILL CREW: ES,BT,
PH(LOGGER)



BORING LOCATION:

N: 250326 **E:** 1797573

[illegible]

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL.GDT 8/16/24

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL.GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG:

DRILL METHOD: HAND AUGER

REMARKS:

DATE DRILLED: 7/1/24

BORING DEPTH: 5 FT.

BORING ELEV.:

DATUM:

WATER DEPTH: NWTE

DRILL CREW: CS,RS



BORING NUMBER: HA-3

BORING LOCATION:

N: 249846 E: 1797757

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _r	N _c		L.L.	P.I.				
0		4.3" Asphalt										
		Red silty sand	1									
		Light brown with dark brown sand with silt	2									
5		B.T. @ 5 FT										
10												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG:

DRILL METHOD: HAND AUGER

REMARKS:

DATE DRILLED: 7/1/24

BORING DEPTH: 5 FT.

BORING ELEV.:

DATUM:

WATER DEPTH: NWTE

DRILL CREW: CS,RS



BORING NUMBER: HA-4

BORING LOCATION:

N: 249866 E: 1797613

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _r	N _c		L.L.	P.I.				
0		4.25" Asphalt										
		Yellowish red silty sand	1									
		Black and gray silty sand	2									
		Light brown and yellow silty clayey sand	3									
5		B.T. @ 5 FT										
10												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG:

DRILL METHOD: HAND AUGER

REMARKS:

DATE DRILLED: 5/17/23

BORING DEPTH: 5 FT.

BORING ELEV.: 12.5 FT.

DATUM:

WATER DEPTH: NWTE

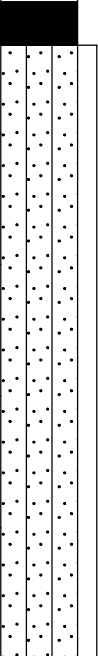
DRILL CREW: BT,
WL(LOGGER)



BORING NUMBER: C-1

BORING LOCATION:

N: 249548 E: 1797099

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _r	N _c		L.L.	P.I.				
0		3.91" Asphalt										
		Red, dark, brown, and gray silty sand with iron rock	1			9	NP	NP		24.8		SM
5		B.T. @ 5 FT										
10												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG:

DRILL METHOD: HAND AUGER

REMARKS:

DATE DRILLED: 5/17/23

BORING DEPTH: 5 FT.

BORING ELEV.: 12 FT.

DATUM:

WATER DEPTH: NWTE

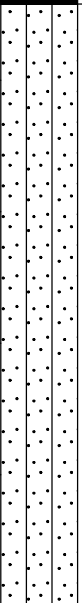
DRILL CREW: BT,
WL(LOGGER)



BORING NUMBER: C-2

BORING LOCATION:

N: 249570 **E:** 1797316

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _r	N _c		L.L.	P.I.				
0		5.23" Asphalt										
		Red, dark brown, and light brown silty sand	1			9	NP	NP		18.0		SM
5		B.T. @ 5 FT										
10												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG:

DRILL METHOD: HAND AUGER

REMARKS: REFUSAL @ 19" DUE TO TREE ROOTS

DATE DRILLED: 5/18/23

BORING DEPTH: 1.6 FT.

BORING ELEV.: 12.5 FT.

DATUM:

WATER DEPTH: NWTE

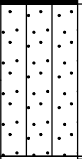
DRILL CREW: BT,
WL(LOGGER)



BORING NUMBER: C-3

BORING LOCATION:

N: 249716 **E:** 1797528

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _r	N _c		L.L.	P.I.				
0		4.70" Asphalt	1			7	NP	NP		21.4		SM
		Dark brownish gray, red, and yellow silty sand with iron rocks, organics, and silt pockets										
		B.T. @ 1.6 FT										
5												
10												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG:

DRILL METHOD: HAND AUGER

REMARKS:

DATE DRILLED: 5/18/23

BORING DEPTH: 5 FT.

BORING ELEV.: 12 FT.

DATUM:

WATER DEPTH: NWTE

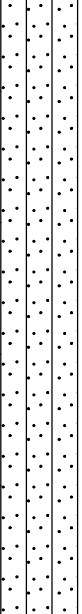
DRILL CREW: BT,
WL(LOGGER)



BORING NUMBER: C-4

BORING LOCATION:

N: 249765 E: 1797762

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _r	N _c		L.L.	P.I.				
0		3.34" Asphalt										
		Light gray, light brown, and red silty sand with silt pockets	1									
5		B.T. @ 5 FT										
												
10												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG:

DRILL METHOD: HAND AUGER

REMARKS: REFUSAL @ 33" DUE TO ENCOUNTERING ROCK

DATE DRILLED: 5/18/23

BORING DEPTH: 2.8 FT.

BORING ELEV.: 12 FT.

DATUM:

WATER DEPTH: NWTE

DRILL CREW: BT,
WL(LOGGER)



BORING NUMBER: C-5

BORING LOCATION:

N: 249279 **E:** 1797136

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _r	N _c		L.L.	P.I.				
0		3.55" Asphalt										
		Dark brownish gray and red silty clayey sand with organics	1			12	17	5		23.1		SC-SM
		B.T. @ 2.8 FT										
5												
10												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG:

DRILL METHOD: HAND AUGER

REMARKS: REFUSAL @ 8" DUE TO CONCRETE-GRAVEL

DATE DRILLED: 5/18/23

BORING DEPTH: 0.7 FT.

BORING ELEV.: 12 FT.

DATUM:

WATER DEPTH: NWTE

DRILL CREW: BT,
WL(LOGGER)



BORING NUMBER: C-6

BORING LOCATION:

N: 249366 **E:** 1797348

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _r	N _c		L.L.	P.I.				
0		2.29" Asphalt Light brown and gray gravel B.T. @ 0.7 FT	1			0	NP	NP		1.5		GP
5												
10												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER

G.E.T. PROJ. NUMBER: 24-120

PROJECT LOCATION: MOBILE, AL

DRILL RIG:

DRILL METHOD: HAND AUGER

REMARKS:

DATE DRILLED: 5/18/23

BORING DEPTH: 5 FT.

BORING ELEV.: 12 FT.

DATUM:

WATER DEPTH: NWTE

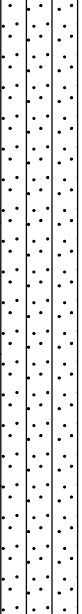
DRILL CREW: BT,
WL(LOGGER)



BORING NUMBER: C-7

BORING LOCATION:

N: 249464 E: 1797491

DEPTH IN FEET	LOG	DESCRIPTION	SAMPLE NO.	S.P.T.		W.C. %	ATTERBERG LIMITS		DRY UNIT WT. pcf	% MINUS #200	SHEAR STRENGTH tsf	UNIFIED CLASS
				N _r	N _c		L.L.	P.I.				
0		3.30" Asphalt										
		Red, gray, light yellowish brown silty sand with silt pockets	1			12	19	3		23.5		SM
5		B.T. @ 5 FT										
10												

NOTE: The stratification lines shown represent the approximate boundary between soil types and the transition may be gradual. The groundwater level stated is for conditions at the time of boring and the level may fluctuate large amounts for other conditions or seasons.

Reviewed By:

MOD DEEP BORING LOG W/O NC VALUES & N-E 24-120 COM CIVIC CENTER.GPJ GETI AL GDT 8/16/24

APPENDIX C

LABORATORY TEST RESULTS

Boring Location	Boring No.	Sample ID	Depth (ft)	Water Content (%)	Atterberg Limits			% Gravel	% Sand	% Passing 200 (if hydrometer data available)		D ₅₀ (mm)	USCS	AASHTO Class
					LL	PL	PI			% Silt	% Clay			
	B-1	5	6.5	29	23	18	5	0.0	85.0	15.0		0.167	SC-SM	A-2-4 (0)
	B-1	7	14.0	29	29	13	16	0.0	71.2	28.8		0.141	SC	A-2-6 (1)
	B-1	T-1	16.5	51	85	23	62	0.0	44.1	55.9			CH	A-7-6 (30)
	B-1	9	24.0	47	27	19	8	0.0	45.6	54.4			CL	A-4 (2)
	B-1	12	39.0	24	NP	NP	NP	0.0	92.7	7.3		0.224	SP-SM	A-3 (0)
	B-1	15	54.0	22	NP	NP	NP	0.0	94.9	5.1		0.534	SP-SM	A-1-b (0)
	B-1	20	79.0	22	NP	NP	NP	0.0	96.2	3.8		0.549	SP	A-1-b (0)
	B-2	5	6.5	27	25	19	6	0.0	79.7	20.3		0.15	SC-SM	A-2-4 (0)
	B-2	8	19.0	26	28	16	12	0.0	59.8	40.2		0.089	SC	A-6 (1)
	B-2	11	34.0	25	NP	NP	NP	0.0	92.2	7.8		0.192	SP-SM	A-3 (0)
	B-2	13	44.0	21	NP	NP	NP	0.0	95.7	4.3		0.388	SP	A-3 (0)
	B-2	16	59.0	21	NP	NP	NP	0.0	93.8	6.2		0.292	SP-SM	A-3 (0)
	B-2	19	74.0	22	NP	NP	NP	0.0	88.6	11.4		0.177	SP-SM	A-2-4 (0)
	B-3	4	5.0	15				0.0	74.9	25.1		0.145		
	B-3	7	14.0	27	28	16	12	0.0	74.4	25.6		0.145	SC	A-2-6 (0)
	B-3	8	19.0	28	28	12	16	0.0	77.3	22.7		0.156	SC	A-2-6 (0)



SOIL CLASSIFICATION SUMMARY

GET PROJECT NUMBER: 24-120
 PROJECT NAME: CITY OF MOBILE
 CIVIC CENTER
 COUNTY: MOBILE

Boring Location	Boring No.	Sample ID	Depth (ft)	Water Content (%)	Atterberg Limits			% Gravel	% Sand	% Passing 200 (if hydrometer data available)		D ₅₀ (mm)	USCS	AASHTO Class
					LL	PL	PI			% Silt	% Clay			
	B-3	12	39.0	21	NP	NP	NP	0.7	94.3	5.0		0.348	SP-SM	A-3 (0)
	B-3	15	54.0	22	NP	NP	NP	0.0	94.9	5.1		0.534	SP-SM	A-1-b (0)
	B-3	20	79.0	34	NP	NP	NP	0.0	94.5	5.5		0.529	SP-SM	A-1-b (0)
	B-4	5	6.5	28	20	13	7	0.0	73.8	26.2		0.286	SC-SM	A-2-4 (0)
	B-4	7	14.0	28	21	7	14	0.0	72.1	27.9		0.145	SC	A-2-6 (1)
	B-4	8	19.0	97				0.0	12.7	87.3				
	B-4	9	24.0	29										
	B-4	11	34.0	24	NP	NP	NP	0.0	94.7	5.3		0.234	SP-SM	A-3 (0)
	B-4	13	44.0	23	NP	NP	NP	0.0	89.7	10.3		0.171	SP-SM	A-3 (0)
	B-4	16	59.0	13	NP	NP	NP	26.7	64.8	8.5		0.611	SW-SM	A-1-b (0)
	B-5	5	6.5	36	31	21	10	0.0	65.4	34.6		0.199	SC	A-2-4 (0)
	B-5	6	9.0	32	NP	NP	NP	0.0	77.5	22.5		0.327	SM	A-2-4 (0)
	B-5	7	14.0	82										
	B-5	11	34.0	24	NP	NP	NP	0.0	93.6	6.4		0.215	SP-SM	A-3 (0)
	B-5	14	49.0	20	NP	NP	NP	0.4	95.3	4.3		0.341	SP	A-3 (0)
	B-5	18	69.0	19	NP	NP	NP	1.5	91.9	6.6		0.321	SP-SM	A-3 (0)



SOIL CLASSIFICATION SUMMARY

GET PROJECT NUMBER: 24-120
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Boring Location	Boring No.	Sample ID	Depth (ft)	Water Content (%)	Atterberg Limits			% Gravel	% Sand	% Passing 200 (if hydrometer data available)		D ₅₀ (mm)	USCS	AASHTO Class
					LL	PL	PI			% Silt	% Clay			
	B-6	4	5.0	19				0.2	77.2	22.6		0.154		
	B-6	7	14.0	28	22	15	7	0.0	74.8	25.2		0.151	SC-SM	A-2-4 (0)
	B-6	10	29.0	24				0.0	95.8	4.2		0.275	SP	
	B-6	14	49.0	22	NP	NP	NP			3.1		0.59	SP	A-1-b (0)
	B-6	18	69.0	17						4.2		0.631	SP	
	B-7	5	6.5	31				0.0	82.8	17.2		0.153		
	B-7	7	14.0	73										
	B-7	T-1	16.5	41	43	12	31							
	B-7	11	34.0	26	NP	NP	NP	0.0	92.7	7.3		0.186	SP-SM	A-3 (0)
	B-7	14	49.0	22	NP	NP	NP	0.0	94.2	5.8		0.217	SP-SM	A-3 (0)
	B-7	18	69.0	20	NP	NP	NP	0.9	93.3	5.8		0.317	SP-SM	A-3 (0)
	B-8	6	9.0	37	34	21	13	0.0	62.2	37.8		0.097	SC	A-6 (1)
	B-8	8	19.0	105										
	B-8	T-1	21.5	99	157	43	114	0.0	3.6	96.4			CH	A-7-5 (132)
	B-8	9	24.0	54										
	B-8	12	39.0		NP	NP	NP	0.0	94.3	5.7		0.221	SP-SM	A-3 (0)



SOIL CLASSIFICATION SUMMARY

GET PROJECT NUMBER: 24-120
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 CIVIC CENTER
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Boring Location	Boring No.	Sample ID	Depth (ft)	Water Content (%)	Atterberg Limits			% Gravel	% Sand	% Passing 200 (if hydrometer data available)		D ₅₀ (mm)	USCS	AASHTO Class
					LL	PL	PI			% Silt	% Clay			
	B-8	15	54.0	21	NP	NP	NP	0.0	87.5	12.5		0.244	SM	A-2-4 (0)
	B-8	18	69.0	23	NP	NP	NP	0.2	89.0	10.8		0.188	SP-SM	A-2-4 (0)
	B-9	5	6.5	21	NP	NP	NP	0.0	85.1	14.9		0.415	SM	A-1-b (0)
	B-9	8	19.0	26	22	17	5	0.0	76.0	24.0		0.161	SC-SM	A-2-4 (0)
	B-9	10	29.0	29	NP	NP	NP	0.0	94.1	5.9		0.2	SP-SM	A-3 (0)
	B-9	12	39.0	26	NP	NP	NP	0.0	95.7	4.3		0.197	SP	A-3 (0)
	B-9	15	54.0	21	NP	NP	NP	1.8	84.1	14.1		0.177	SM	A-2-4 (0)
	B-9	19	74.0	22	NP	NP	NP	0.0	88.9	11.1		0.189	SP-SM	A-2-4 (0)
	B-10	5	6.5	30	NP	NP	NP	0.0	62.7	37.3		0.104	SM	A-4 (0)
	B-10	7	14.0	28				0.0	76.5	23.5		0.154		
	B-10	9	24.0	24	NP	NP	NP	0.0	96.2	3.8		0.293	SP	A-3 (0)
	B-10	11	34.0	25	NP	NP	NP	0.0	95.3	4.7		0.266	SP	A-3 (0)
	B-10	13	44.0	21	NP	NP	NP	0.0	95.3	4.7		0.377	SP	A-3 (0)
	B-10	15	54.0	22	NP	NP	NP	0.9	92.3	6.8		0.272	SP-SM	A-3 (0)
	B-10	17	64.0	20	NP	NP	NP	0.0	94.1	5.9		0.525	SP-SM	A-1-b (0)
	B-10	22	89.0	31	NP	NP	NP	1.5	92.8	5.7		0.543	SP-SM	A-1-b (0)



SOIL CLASSIFICATION SUMMARY

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 CIVIC CENTER
 COUNTY: MOBILE

Boring Location	Boring No.	Sample ID	Depth (ft)	Water Content (%)	Atterberg Limits			% Gravel	% Sand	% Passing 200 (if hydrometer data available)		D ₅₀ (mm)	USCS	AASHTO Class
					LL	PL	PI			% Silt	% Clay			
	B-11	4	5.0	10	NP	NP	NP	0.3	85.3	14.5		0.163	SM	A-2-4 (0)
	B-11	7	14.0	27	25	17	8	0.0	75.3	24.7		0.135	SC	A-2-4 (0)
	B-11	8	19.0	27	28	17	11	0.2	77.3	22.5		0.163	SC	A-2-6 (0)
	B-11	9	24.0	73										
	B-11	T-1	26.5	120	150	68	82	0.0	33.4	66.6			MH	A-7-5 (61)
	B-11	10	29.0	49										
	B-11	16	59.0	21	NP	NP	NP	0.0	92.6	7.4		0.327	SP-SM	A-3 (0)
	B-11	20	79.0	22	NP	NP	NP	0.0	91.4	8.6		0.29	SP-SM	A-3 (0)
	B-13	8	19.0	27	26	17	9	0.0	77.8	22.2		0.33	SC	A-2-4 (0)
	B-13	9	24.0	71	76	21	55	0.0	11.2	88.8			CH	A-7-6 (54)
	B-13	T-1	26.5	78	78	21	57							
	B-13	12	39.0	19	NP	NP	NP	0.8	87.3	11.9		0.312	SP-SM	A-2-4 (0)
	B-13	15	54.0	22	NP	NP	NP	0.0	84.7	15.3		0.23	SM	A-2-4 (0)
	B-13	19	74.0	19	NP	NP	NP	0.9	88.9	10.2		0.247	SP-SM	A-3 (0)
	B-13	22	89.0	20	NP	NP	NP	0.6	93.7	5.7		0.294	SP-SM	A-3 (0)
	B-14	4	5.0	25	NP	NP	NP	0.0	88.6	11.4		0.173	SP-SM	A-2-4 (0)



SOIL CLASSIFICATION SUMMARY

GET PROJECT NUMBER: 24-120
PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
COUNTY: MOBILE

Boring Location	Boring No.	Sample ID	Depth (ft)	Water Content (%)	Atterberg Limits			% Gravel	% Sand	% Passing 200 (if hydrometer data available)		D ₅₀ (mm)	USCS	AASHTO Class
					LL	PL	PI			% Silt	% Clay			
	B-14	6	9.0	29	NP	NP	NP	0.0	78.5	21.5		0.141	SM	A-2-4 (0)
	B-14	7	14.0	126										
	B-14	T-1	16.5	32	50	17	33							
	B-14	10	29.0	24	NP	NP	NP	0.3	95.0	4.7		0.298	SP	A-3 (0)
	B-14	14	49.0	20	NP	NP	NP	0.0	94.4	5.6		0.242	SP-SM	A-3 (0)
	B-14	17	64.0	20	NP	NP	NP	1.8	91.3	6.9		0.329	SP-SM	A-3 (0)
	B-15	6	9.0	19				0.0	82.3	17.7		0.157		
	B-15	8	19.0	27	NP	NP	NP	0.0	78.6	21.4		0.166	SM	A-2-4 (0)
	B-15	9	24.0	81										
	B-15	T-1	26.5	64	96	35	61	0.0	14.2	85.8			CH	A-7-5 (61)
	B-15	12	39.0	21	NP	NP	NP	0.0	93.8	6.2		0.343	SP-SM	A-3 (0)
	B-15	16	59.0	22	NP	NP	NP	0.0	93.3	6.7		0.22	SP-SM	A-3 (0)
	B-16	5	6.5	24	20	16	4	0.0	75.5	24.5		0.305	SC-SM	A-2-4 (0)
	B-16	7	14.0	156										
	B-16	8	19.0	21				0.0	91.0	9.0		0.195		
	B-16	12	39.0	23	NP	NP	NP	0.0	94.0	6.0		0.296	SP-SM	A-3 (0)



SOIL CLASSIFICATION SUMMARY

GET PROJECT NUMBER: 24-120
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 CIVIC CENTER
 COUNTY: MOBILE

Boring Location	Boring No.	Sample ID	Depth (ft)	Water Content (%)	Atterberg Limits			% Gravel	% Sand	% Passing 200 (if hydrometer data available)		D ₅₀ (mm)	USCS	AASHTO Class
					LL	PL	PI			% Silt	% Clay			
	B-16	15	54.0	22	NP	NP	NP	0.0	96.0	4.0		0.3	SP	A-3 (0)
	B-16	20	79.0	21	NP	NP	NP	0.0	95.1	4.9		0.536	SP	A-1-b (0)
	B-17	5	6.5	28	NP	NP	NP	0.0	78.3	21.7		0.148	SM	A-2-4 (0)
	B-17	7	14.0	29	30	17	13	0.0	66.3	33.7		0.138	SC	A-2-6 (1)
	B-17	10	29.0	22	NP	NP	NP	0.0	93.2	6.8		0.276	SP-SM	A-3 (0)
	B-17	12	39.0	19	NP	NP	NP	1.8	93.1	5.0		0.4	SP-SM	A-3 (0)
	B-17	14	49.0	22	NP	NP	NP	0.0	93.7	6.3		0.258	SP-SM	A-3 (0)
	B-17	16	59.0	19	NP	NP	NP	0.4	93.7	5.9		0.529	SP-SM	A-1-b (0)
	C-1	1	0.3	9	NP	16	NP	0.0	75.2	24.8		0.155	SM	A-2-4 (0)
	C-2	1	0.4	9	NP	NP	NP	0.4	81.6	18.0		0.164	SM	A-2-4 (0)
	C-3	1	0.4	7	NP	14	NP	0.0	78.6	21.4		0.158	SM	A-2-4 (0)
	C-4		0.4	12	NP	14	NP	0.0	81.7	18.3		0.188	SM	A-2-4 (0)
	C-5	1	0.3	12	17	12	5	0.8	76.1	23.1		0.163	SC-SM	A-2-4 (0)
	C-6	1	0.2	0	NP	NP	NP	94.6	3.9	1.5		15.585	GP	A-1-a (0)
	C-7	1	0.3	12	19	16	3	0.0	76.5	23.5		0.187	SM	A-2-4 (0)

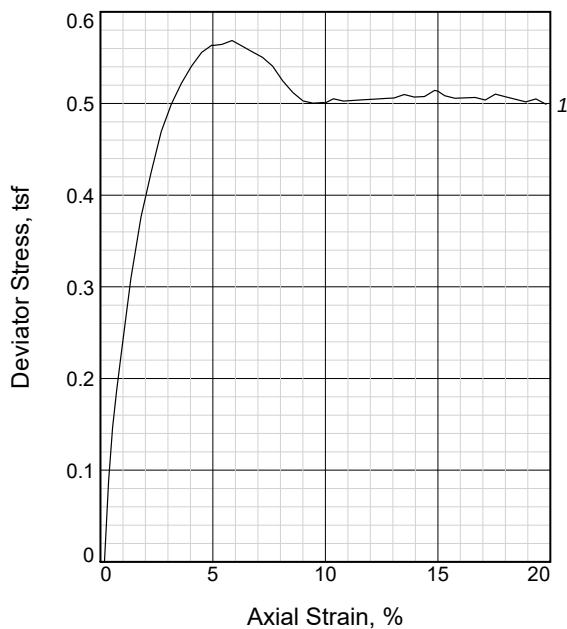
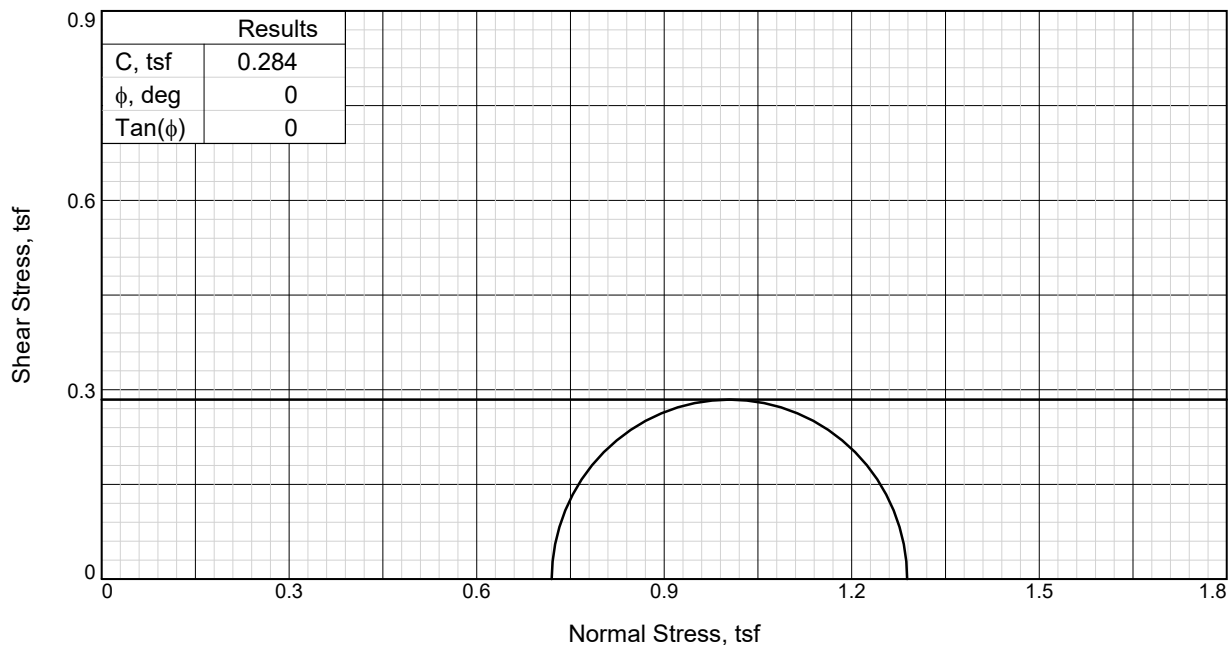


SOIL CLASSIFICATION SUMMARY

GET PROJECT NUMBER: 24-120
 PROJECT NAME: CITY OF MOBILE
 CIVIC CENTER
 COUNTY: MOBILE

**CITY OF MOBILE
CIVIC CENTER
MOBILE COUNTY
TABULATION OF SHEAR STRENGTH TEST RESULTS**

Boring Number/ Sample Number	Sample Depth (Ft)	Sample Elevation (Ft)	USCS CLASS	Moisture Content (%)	Dry Density (pcf)	Undrained Shear Strength				Drained Shear Strength	
						Unconfined Compression (UC, tsf)	Confined Compression (CC, tsf)	Unconsolidated Undrained (UU)		Consolidated Undrained (CU) w/ Pore Pressure	
								Cohesion, tsf	Friction Angle	Effective Cohesion, tsf	Effective Friction Angle
B-01/T-1	16.5	-3		51	72			0.28			
B-08/T-1	21.5	-8	CH	99	46			0.56			
B-11/T-1	26.5	-10	MH	120	41			0.47			
B-13/T-1	26.5	-9		78	56			0.46			
B-14/T-1	16.5	-4		32	89			0.25			
B-15/T-1	26.5	-10		64	60			0.93			



Sample No.		1
Initial	Water Content, %	45.8
	Dry Density, pcf	74.3
	Saturation, %	98.1
	Void Ratio	1.2524
	Diameter, in.	2.76
	Height, in.	5.55
At Test	Water Content, %	46.7
	Dry Density, pcf	74.3
	Saturation, %	100.0
	Void Ratio	1.2524
	Diameter, in.	2.76
	Height, in.	5.55
Strain rate, in./min.		0.06
Back Pressure, psi		0.00
Cell Pressure, psi		10.00
Fail. Stress, tsf		0.57
Strain, %		5.9
Ult. Stress, tsf		
Strain, %		
σ_1 Failure, tsf		1.29
σ_3 Failure, tsf		0.72

Type of Test:

Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: DK GRAY SANDY CLAY

LL= 85 **PL=** 23 **PI=** 62

Assumed Specific Gravity= 2.68

Remarks:

Figure _____

Client:

Project: COM CIVIC CENTER

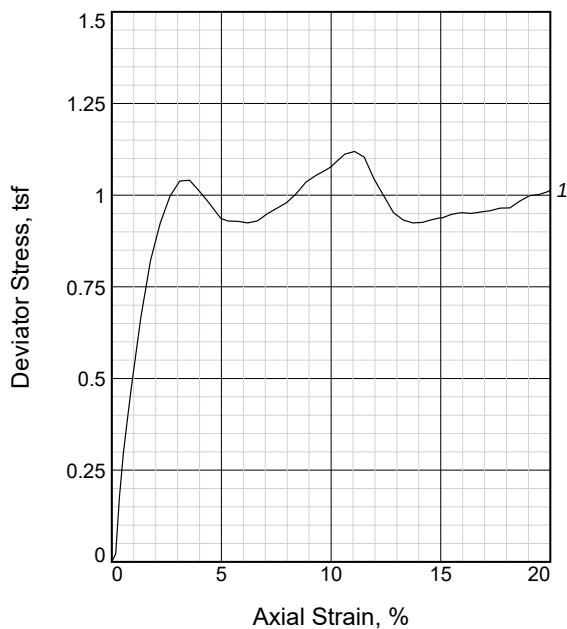
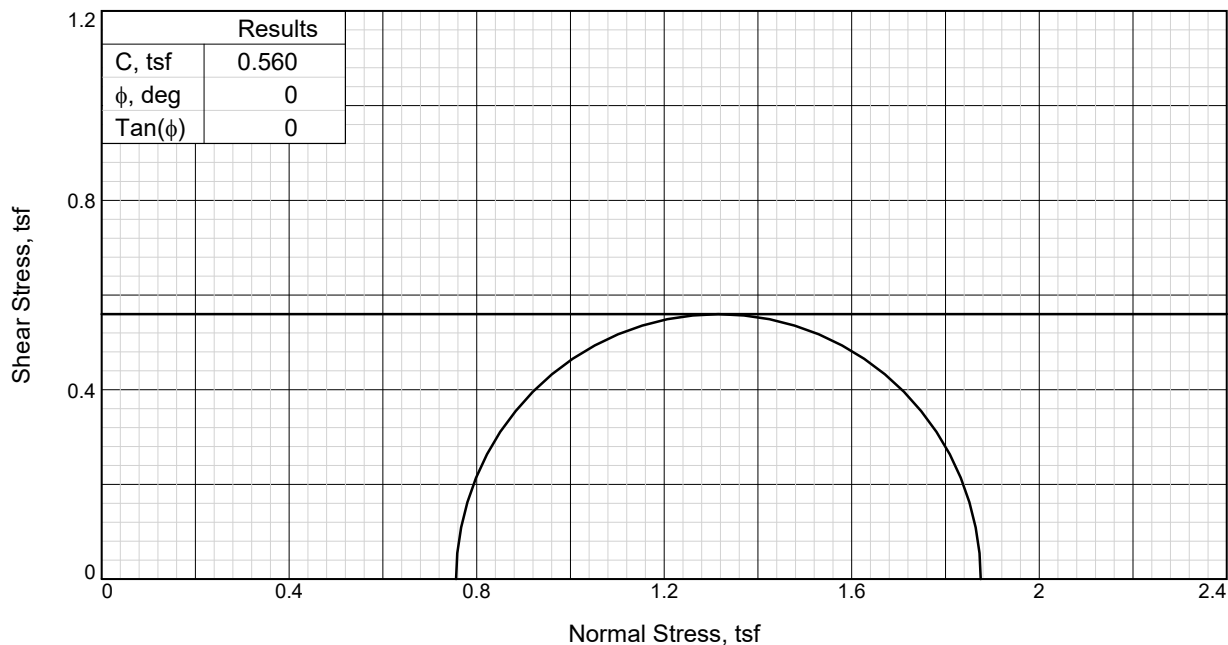
Source of Sample: SHELBY TUBE **Depth:** 16.5-18.5

Sample Number: B-1 T-1

Proj. No.: 24-120

Date Sampled:

TRIAXIAL SHEAR TEST REPORT
 Geotechnical Engineering-Testing, Inc.
 Mobile, AL



Sample No.		1
Initial	Water Content, %	94.7
	Dry Density, pcf	46.8
	Saturation, %	98.5
	Void Ratio	2.5787
	Diameter, in.	2.80
	Height, in.	5.65
At Test	Water Content, %	96.2
	Dry Density, pcf	46.8
	Saturation, %	100.0
	Void Ratio	2.5787
	Diameter, in.	2.80
	Height, in.	5.65
Strain rate, in./min.		0.06
Back Pressure, psi		0.00
Cell Pressure, psi		10.50
Fail. Stress, tsf		1.12
Strain, %		11.1
Ult. Stress, tsf		
Strain, %		
σ_1 Failure, tsf		1.88
σ_3 Failure, tsf		0.76

Type of Test:

Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: DK GRAY CLAY W/ORGANICS

LL= 157 **PL=** 43 **PI=** 114

Assumed Specific Gravity= 2.68

Remarks:

Figure _____

Client:

Project: COM CIVIC CENTER

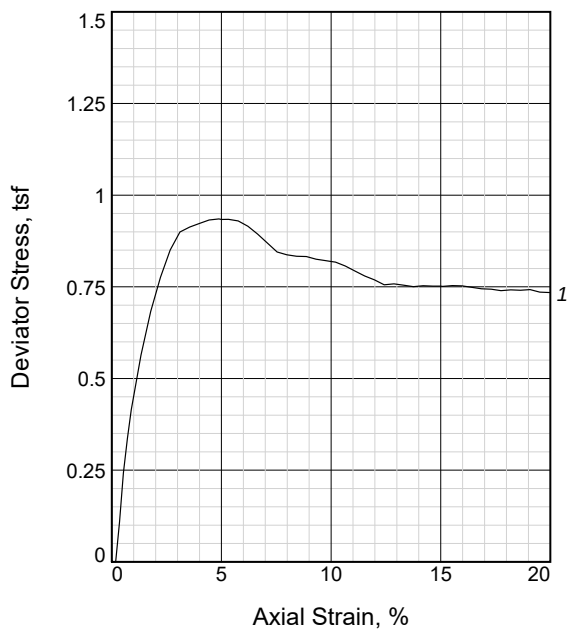
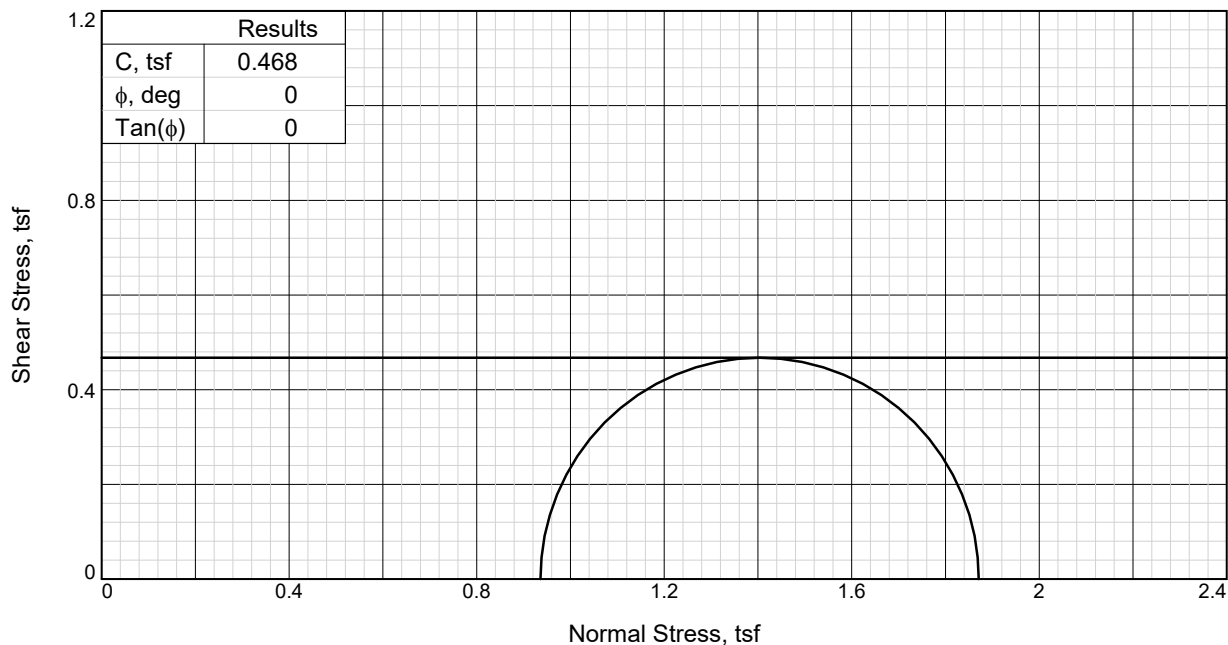
Source of Sample: SHELBY TUBE **Depth:** 21.5-23.5

Sample Number: B-8 T-1

Proj. No.: 24-120

Date Sampled:

TRIAXIAL SHEAR TEST REPORT
 Geotechnical Engineering-Testing, Inc.
 Mobile, AL



Sample No.		1
Initial	Water Content, %	99.3
	Dry Density, pcf	45.0
	Saturation, %	98.0
	Void Ratio	2.7161
	Diameter, in.	2.78
	Height, in.	5.63
At Test	Water Content, %	101.3
	Dry Density, pcf	45.0
	Saturation, %	100.0
	Void Ratio	2.7161
	Diameter, in.	2.78
	Height, in.	5.63
Strain rate, in./min.		0.06
Back Pressure, psi		0.00
Cell Pressure, psi		13.00
Fail. Stress, tsf		0.94
	Strain, %	4.9
Ult. Stress, tsf		
	Strain, %	
σ_1 Failure, tsf		1.87
σ_3 Failure, tsf		0.94

Type of Test:

Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: DK GRAY CLAY W/ORGANICS

LL= 150 **PL=** 68 **PI=** 82

Assumed Specific Gravity= 2.68

Remarks:

Client:

Project: COM CIVIC CENTER

Source of Sample: SHELBY TUBE **Depth:** 26.5-28.5

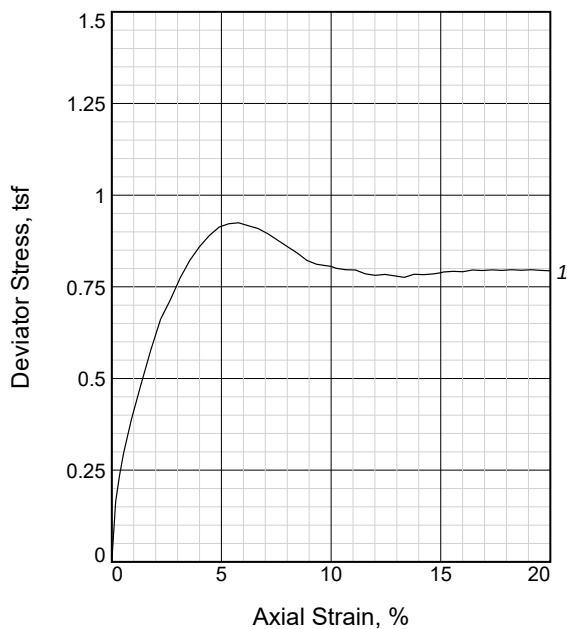
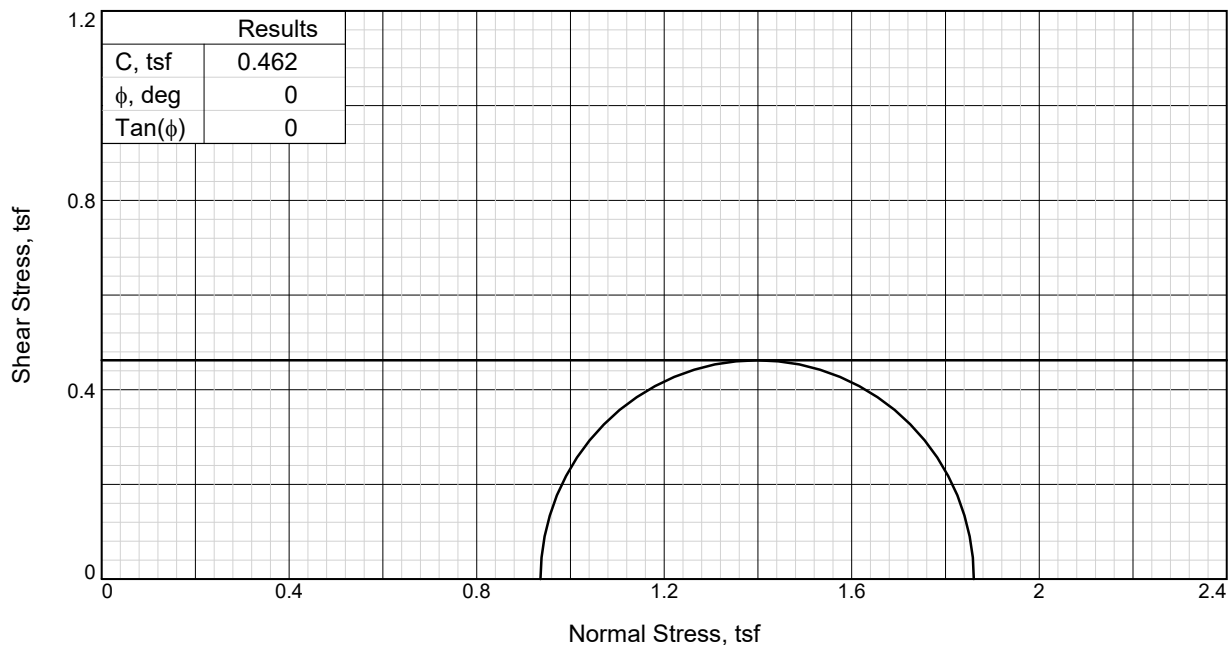
Sample Number: B-11 T-1

Proj. No.: 24-120

Date Sampled:

TRIAXIAL SHEAR TEST REPORT
 Geotechnical Engineering-Testing, Inc.
 Mobile, AL

Figure _____



Sample No.		1
Initial	Water Content, %	69.8
	Dry Density, pcf	58.4
	Saturation, %	100.0
	Void Ratio	1.8841
	Diameter, in.	2.80
	Height, in.	5.62
At Test	Water Content, %	69.8
	Dry Density, pcf	58.4
	Saturation, %	100.0
	Void Ratio	1.8841
	Diameter, in.	2.80
	Height, in.	5.62
Strain rate, in./min.		0.06
Back Pressure, psi		0.00
Cell Pressure, psi		13.00
Fail. Stress, tsf		0.92
Strain, %		5.8
Ult. Stress, tsf		
Strain, %		
σ_1 Failure, tsf		1.86
σ_3 Failure, tsf		0.94

Type of Test:

Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: DK GRAY/BRN CLAY W/
ORGANICS

Assumed Specific Gravity= 2.70

Remarks:

Client:

Project: COM CIVIC CENTER

Source of Sample: SHELBY TUBE **Depth:** 26.5-28.5

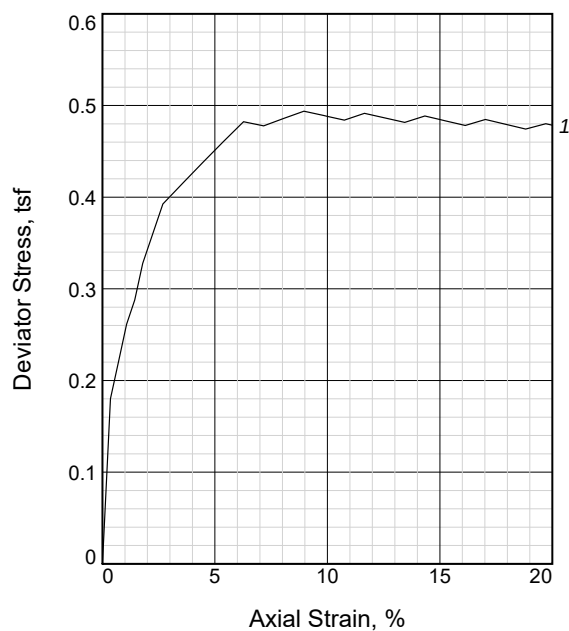
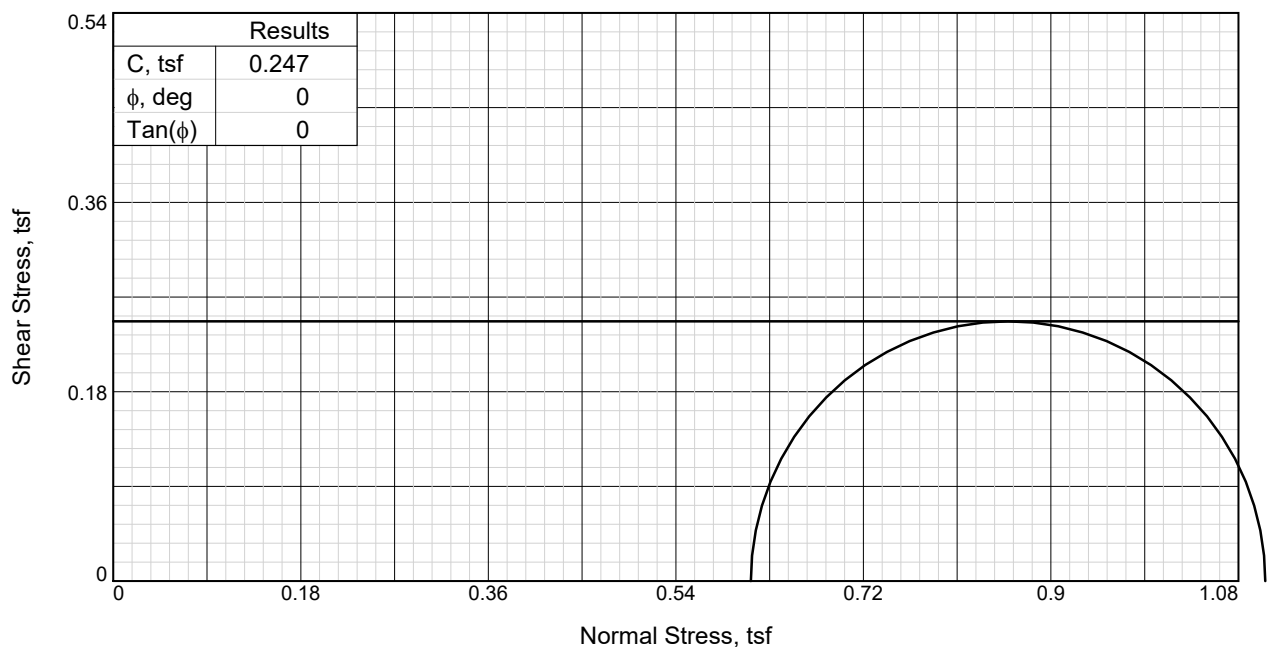
Sample Number: B-13 T-1

Proj. No.: 24-120

Date Sampled:

TRIAXIAL SHEAR TEST REPORT
Geotechnical Engineering-Testing, Inc.
Mobile, AL

Figure _____



Sample No. 1	
Initial	Water Content, % 34.1
	Dry Density, pcf 87.7
	Saturation, % 99.7
	Void Ratio 0.9225
	Diameter, in. 1.42
	Height, in. 2.79
At Test	Water Content, % 34.2
	Dry Density, pcf 87.7
	Saturation, % 100.0
	Void Ratio 0.9225
	Diameter, in. 1.42
	Height, in. 2.79
Strain rate, in./min. 0.03	
Back Pressure, psi 0.00	
Cell Pressure, psi 8.50	
Fail. Stress, tsf 0.49	
Strain, % 9.0	
Ult. Stress, tsf	
Strain, %	
σ_1 Failure, tsf 1.11	
σ_3 Failure, tsf 0.61	

Type of Test:

Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: LT GRAY CLAY

LL= 50 PL= 17 PI= 33

Assumed Specific Gravity= 2.70

Remarks:

Figure _____

Client:

Project: COM CIVIC CENTER

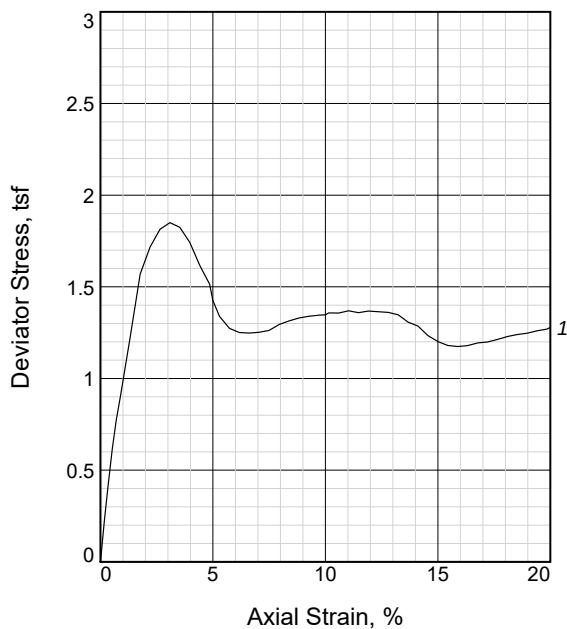
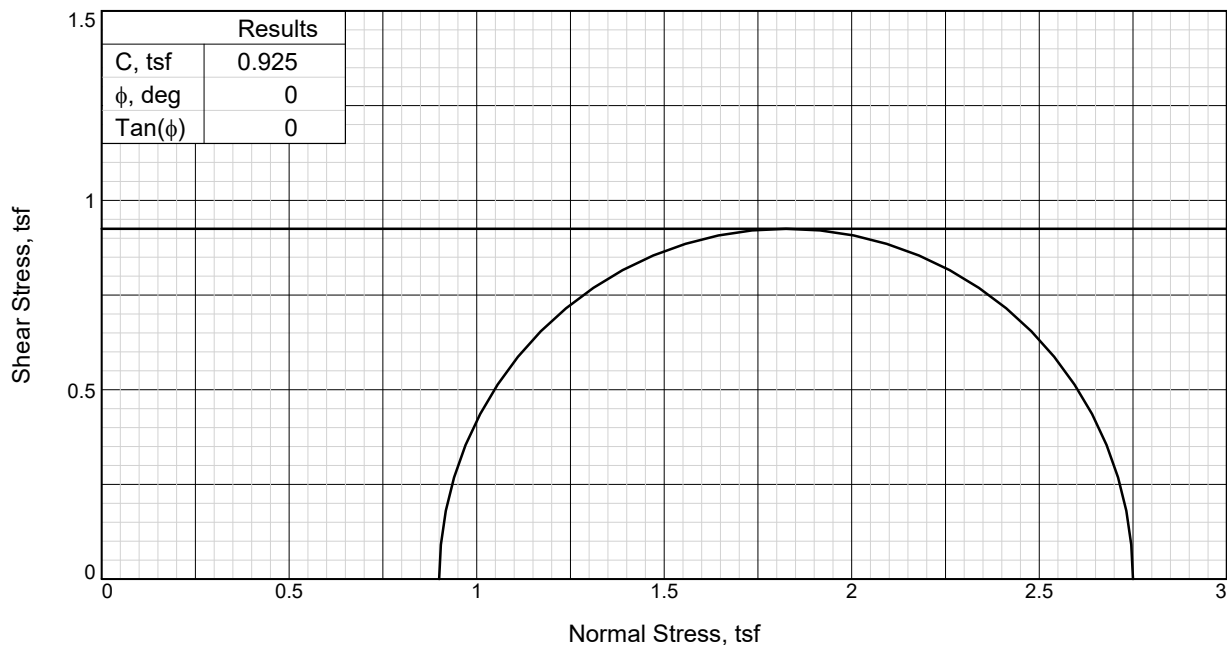
Source of Sample: SHELBY TUBE **Depth:** 16.5-18.5

Sample Number: B-14 T-1

Proj. No.: 24-120

Date Sampled:

TRIAXIAL SHEAR TEST REPORT
 Geotechnical Engineering-Testing, Inc.
 Mobile, AL



Sample No.		1
Initial	Water Content, %	71.6
	Dry Density, pcf	57.0
	Saturation, %	99.1
	Void Ratio	1.9366
	Diameter, in.	2.84
	Height, in.	5.67
At Test	Water Content, %	72.3
	Dry Density, pcf	57.0
	Saturation, %	100.0
	Void Ratio	1.9366
	Diameter, in.	2.84
	Height, in.	5.67
Strain rate, in./min.		0.06
Back Pressure, psi		0.00
Cell Pressure, psi		12.50
Fail. Stress, tsf		1.85
Strain, %		3.1
Ult. Stress, tsf		
Strain, %		
σ_1 Failure, tsf		2.75
σ_3 Failure, tsf		0.90

Type of Test:

Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: DK GRAY/BRN CLAY W/
ORGANICS

Assumed Specific Gravity= 2.68

Remarks: TYPE II-B SHEAR FAILURE

Client:

Project: COM CIVIC CENTER

Source of Sample: SHELBY TUBE **Depth:** 26.5-28.5

Sample Number: B-15 T-1

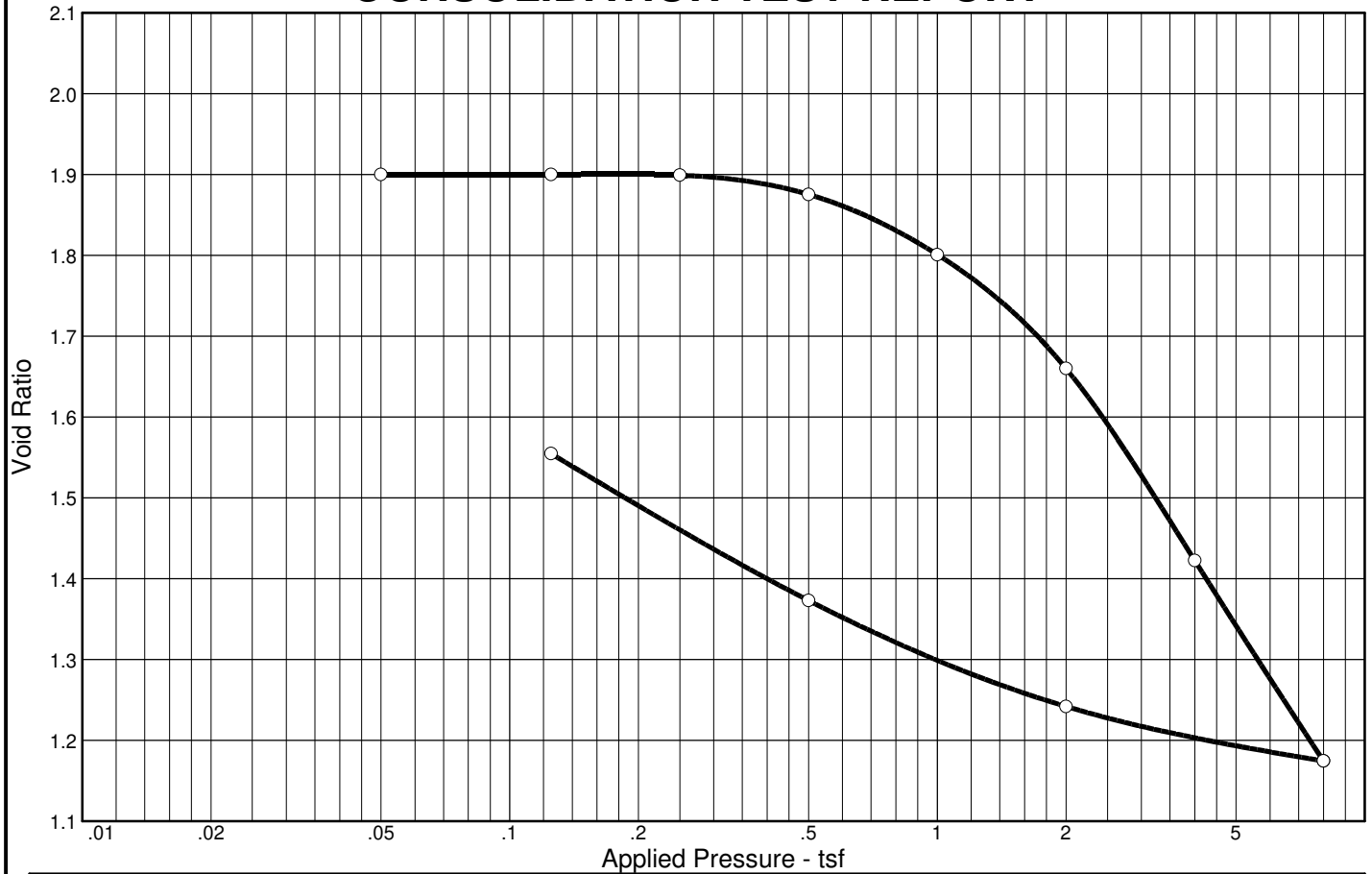
Proj. No.: 24-120

Date Sampled:

TRIAXIAL SHEAR TEST REPORT
Geotechnical Engineering-Testing, Inc.
Mobile, AL

Figure _____

CONSOLIDATION TEST REPORT



Coefficients of Consolidation and Secondary Consolidation

No.	Load (tsf)	C_v (ft.2/day)	C_α	No.	Load (tsf)	C_v (ft.2/day)	C_α	No.	Load (tsf)	C_v (ft.2/day)	C_α
3	0.25	0.07	0.000								
4	0.50	0.20	0.001								

Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	Overburden (tsf)	P_c (tsf)	Initial Void Ratio
Saturation	Moisture							
84.7 %	60.1 %	57.7	85	62	2.68		1.86	1.900

MATERIAL DESCRIPTION	USCS	AASHTO
DK GRAY SANDY CLAY		

Project No. 24-120

Client:

Project: COM CIVIC CENTER

Source: SHELBY TUBE

Sample No.: B-1 T-1

Elev./Depth: 16.5-18.5

Geotechnical Engineering-Testing, Inc.

Mobile, AL

Remarks:

Figure

Dial Reading vs. Time

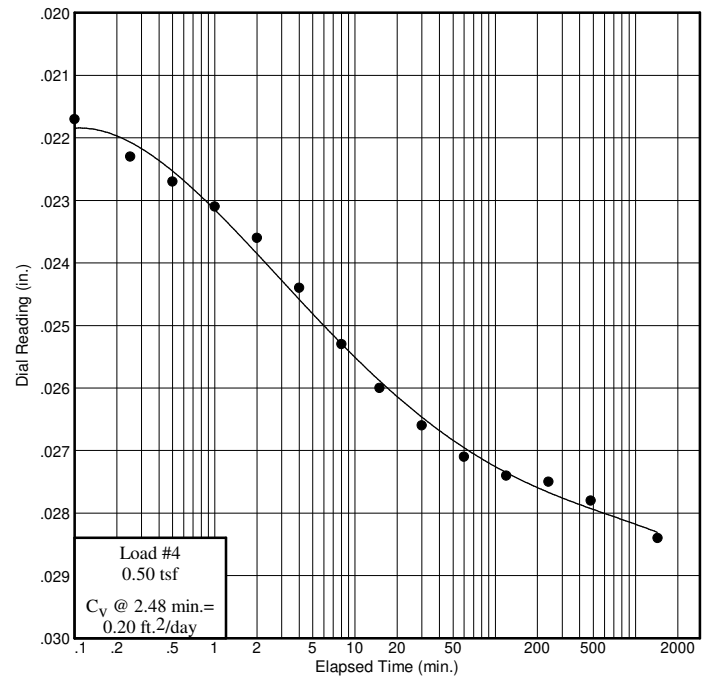
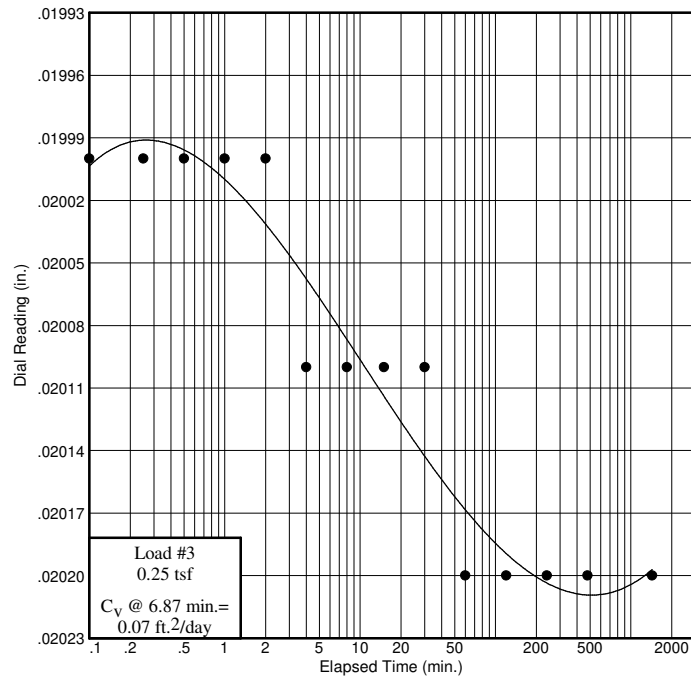
Project No.: 24-120

Project: COM CIVIC CENTER

Source: SHELBY TUBE

Sample No.: B-1 T-1

Elev./Depth: 16.5-18.5



Geotechnical Engineering-Testing, Inc.
Mobile, AL

Figure

Dial Reading vs. Time

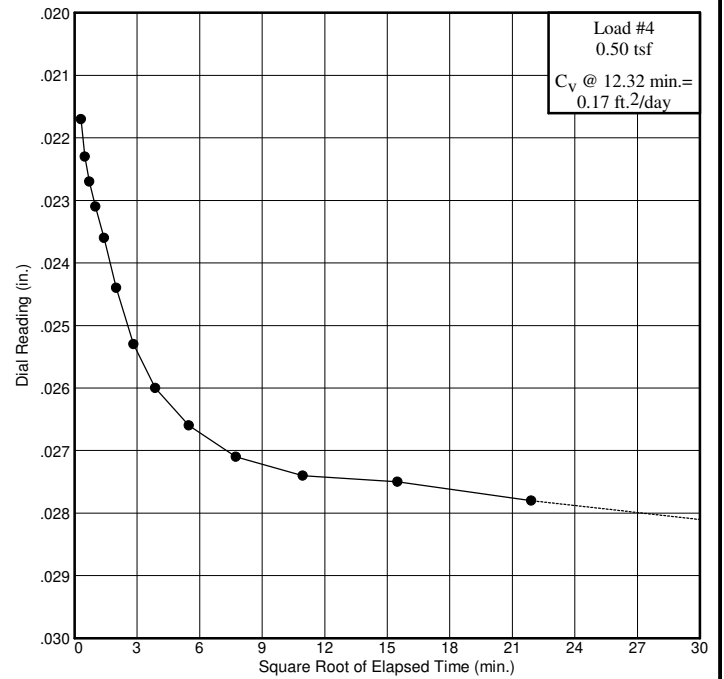
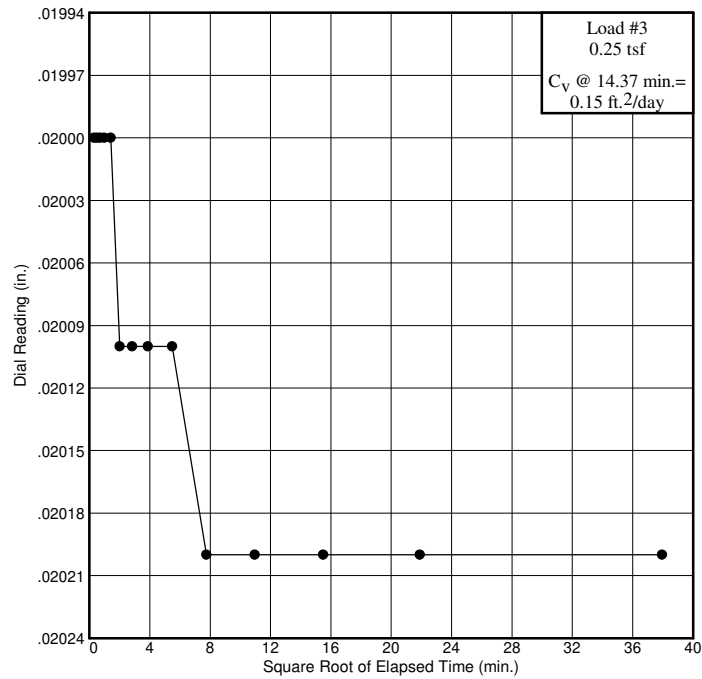
Project No.: 24-120

Project: COM CIVIC CENTER

Source: SHELBY TUBE

Sample No.: B-1 T-1

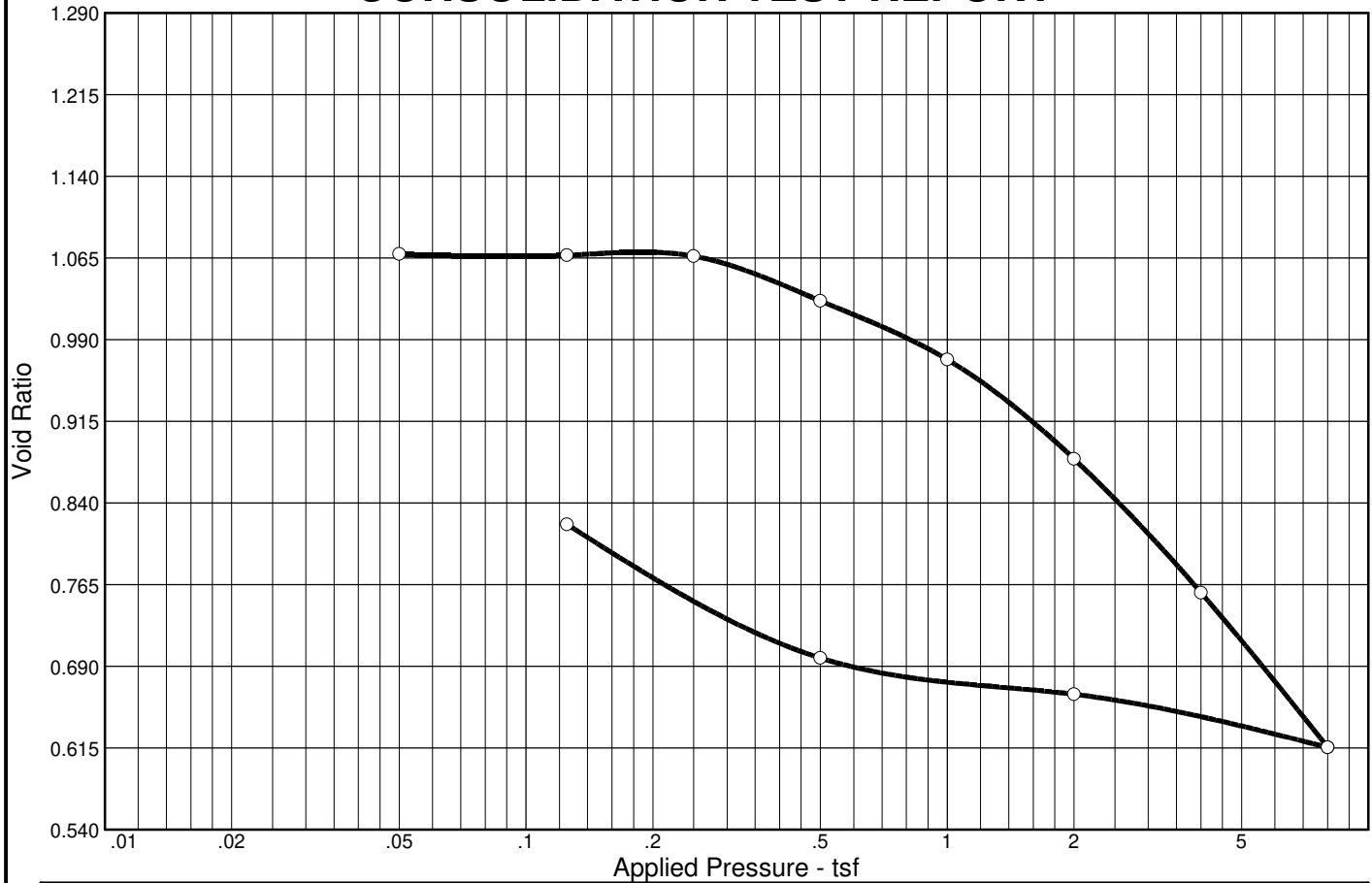
Elev./Depth: 16.5-18.5



Geotechnical Engineering-Testing, Inc.
Mobile, AL

Figure

CONSOLIDATION TEST REPORT



Coefficients of Consolidation and Secondary Consolidation

No.	Load (tsf)	C_v (ft. ² /day)	C_α	No.	Load (tsf)	C_v (ft. ² /day)	C_α	No.	Load (tsf)	C_v (ft. ² /day)	C_α
3	0.25	0.06	0.000								
4	0.50	0.14	0.002								

Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	Overburden (tsf)	P_c (tsf)	Initial Void Ratio
Saturation	Moisture							
102.1 %	40.7 %	80.9	43	31	2.68		1.00	1.069

MATERIAL DESCRIPTION							USCS	AASHTO
GRAY CLAY								

Project No. 24-120 Client:			Remarks:
Project: COM CIVIC CENTER			
Source: SHELBY TUBE	Sample No.: B-7 T-1	Elev./Depth: 16.5-18.5	
Geotechnical Engineering-Testing, Inc. Mobile, AL			
			Figure

Dial Reading vs. Time

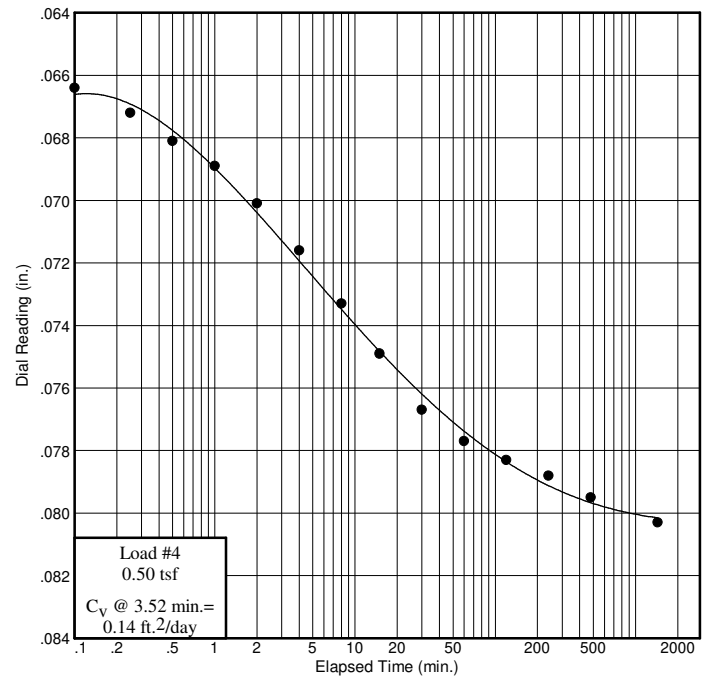
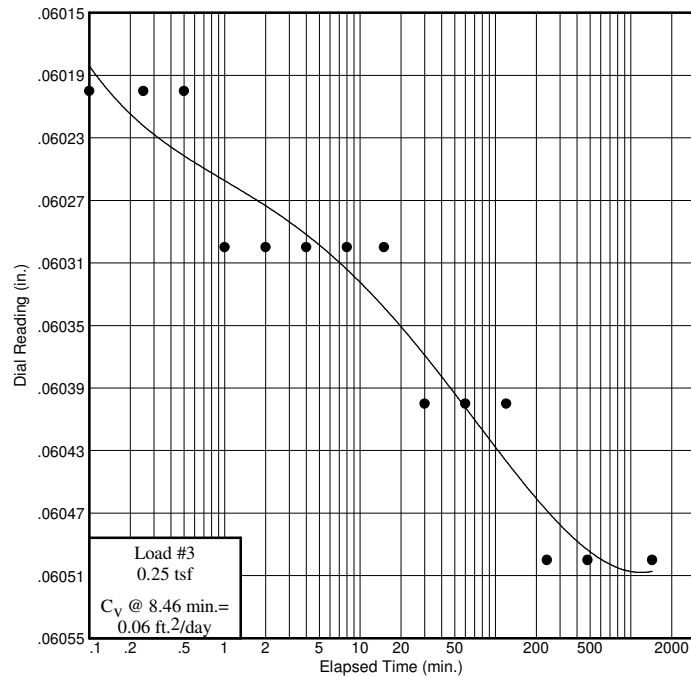
Project No.: 24-120

Project: COM CIVIC CENTER

Source: SHELBY TUBE

Sample No.: B-7 T-1

Elev./Depth: 16.5-18.5



Dial Reading vs. Time

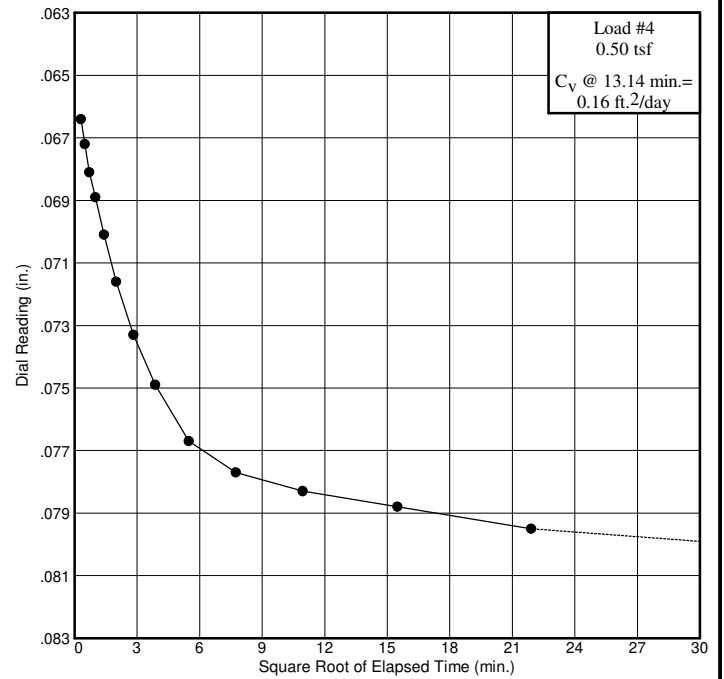
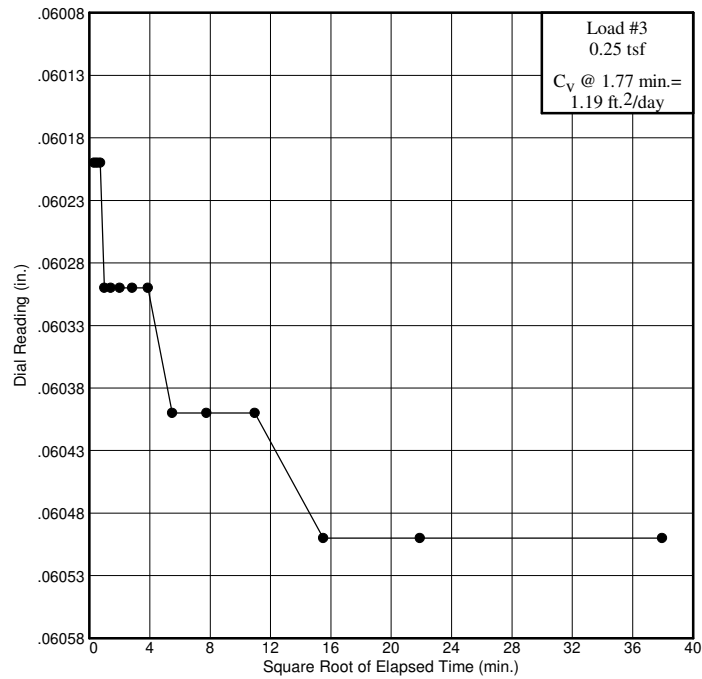
Project No.: 24-120

Project: COM CIVIC CENTER

Source: SHELBY TUBE

Sample No.: B-7 T-1

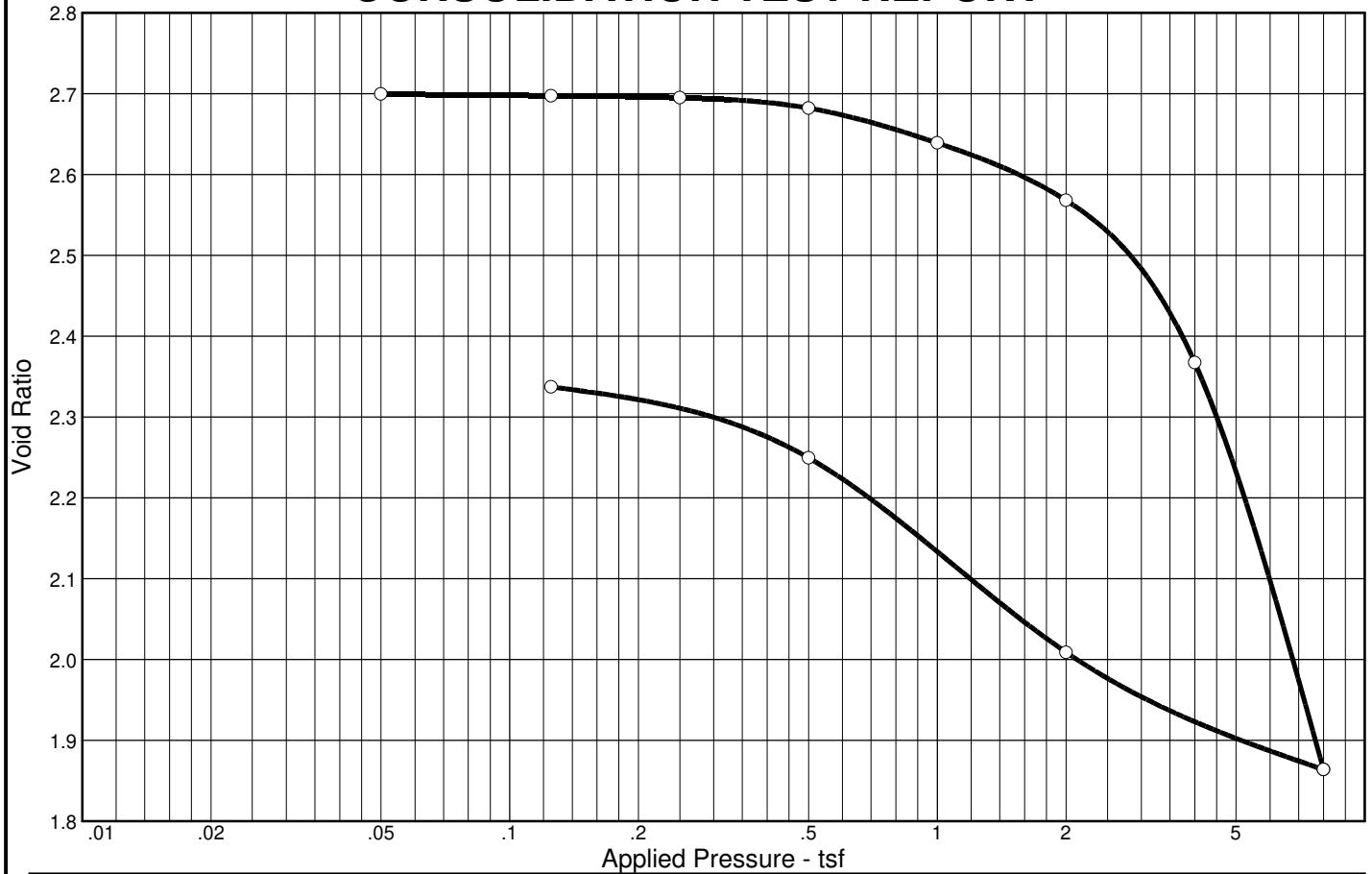
Elev./Depth: 16.5-18.5



Geotechnical Engineering-Testing, Inc.
Mobile, AL

Figure

CONSOLIDATION TEST REPORT



Coefficients of Consolidation and Secondary Consolidation

No.	Load (tsf)	C_v (ft.2/day)	C_α	No.	Load (tsf)	C_v (ft.2/day)	C_α	No.	Load (tsf)	C_v (ft.2/day)	C_α
3	0.25	0.05	0.000								
4	0.50	0.71	0.000								

Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	Overburden (tsf)	P_c (tsf)	Initial Void Ratio
Saturation	Moisture							
95.7 %	96.4 %	45.2	157	114	2.68		3.66	2.700

MATERIAL DESCRIPTION

USCS

AASHTO

DK GRAY CLAY W/ORGANICS

Project No. 24-120

Client:

Project: COM CIVIC CENTER

Source: SHELBY TUBE

Sample No.: B-8 T-1

Elev./Depth: 21.5-23.5

Geotechnical Engineering-Testing, Inc.

Mobile, AL

Remarks:

Figure

Dial Reading vs. Time

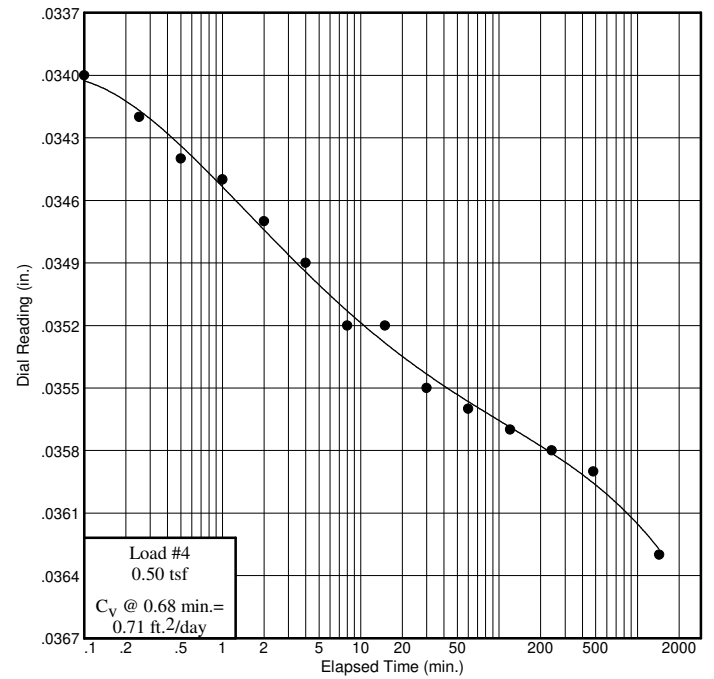
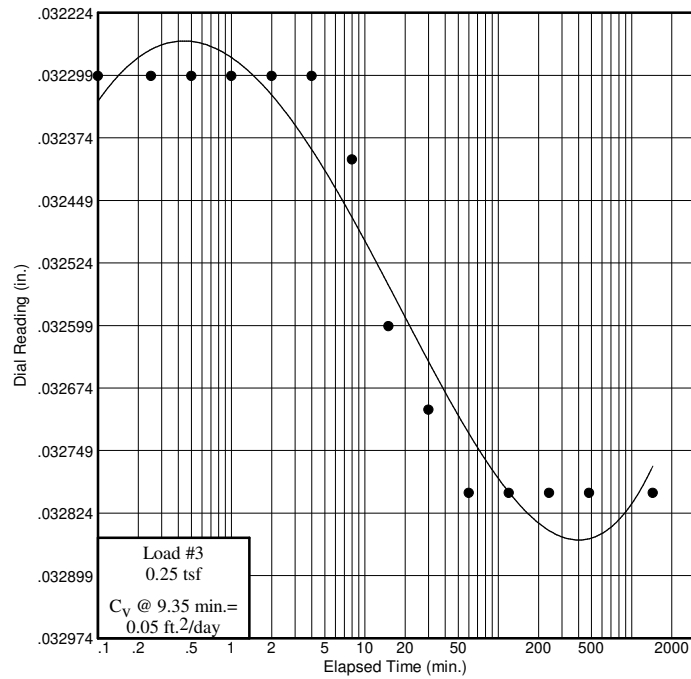
Project No.: 24-120

Project: COM CIVIC CENTER

Source: SHELBY TUBE

Sample No.: B-8 T-1

Elev./Depth: 21.5-23.5



Dial Reading vs. Time

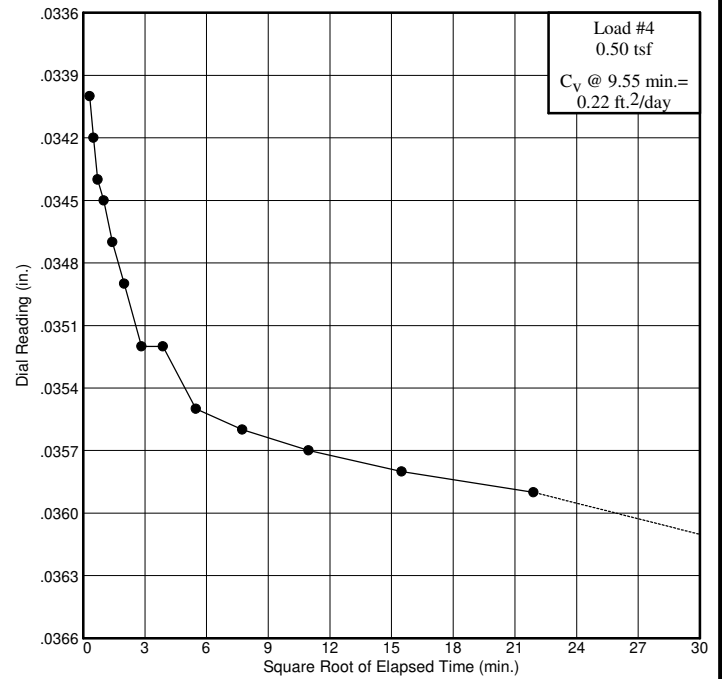
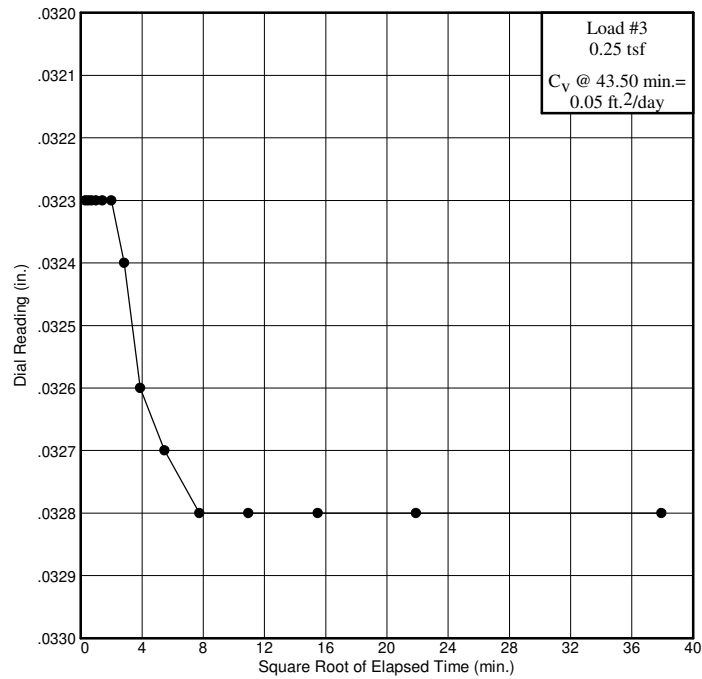
Project No.: 24-120

Project: COM CIVIC CENTER

Source: SHELBY TUBE

Sample No.: B-8 T-1

Elev./Depth: 21.5-23.5

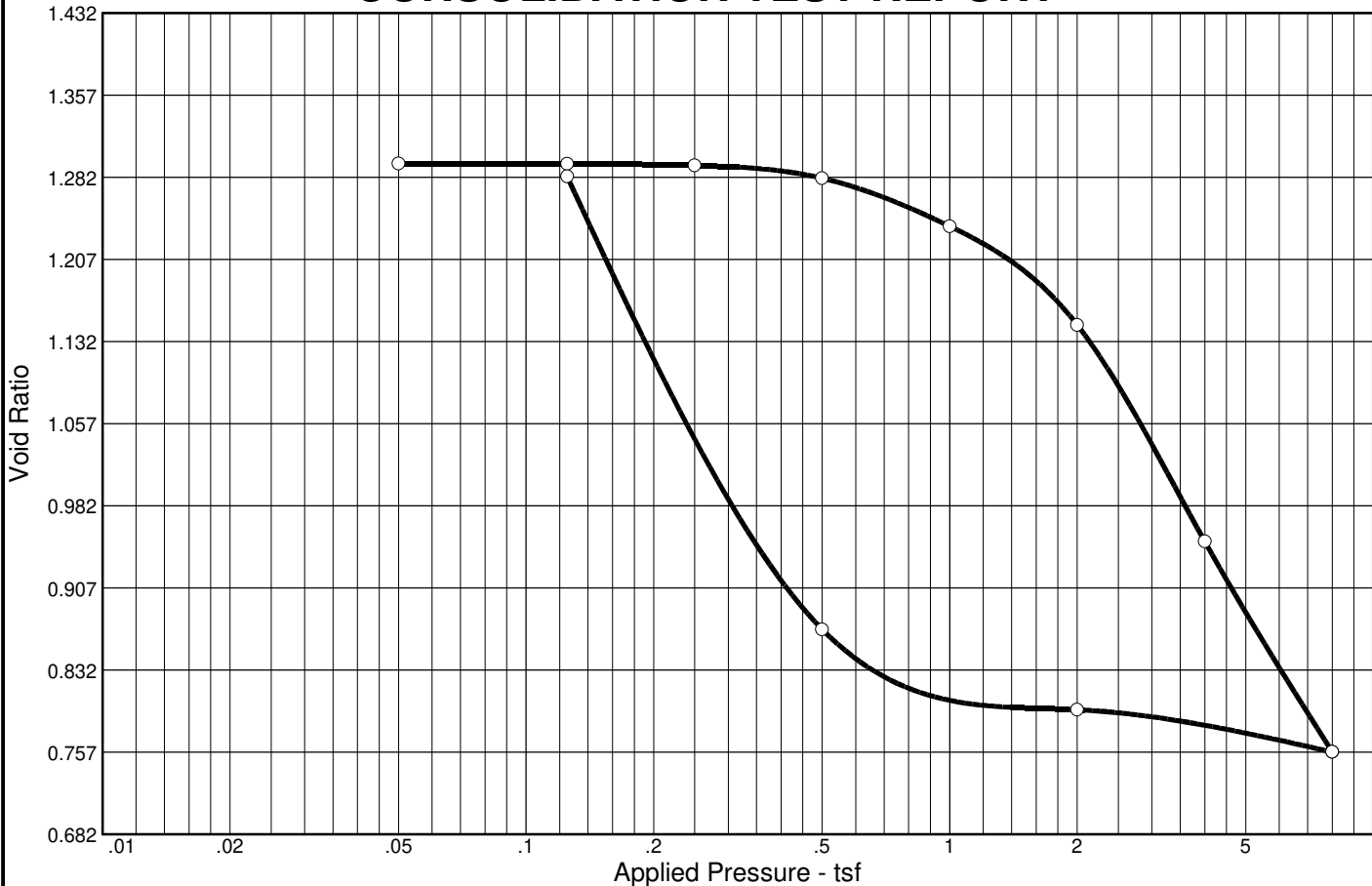


Geotechnical Engineering-Testing, Inc.

Mobile, AL

Figure

CONSOLIDATION TEST REPORT



Coefficients of Consolidation and Secondary Consolidation											
No.	Load (tsf)	C_v (ft.2/day)	C_α	No.	Load (tsf)	C_v (ft.2/day)	C_α	No.	Load (tsf)	C_v (ft.2/day)	C_α
4	0.50	1.18	0.000								

Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	Overburden (tsf)	P _C (tsf)	Initial Void Ratio
Saturation	Moisture							
95.2 %	46.0 %	72.9			2.68		1.71	1.294

MATERIAL DESCRIPTION	USCS	AASHTO
DK GRAY/BRN CLAY W/ ORGANICS		

Project No. 24-120	Client:	Remarks:
Project: COM CIVIC CENTER		
Source: SHELBY TUBE	Sample No.: B-13 T-1 Elev./Depth: 26.5-28.5	
Geotechnical Engineering-Testing, Inc. Mobile, AL		
		Figure

Dial Reading vs. Time

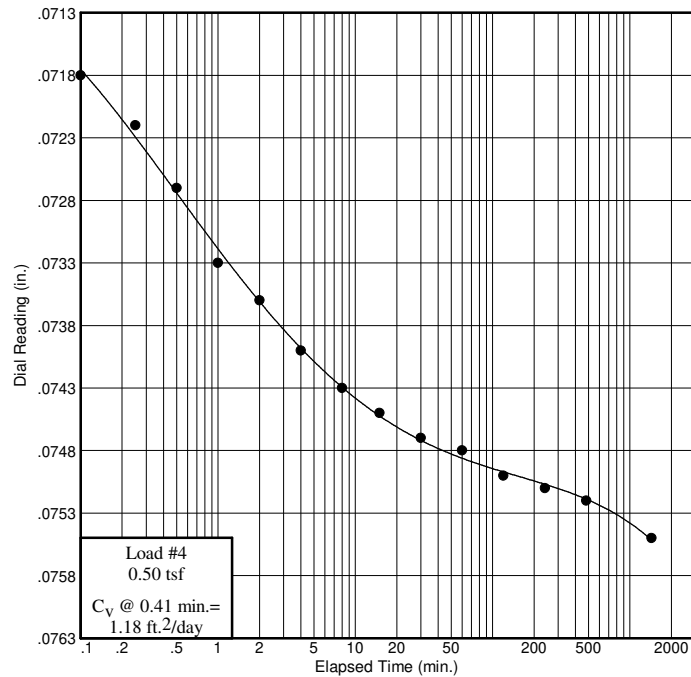
Project No.: 24-120

Project: COM CIVIC CENTER

Source: SHELBY TUBE

Sample No.: B-13 T-1

Elev./Depth: 26.5-28.5



Geotechnical Engineering-Testing, Inc.
Mobile, AL

Figure

Dial Reading vs. Time

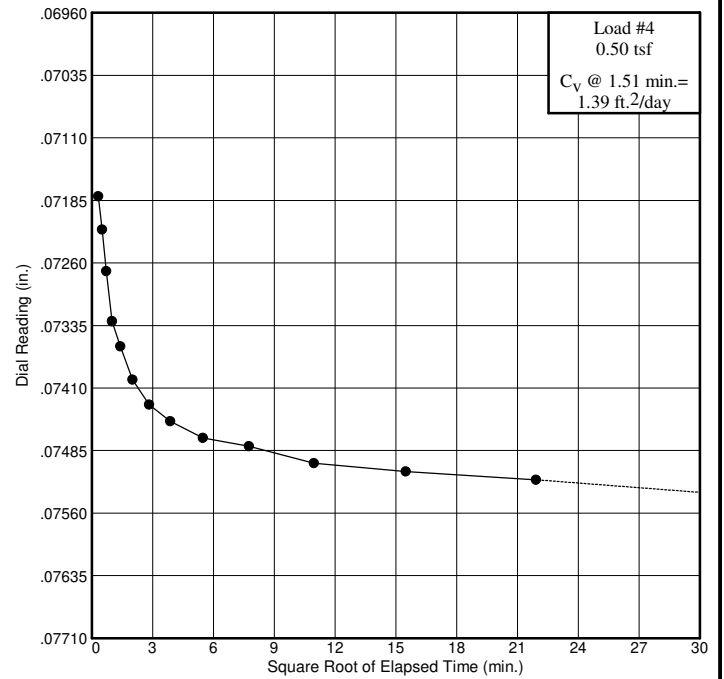
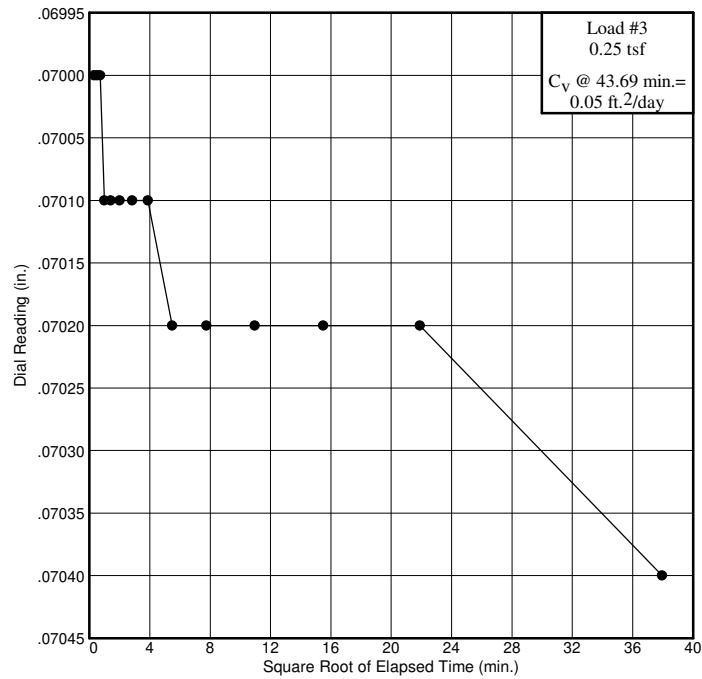
Project No.: 24-120

Project: COM CIVIC CENTER

Source: SHELBY TUBE

Sample No.: B-13 T-1

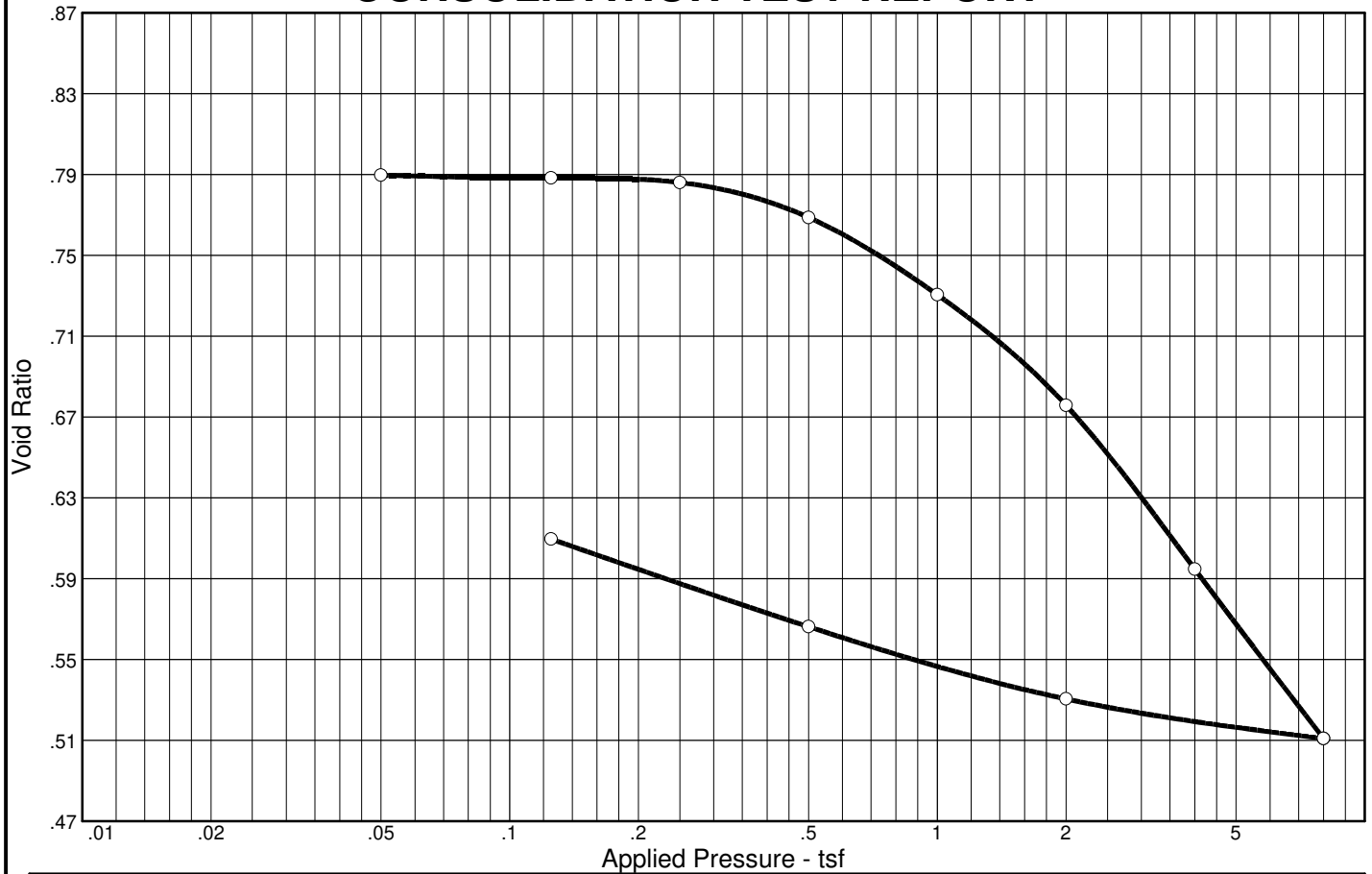
Elev./Depth: 26.5-28.5



Geotechnical Engineering-Testing, Inc.
Mobile, AL

Figure

CONSOLIDATION TEST REPORT



Coefficients of Consolidation and Secondary Consolidation

No.	Load (tsf)	C_v (ft.2/day)	C_α	No.	Load (tsf)	C_v (ft.2/day)	C_α	No.	Load (tsf)	C_v (ft.2/day)	C_α
3	0.25	0.13	0.000								
4	0.50	0.04	0.001								

Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	Overburden (tsf)	P_c (tsf)	Initial Void Ratio
Saturation	Moisture							
104.8 %	31.8 %	92.3	50	33	2.68		1.07	0.813

MATERIAL DESCRIPTION							USCS	AASHTO
LT GRAY CLAY								

Project No. 24-120 Client:			Remarks:
Project: COM CIVIC CENTER			
Source: SHELBY TUBE	Sample No.: B-14 T-1	Elev./Depth: 16.5-18.5	
Geotechnical Engineering-Testing, Inc. Mobile, AL			
			Figure

Figure

Dial Reading vs. Time

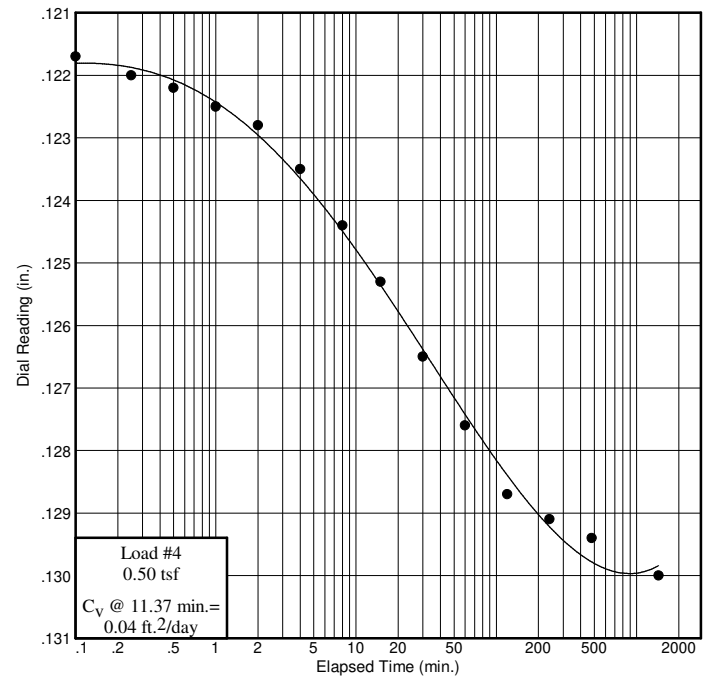
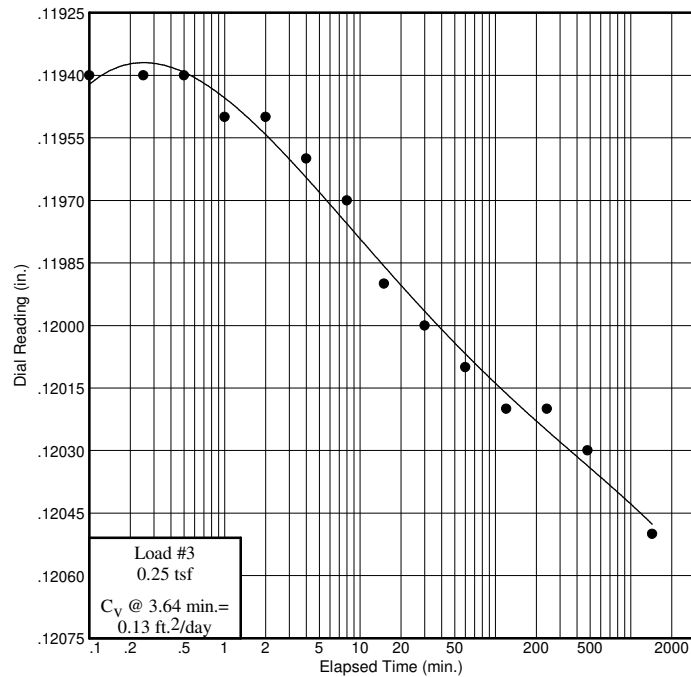
Project No.: 24-120

Project: COM CIVIC CENTER

Source: SHELBY TUBE

Sample No.: B-14 T-1

Elev./Depth: 16.5-18.5



Geotechnical Engineering-Testing, Inc.
Mobile, AL

Figure

Dial Reading vs. Time

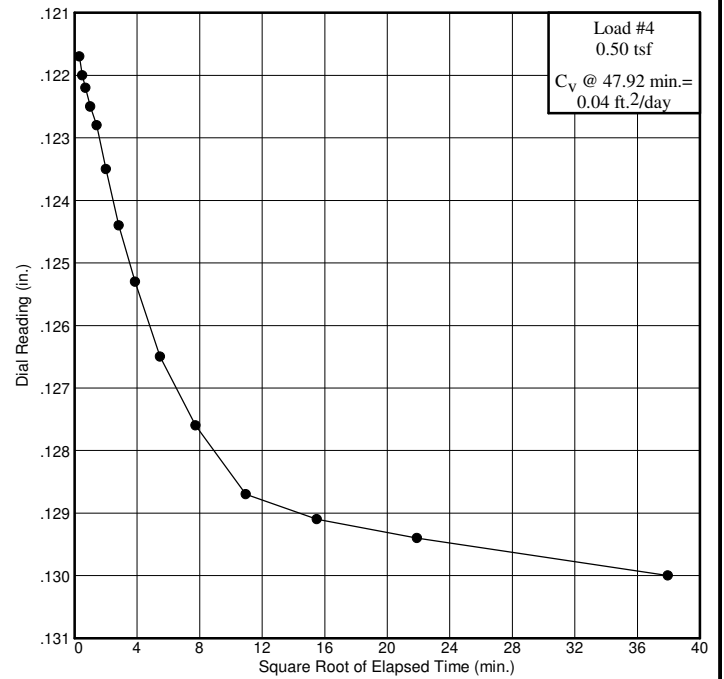
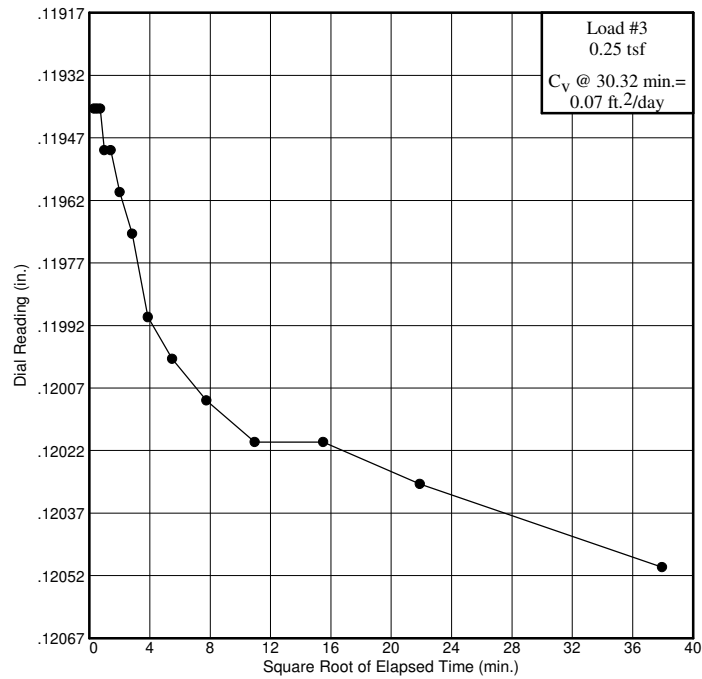
Project No.: 24-120

Project: COM CIVIC CENTER

Source: SHELBY TUBE

Sample No.: B-14 T-1

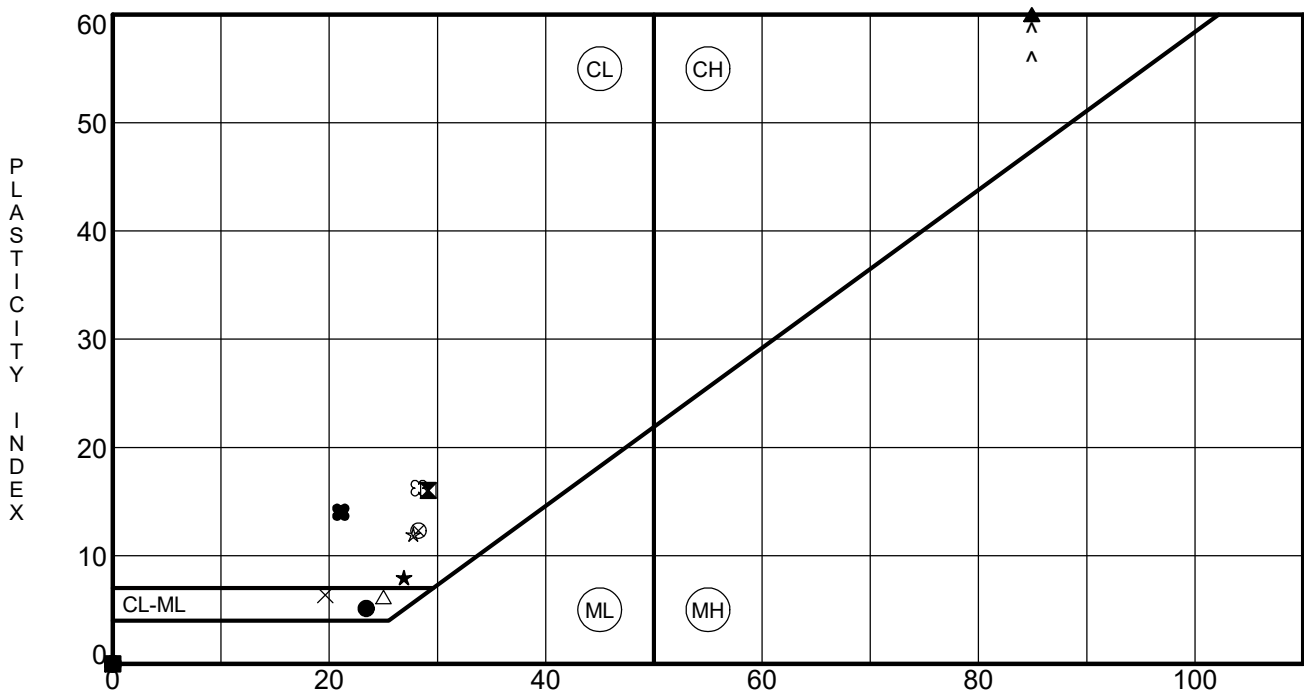
Elev./Depth: 16.5-18.5



Geotechnical Engineering-Testing, Inc.

Mobile, AL

Figure



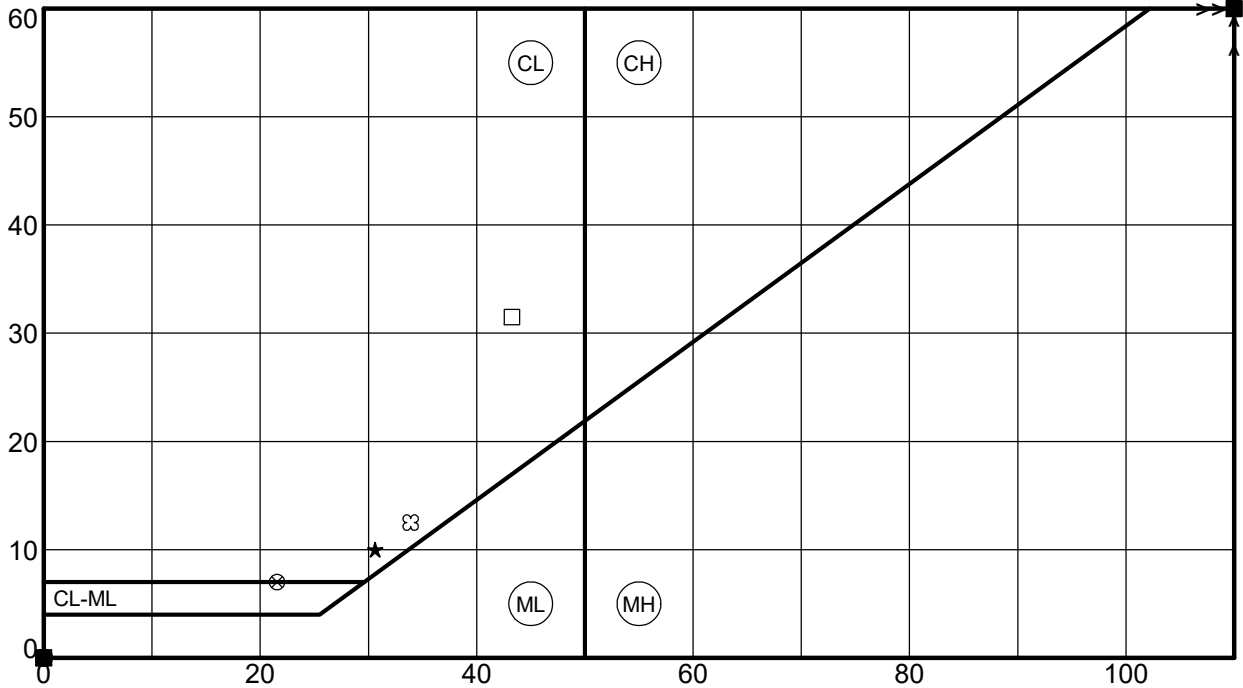
Boring ID	Depth (ft.)	LL	PL	PI	Fines	Classification
● B-1	6.5	23	18	5	15.0	SILTY, CLAYEY SAND (SC-SM)
⊗ B-1	14.0	29	13	16	28.8	CLAYEY SAND (SC)
▲ B-1	16.5	85	23	62	55.9	SANDY FAT CLAY (CH)
★ B-1	24.0	27	19	8	54.4	SANDY LEAN CLAY (CL)
⊙ B-1	39.0	NP	NP	NP	7.3	POORLY GRADED SAND with SILT (SP-SM)
⊕ B-1	54.0	NP	NP	NP	5.1	POORLY GRADED SAND with SILT (SP-SM)
○ B-1	79.0	NP	NP	NP	3.8	POORLY GRADED SAND (SP)
△ B-2	6.5	25	19	6	20.3	SILTY, CLAYEY SAND (SC-SM)
⊗ B-2	19.0	28	16	12	40.2	CLAYEY SAND (SC)
⊕ B-2	34.0	NP	NP	NP	7.8	POORLY GRADED SAND with SILT (SP-SM)
□ B-2	44.0	NP	NP	NP	4.3	POORLY GRADED SAND (SP)
⊕ B-2	59.0	NP	NP	NP	6.2	POORLY GRADED SAND with SILT (SP-SM)
⊕ B-2	74.0	NP	NP	NP	11.4	POORLY GRADED SAND with SILT (SP-SM)
★ B-3	14.0	28	16	12	25.6	CLAYEY SAND (SC)
⊗ B-3	19.0	28	12	16	22.7	CLAYEY SAND (SC)
■ B-3	39.0	NP	NP	NP	5.0	POORLY GRADED SAND with SILT (SP-SM)
◆ B-3	54.0	NP	NP	NP	5.1	POORLY GRADED SAND with SILT (SP-SM)
◇ B-3	79.0	NP	NP	NP	5.5	POORLY GRADED SAND with SILT (SP-SM)
× B-4	6.5	20	13	7	26.2	SILTY, CLAYEY SAND (SC-SM)
⊗ B-4	14.0	21	7	14	27.9	CLAYEY SAND (SC)



ATTERBERG LIMITS RESULTS

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL

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Test Method: _____ LIQUID LIMIT

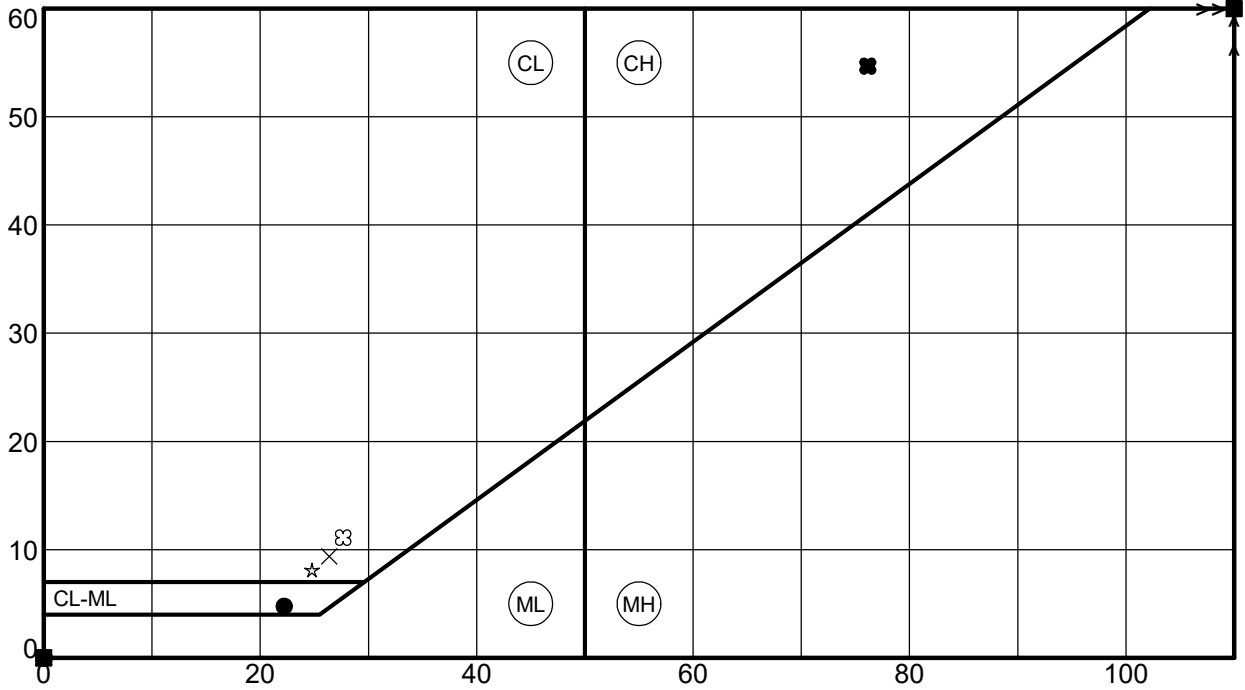
Boring ID	Depth (ft.)	LL	PL	PI	Fines	Classification
● B-4	34.0	NP	NP	NP	5.3	POORLY GRADED SAND with SILT (SP-SM)
⊠ B-4	44.0	NP	NP	NP	10.3	POORLY GRADED SAND with SILT (SP-SM)
▲ B-4	59.0	NP	NP	NP	8.5	WELL-GRADED SAND with SILT and GRAVEL (SW-SM)
★ B-5	6.5	31	21	10	34.6	CLAYEY SAND (SC)
⊙ B-5	9.0	NP	NP	NP	22.5	SILTY SAND (SM)
⊕ B-5	34.0	NP	NP	NP	6.4	POORLY GRADED SAND with SILT (SP-SM)
○ B-5	49.0	NP	NP	NP	4.3	POORLY GRADED SAND (SP)
△ B-5	69.0	NP	NP	NP	6.6	POORLY GRADED SAND with SILT (SP-SM)
⊗ B-6	14.0	22	15	7	25.2	SILTY, CLAYEY SAND (SC-SM)
⊕ B-6	49.0	NP	NP	NP	3.1	POORLY GRADED SAND (SP)
□ B-7	16.5	43	12	31		
⊕ B-7	34.0	NP	NP	NP	7.3	POORLY GRADED SAND with SILT (SP-SM)
⊕ B-7	49.0	NP	NP	NP	5.8	POORLY GRADED SAND with SILT (SP-SM)
★ B-7	69.0	NP	NP	NP	5.8	POORLY GRADED SAND with SILT (SP-SM)
⊗ B-8	9.0	34	21	13	37.8	CLAYEY SAND (SC)
■ B-8	21.5	157	43	114	96.4	FAT CLAY (CH)
◆ B-8	39.0	NP	NP	NP	5.7	POORLY GRADED SAND with SILT (SP-SM)
◇ B-8	54.0	NP	NP	NP	12.5	SILTY SAND (SM)
× B-8	69.0	NP	NP	NP	10.8	POORLY GRADED SAND with SILT (SP-SM)
⬢ B-9	6.5	NP	NP	NP	14.9	SILTY SAND (SM)



ATTERBERG LIMITS RESULTS

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL

PLASTICITY INDEX



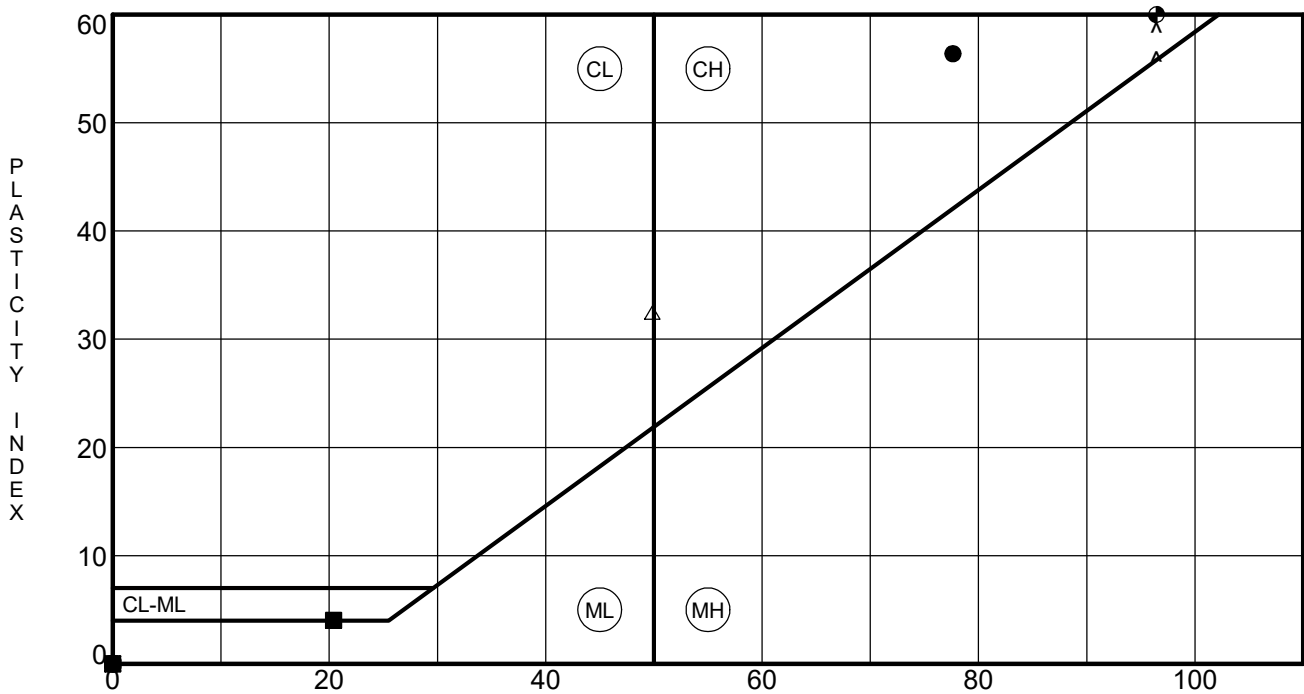
Test Method: _____ LIQUID LIMIT

Boring ID	Depth (ft.)	LL	PL	PI	Fines	Classification
● B-9	19.0	22	17	5	24.0	SILTY, CLAYEY SAND (SC-SM)
✕ B-9	29.0	NP	NP	NP	5.9	POORLY GRADED SAND with SILT (SP-SM)
▲ B-9	39.0	NP	NP	NP	4.3	POORLY GRADED SAND (SP)
★ B-9	54.0	NP	NP	NP	14.1	SILTY SAND (SM)
⊙ B-9	74.0	NP	NP	NP	11.1	POORLY GRADED SAND with SILT (SP-SM)
⊕ B-10	6.5	NP	NP	NP	37.3	SILTY SAND (SM)
○ B-10	24.0	NP	NP	NP	3.8	POORLY GRADED SAND (SP)
△ B-10	34.0	NP	NP	NP	4.7	POORLY GRADED SAND (SP)
⊗ B-10	44.0	NP	NP	NP	4.7	POORLY GRADED SAND (SP)
⊕ B-10	54.0	NP	NP	NP	6.8	POORLY GRADED SAND with SILT (SP-SM)
□ B-10	64.0	NP	NP	NP	5.9	POORLY GRADED SAND with SILT (SP-SM)
⊕ B-10	89.0	NP	NP	NP	5.7	POORLY GRADED SAND with SILT (SP-SM)
⊕ B-11	5.0	NP	NP	NP	14.5	SILTY SAND (SM)
★ B-11	14.0	25	17	8	24.7	CLAYEY SAND (SC)
✕ B-11	19.0	28	17	11	22.5	CLAYEY SAND (SC)
■ B-11	26.5	150	68	82	66.6	SANDY ELASTIC SILT (MH)
◆ B-11	59.0	NP	NP	NP	7.4	POORLY GRADED SAND with SILT (SP-SM)
◇ B-11	79.0	NP	NP	NP	8.6	POORLY GRADED SAND with SILT (SP-SM)
✕ B-13	19.0	26	17	9	22.2	CLAYEY SAND (SC)
✱ B-13	24.0	76	21	55	88.8	FAT CLAY (CH)



ATTERBERG LIMITS RESULTS

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



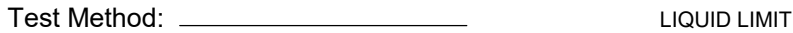
Test Method: _____ LIQUID LIMIT

Boring ID	Depth (ft.)	LL	PL	PI	Fines	Classification
● B-13	26.5	78	21	57		
⊠ B-13	39.0	NP	NP	NP	11.9	POORLY GRADED SAND with SILT (SP-SM)
▲ B-13	54.0	NP	NP	NP	15.3	SILTY SAND (SM)
★ B-13	74.0	NP	NP	NP	10.2	POORLY GRADED SAND with SILT (SP-SM)
⊙ B-13	89.0	NP	NP	NP	5.7	POORLY GRADED SAND with SILT (SP-SM)
⊕ B-14	5.0	NP	NP	NP	11.4	POORLY GRADED SAND with SILT (SP-SM)
○ B-14	9.0	NP	NP	NP	21.5	SILTY SAND (SM)
△ B-14	16.5	50	17	33		
⊗ B-14	29.0	NP	NP	NP	4.7	POORLY GRADED SAND (SP)
⊕ B-14	49.0	NP	NP	NP	5.6	POORLY GRADED SAND with SILT (SP-SM)
□ B-14	64.0	NP	NP	NP	6.9	POORLY GRADED SAND with SILT (SP-SM)
⊕ B-15	19.0	NP	NP	NP	21.4	SILTY SAND (SM)
⊕ B-15	26.5	96	35	61	85.8	FAT CLAY (CH)
★ B-15	39.0	NP	NP	NP	6.2	POORLY GRADED SAND with SILT (SP-SM)
⊗ B-15	59.0	NP	NP	NP	6.7	POORLY GRADED SAND with SILT (SP-SM)
■ B-16	6.5	20	16	4	24.5	SILTY, CLAYEY SAND (SC-SM)
◆ B-16	39.0	NP	NP	NP	6.0	POORLY GRADED SAND with SILT (SP-SM)
◇ B-16	54.0	NP	NP	NP	4.0	POORLY GRADED SAND (SP)
⊗ B-16	79.0	NP	NP	NP	4.9	POORLY GRADED SAND (SP)
⊕ B-17	6.5	NP	NP	NP	21.7	SILTY SAND (SM)

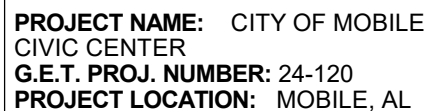


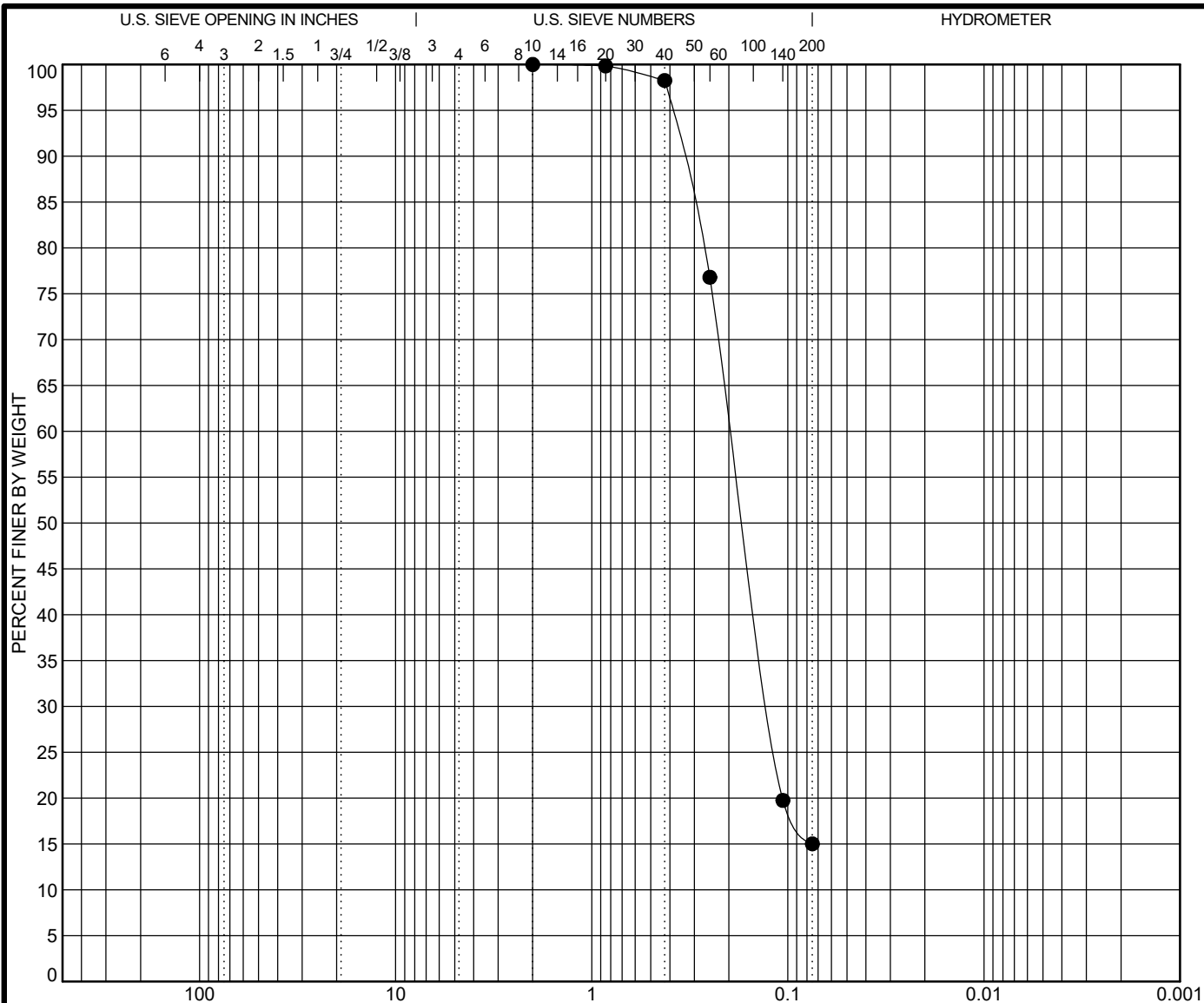
ATTERBERG LIMITS RESULTS

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



US ATTERBERG LIMITS 24-120 COM CIVIC CENTER.GPJ GETI AL.GDT 8/16/24





Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-1 S-5; 6.5 ft.	SILTY, CLAYEY SAND (SC-SM)					23	18	5		

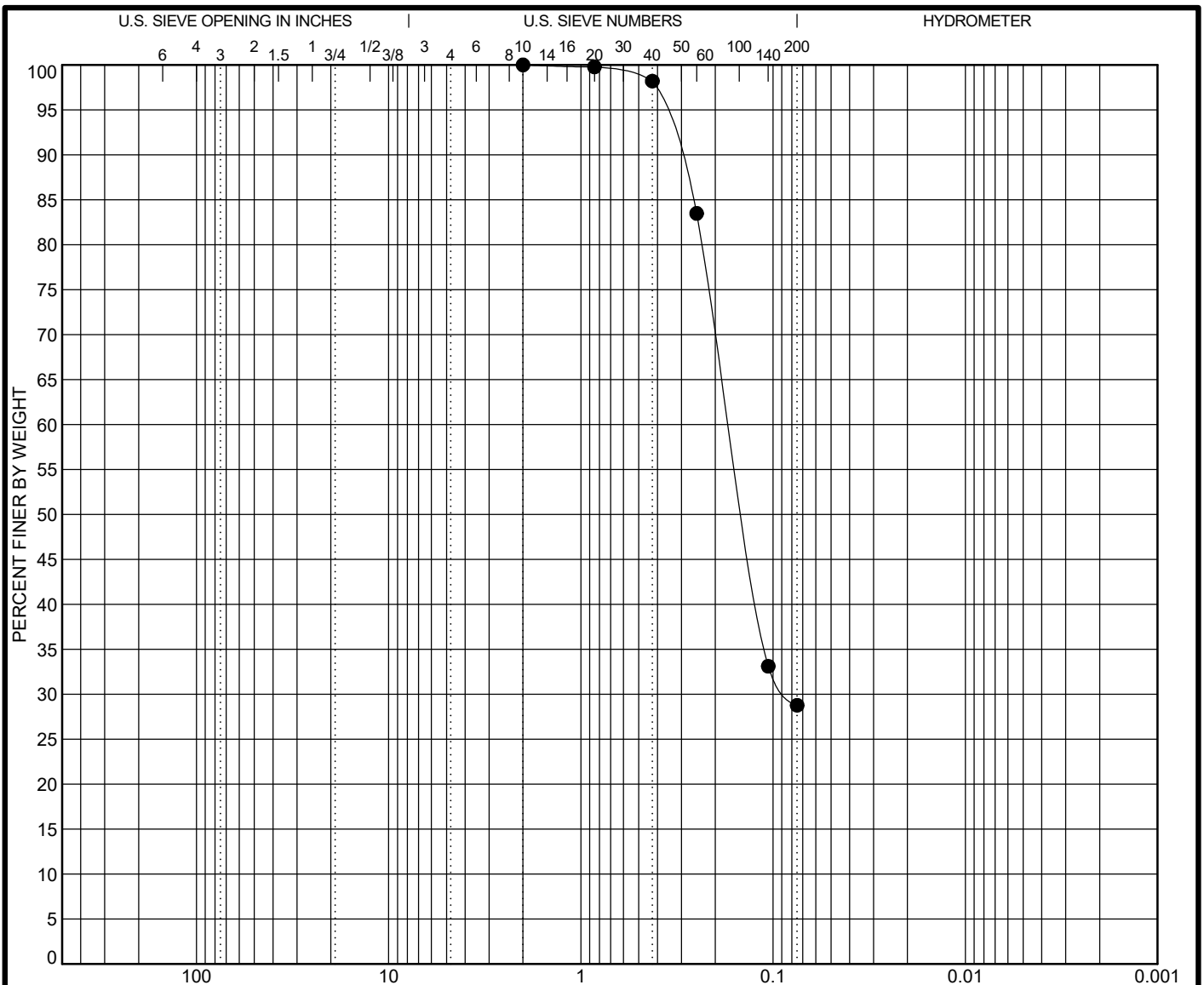
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-01 6.5 ft.	2	0.194	0.124		0.0	85.0	15.0	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● B-1 S-7;14.0 ft.	CLAYEY SAND (SC)	29	13	16		

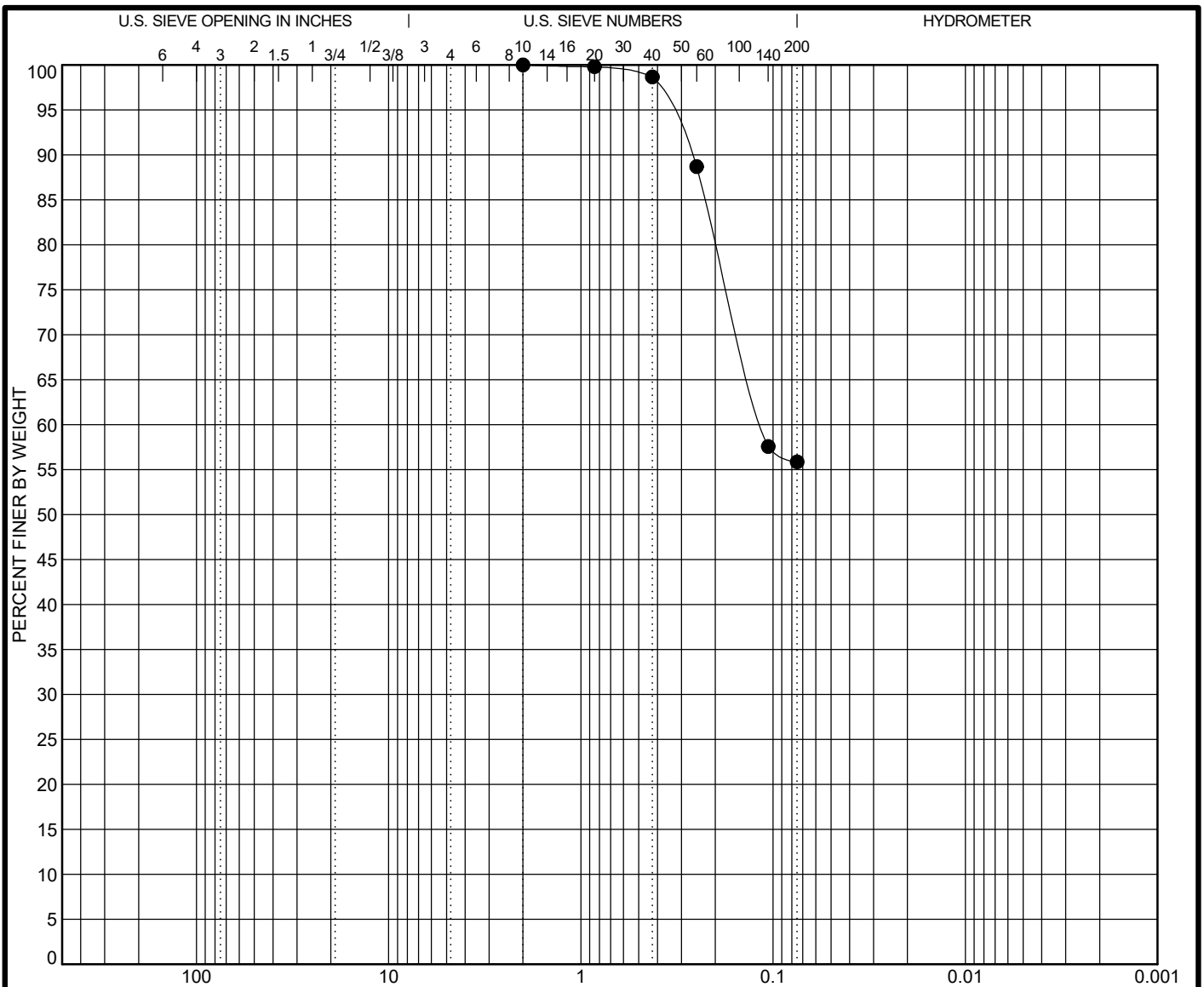
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-01 14.0 ft.	2	0.168	0.083		0.0	71.2	28.8	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● B-1 T-1;16.5 ft.	SANDY FAT CLAY (CH)	85	23	62		

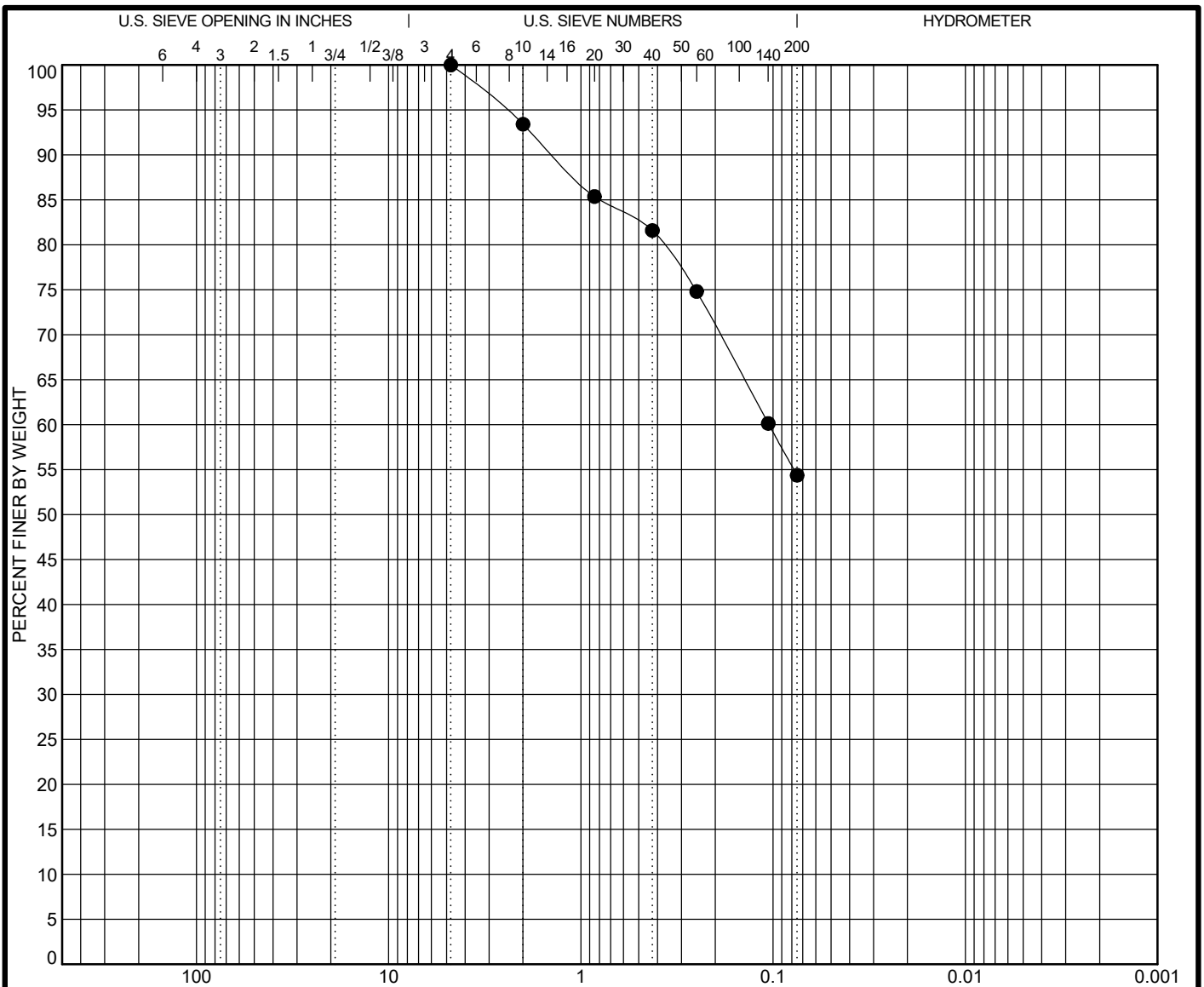
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-01 16.5 ft.	2	0.113			0.0	44.1	55.9	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-1 S-9;24.0 ft.	SANDY LEAN CLAY (CL)					27	19	8		

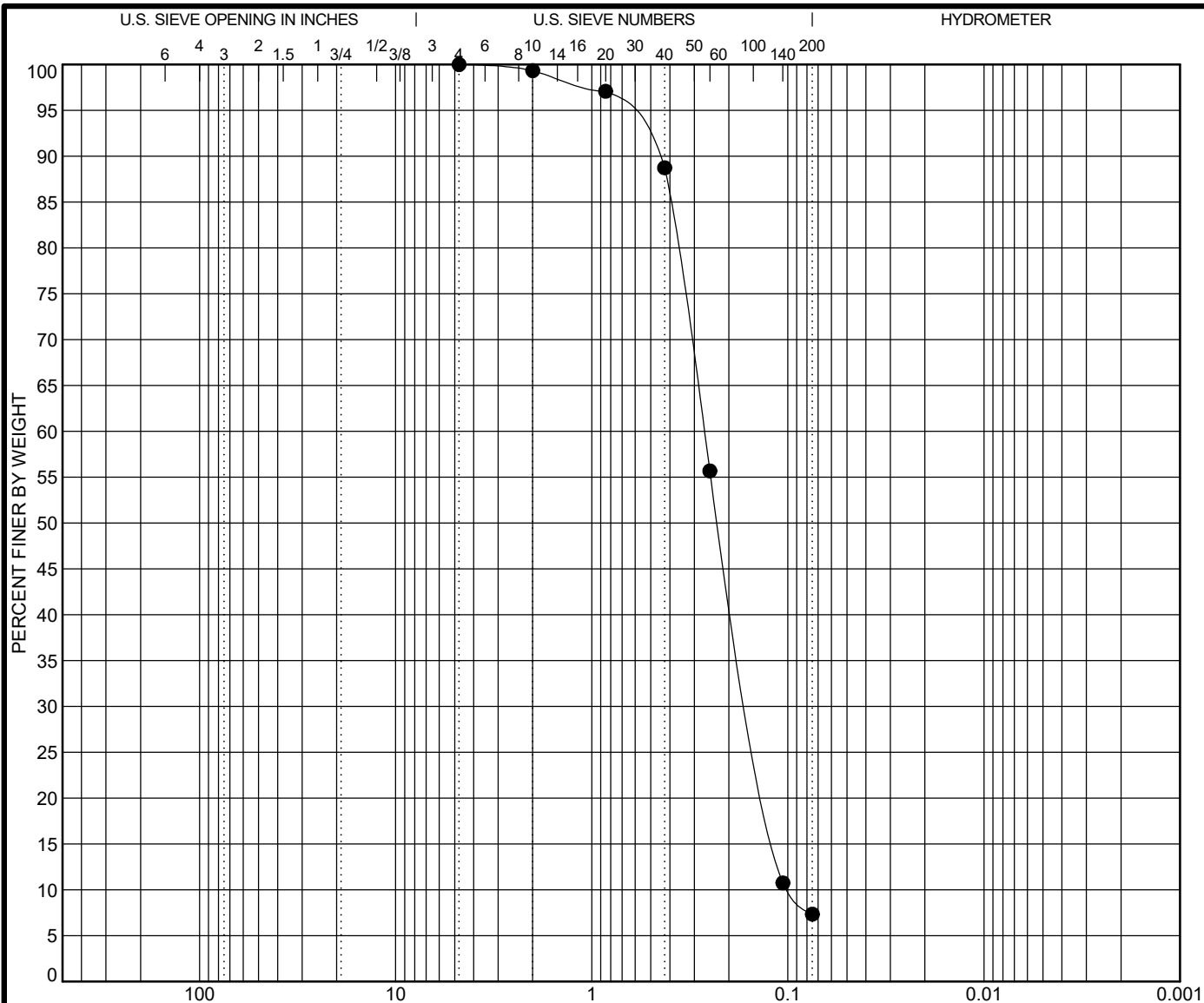
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-01 24.0 ft.	4.75	0.105			0.0	45.6	54.4	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-1 S-12;39.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	0.89	2.73

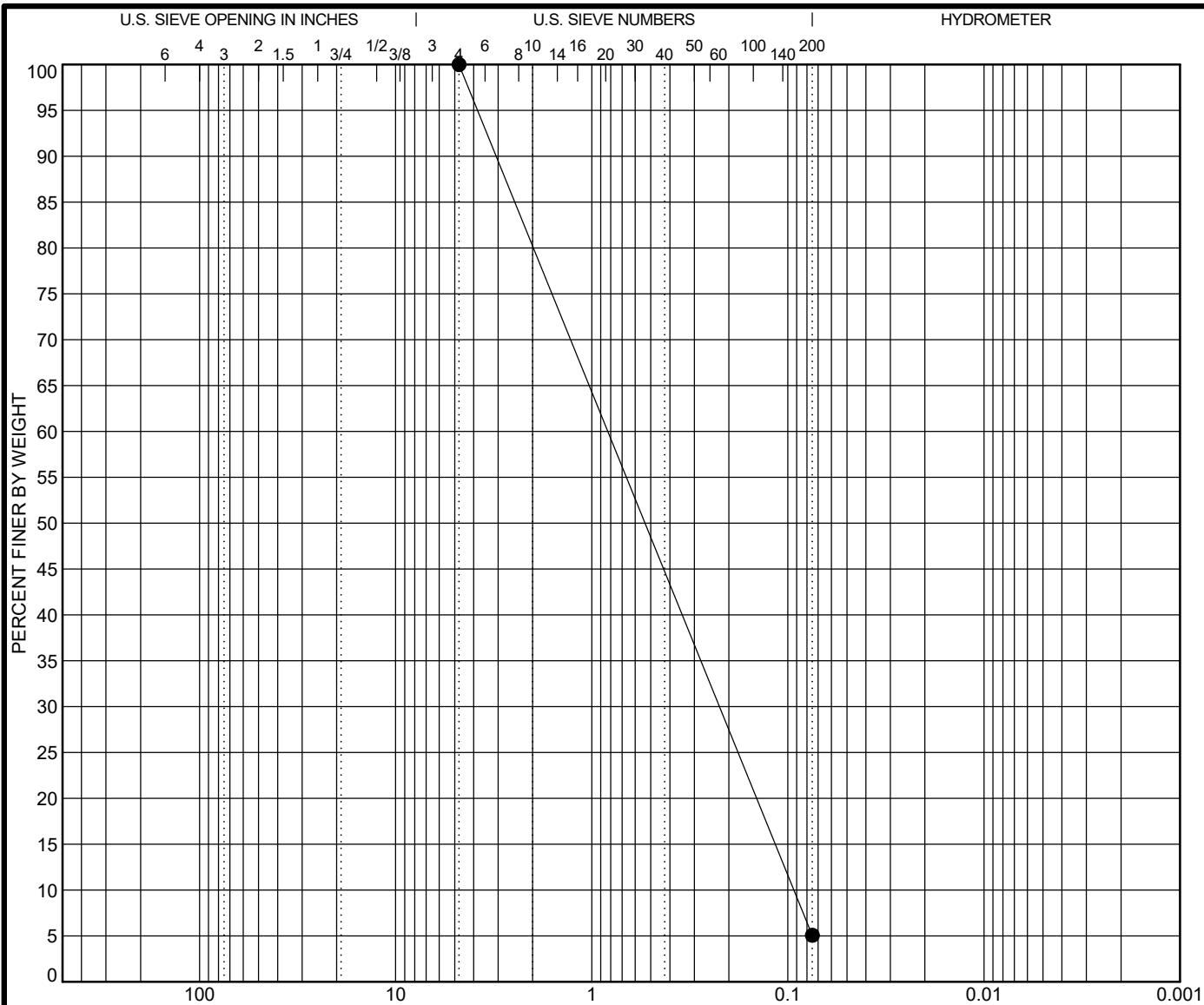
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-01 39.0 ft.	4.75	0.268	0.153	0.098	0.0	92.7	7.3	

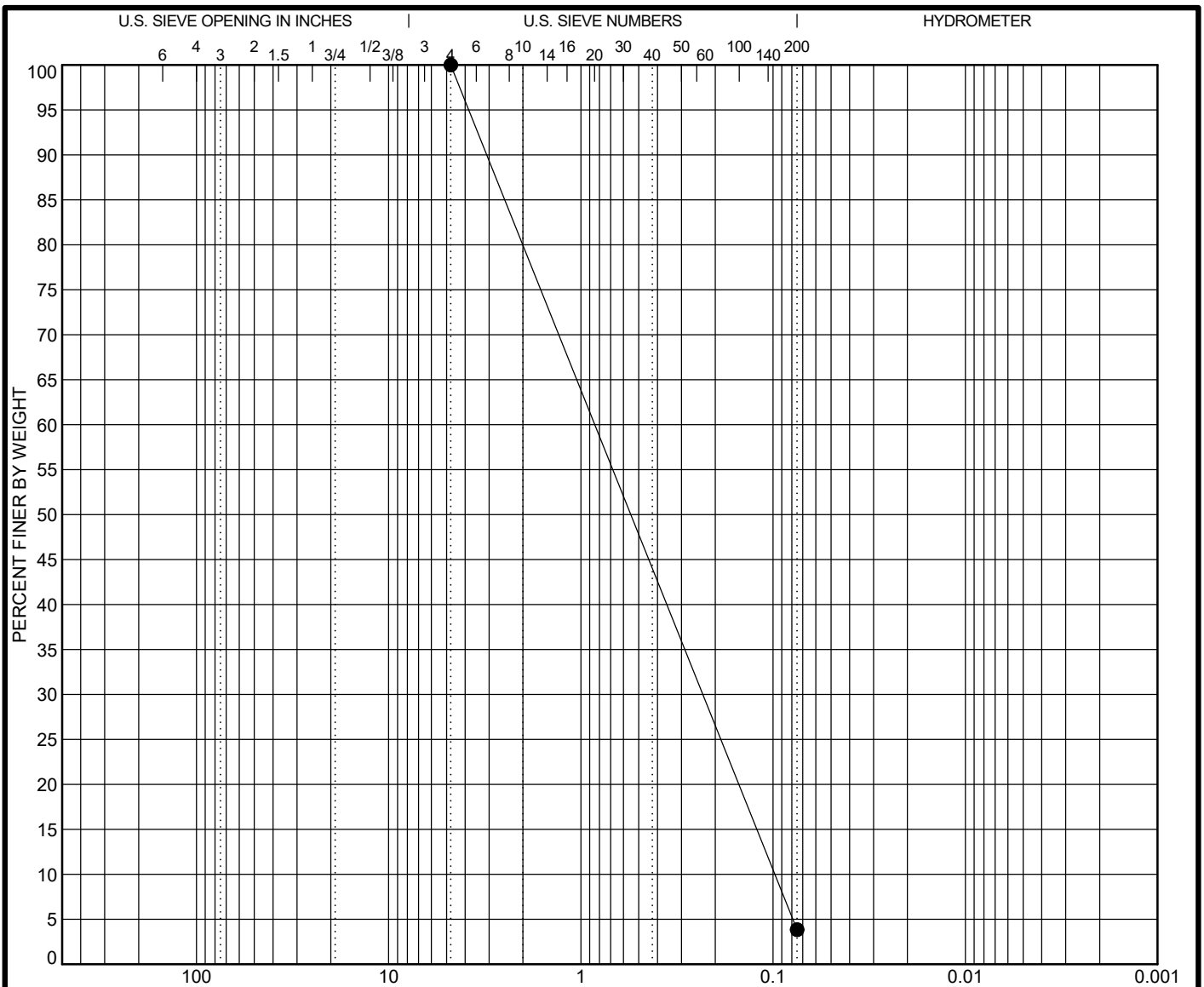
REMARKS:

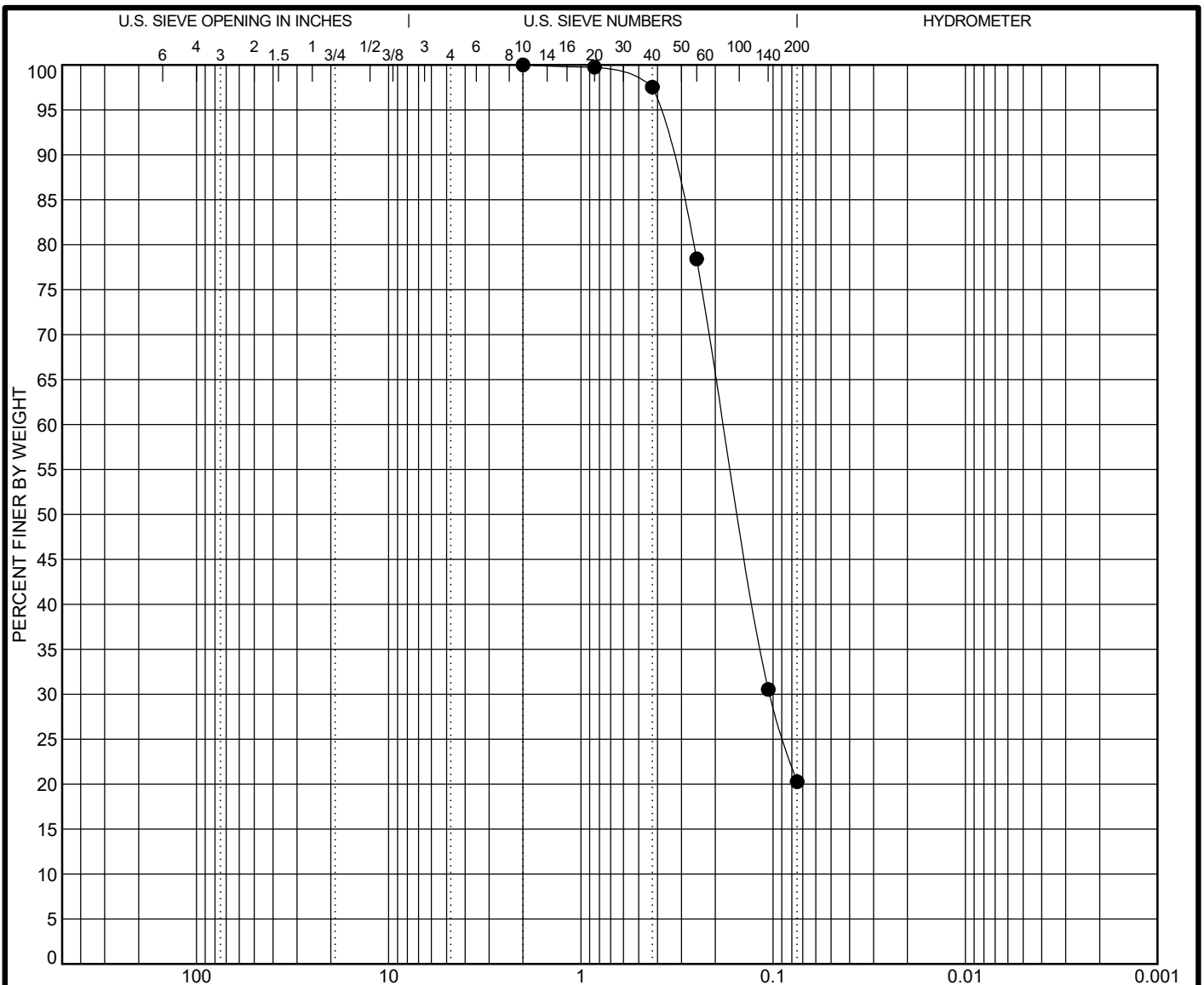


GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL







Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-2 S-5;6.5 ft.	SILTY, CLAYEY SAND (SC-SM)					25	19	6		

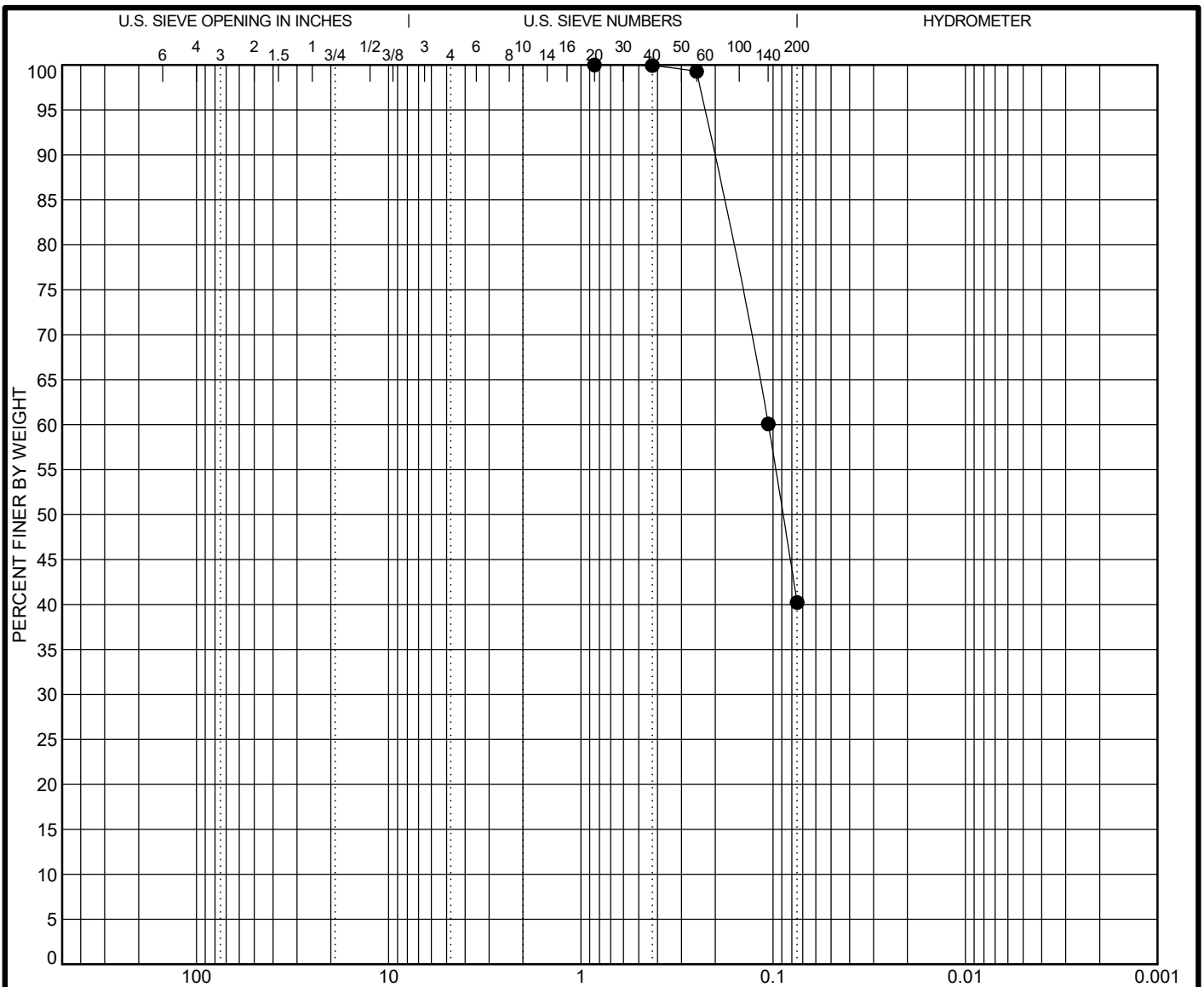
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-02 6.5 ft.	2	0.18	0.104		0.0	79.7	20.3	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● B-2 S-8;19.0 ft.	CLAYEY SAND (SC)	28	16	12		

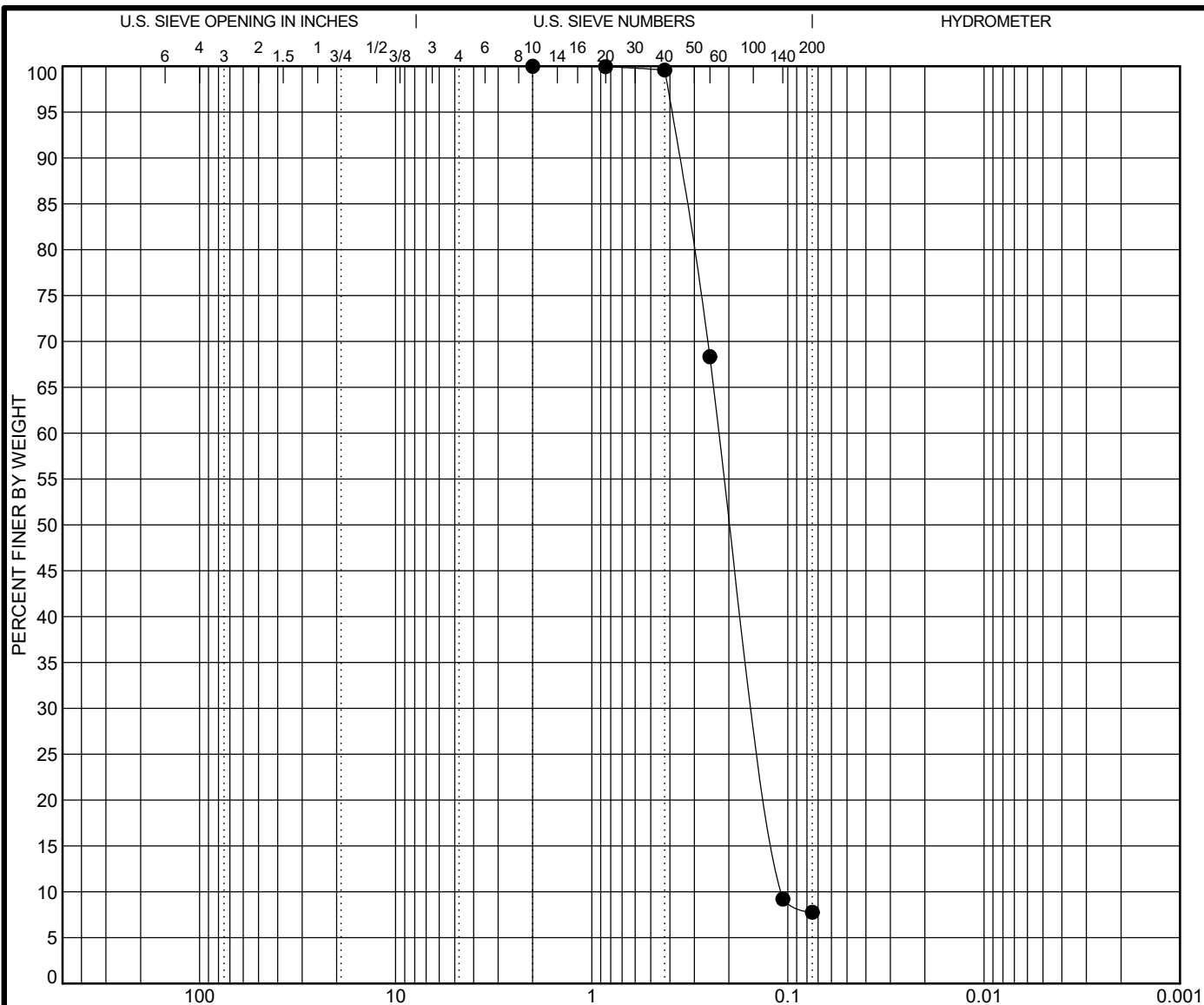
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-02 19.0 ft.	0.85	0.106			0.0	59.8	40.2	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-2 S-11;34.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	0.86	2.07

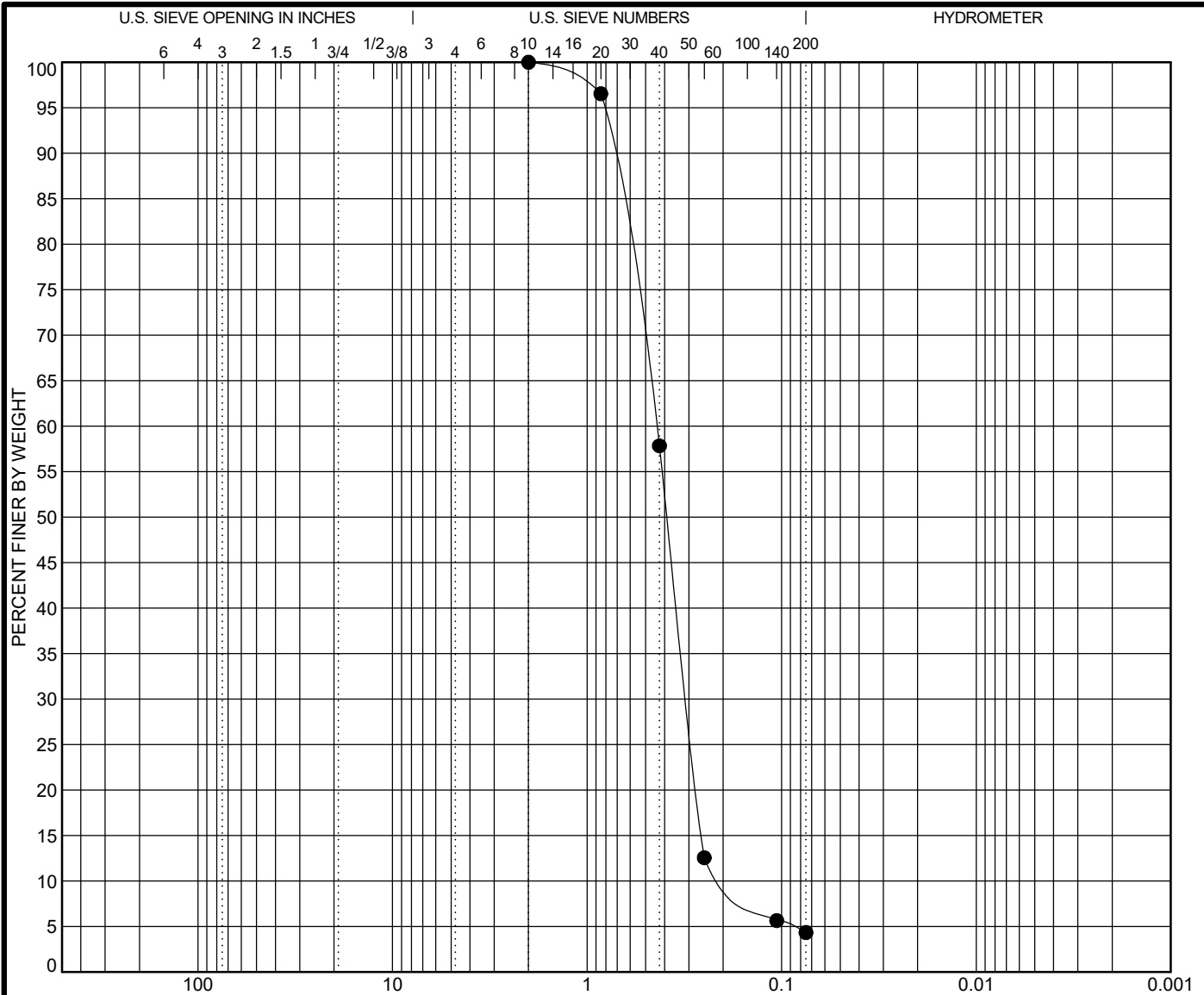
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-02 34.0 ft.	2	0.222	0.143	0.107	0.0	92.2	7.8	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-2 S-13;44.0 ft.	POORLY GRADED SAND (SP)					NP	NP	NP	1.17	2.43

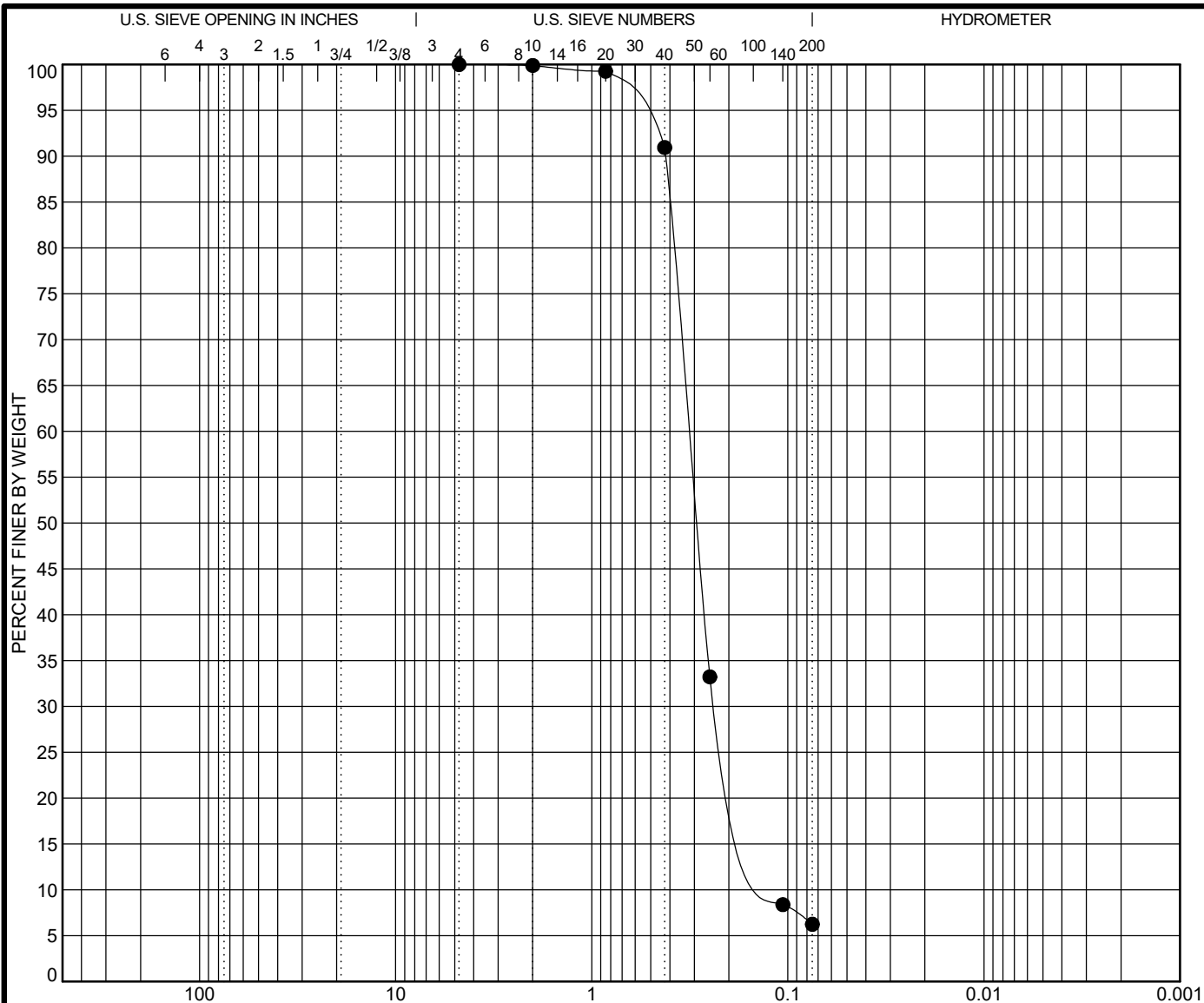
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-02 44.0 ft.	2	0.442	0.307	0.182	0.0	95.7	4.3	

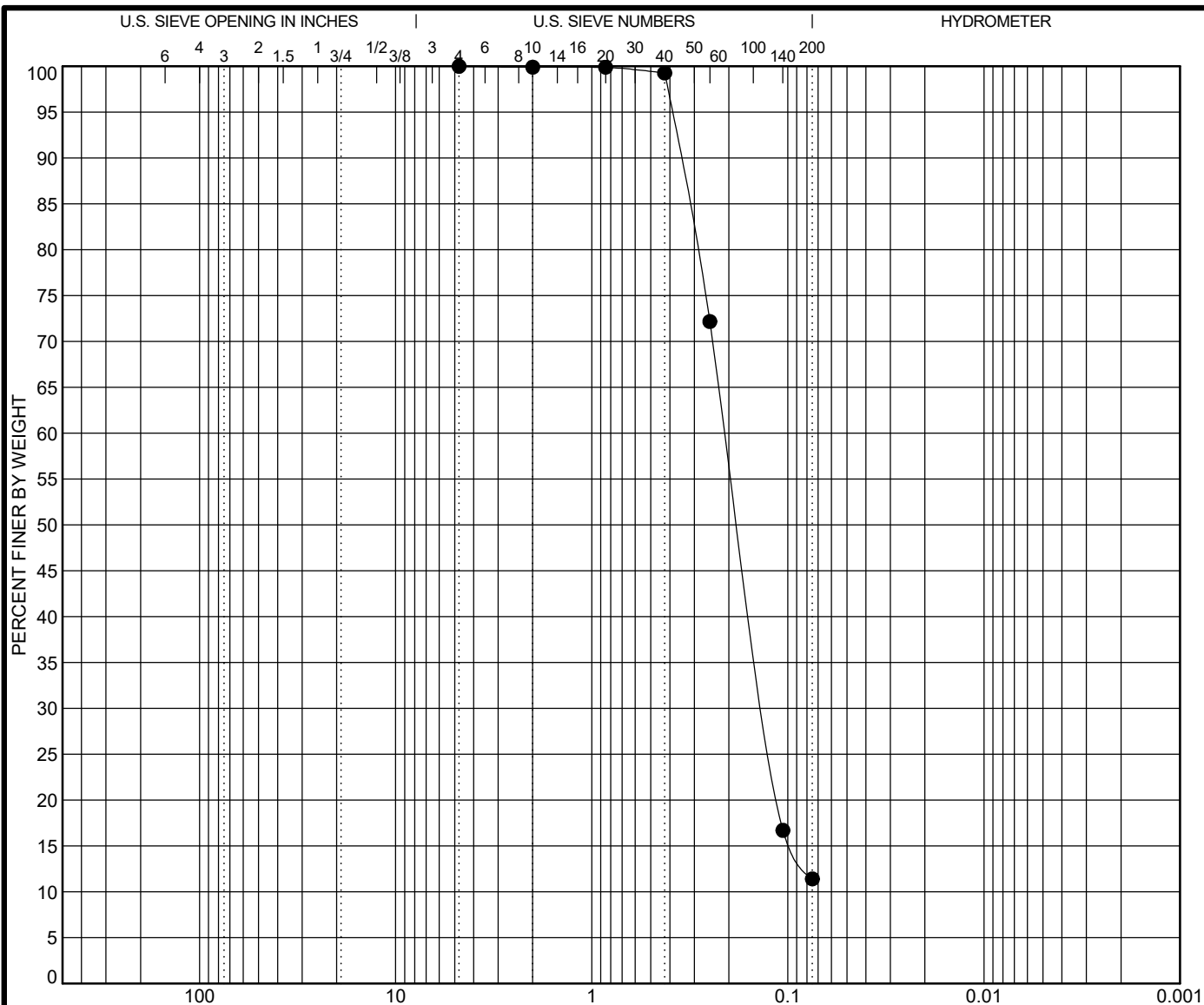
REMARKS:

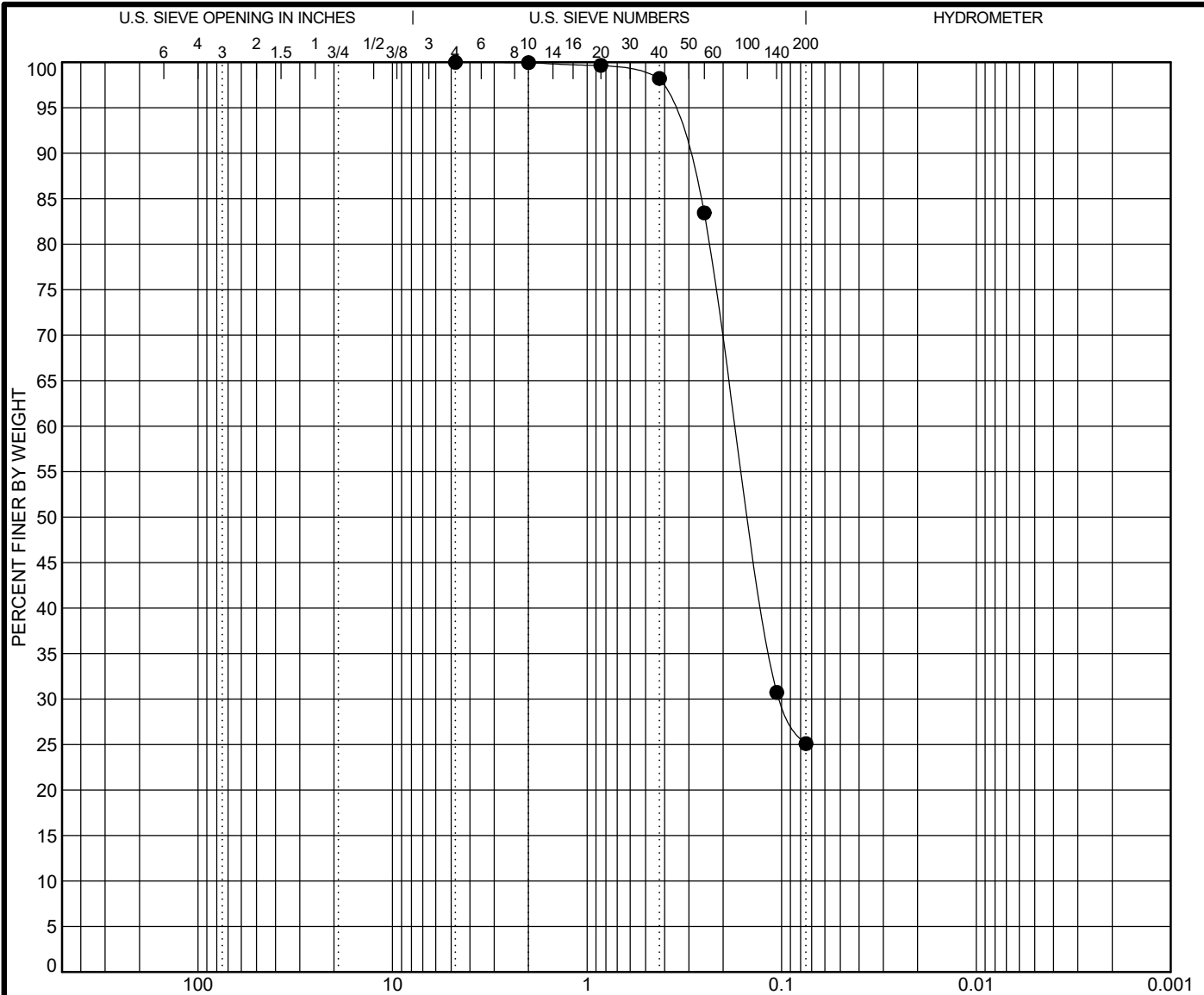


GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL







Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● B-3 S-4;5.0 ft.						

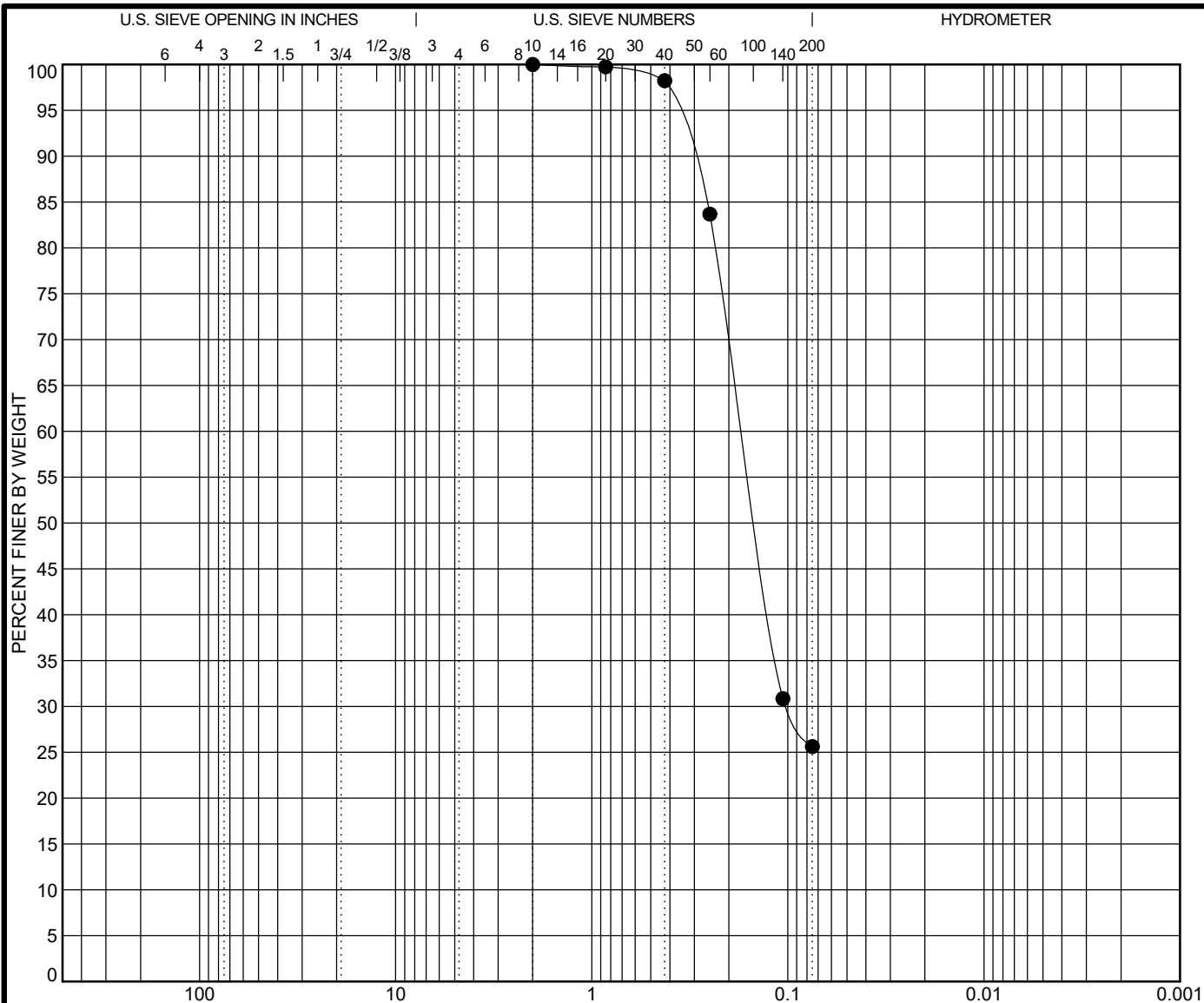
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-03 5.0 ft.	4.75	0.171	0.101		0.0	74.9	25.1	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● B-3 S-7;14.0 ft.	CLAYEY SAND (SC)	28	16	12		

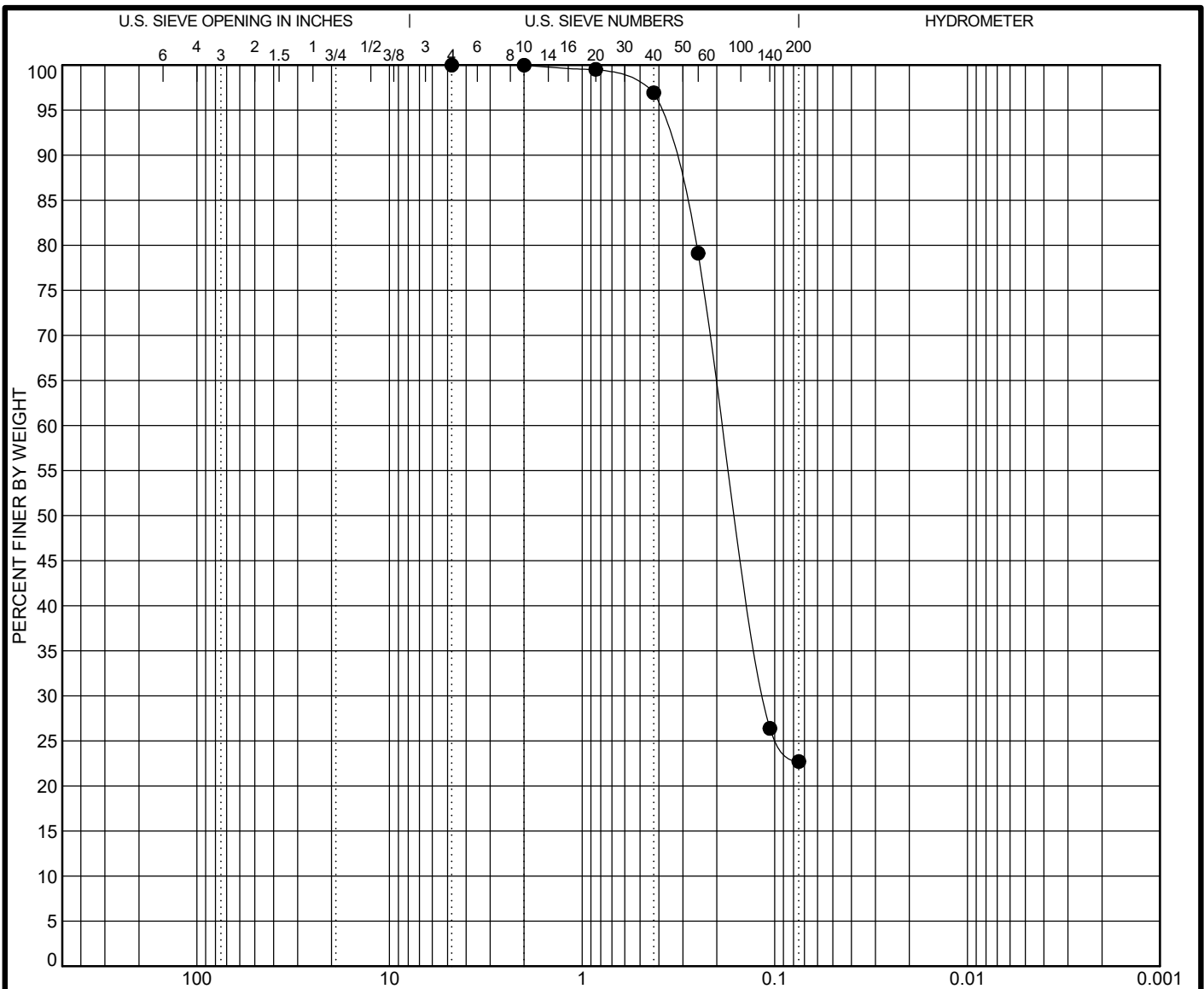
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-03 14.0 ft.	2	0.17	0.1		0.0	74.4	25.6	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● B-3 S-8; 19.0 ft.	CLAYEY SAND (SC)	28	12	16		

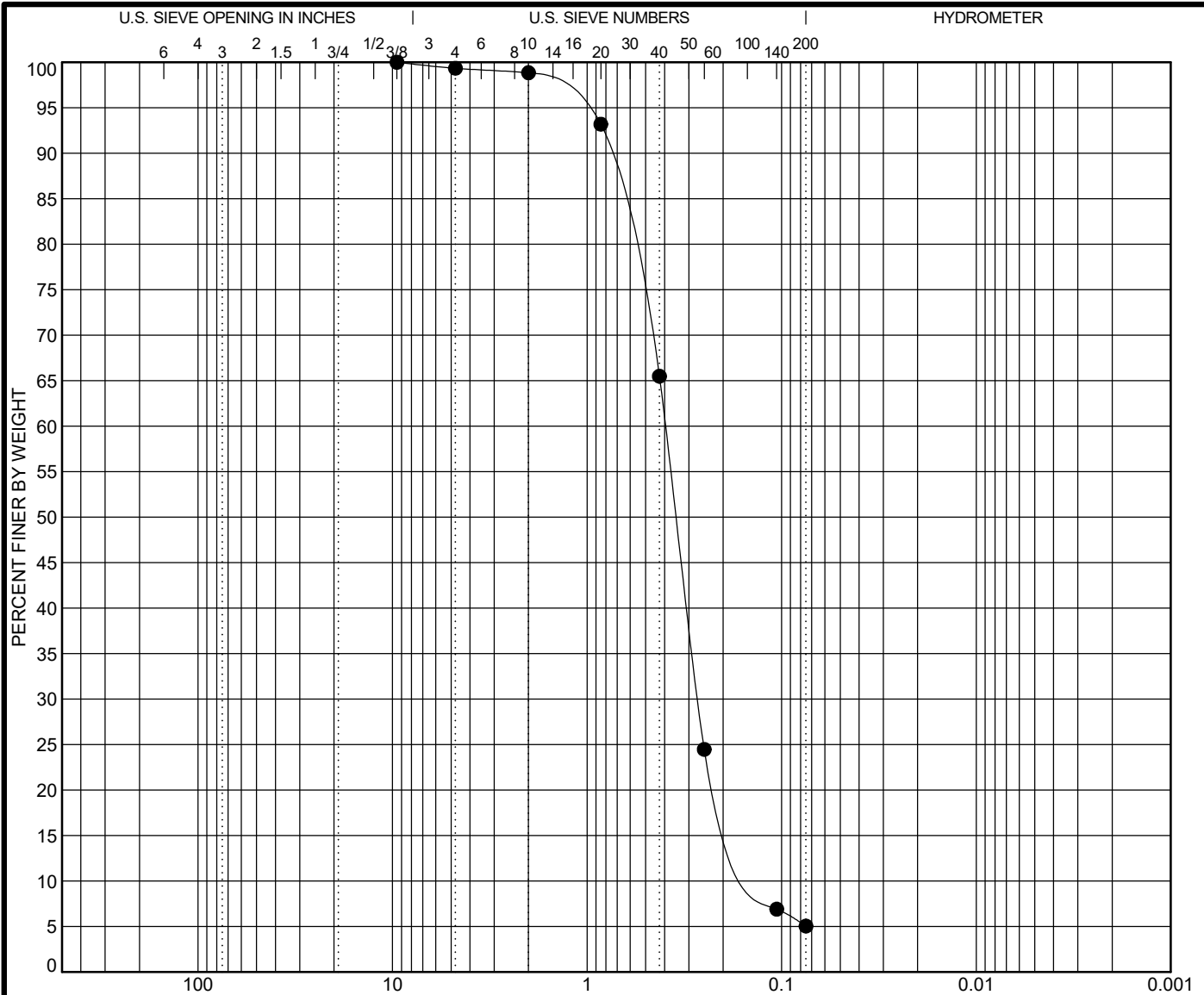
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-03 19.0 ft.	4.75	0.183	0.112		0.0	77.3	22.7	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-3 S-12;39.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	1.48	3.21

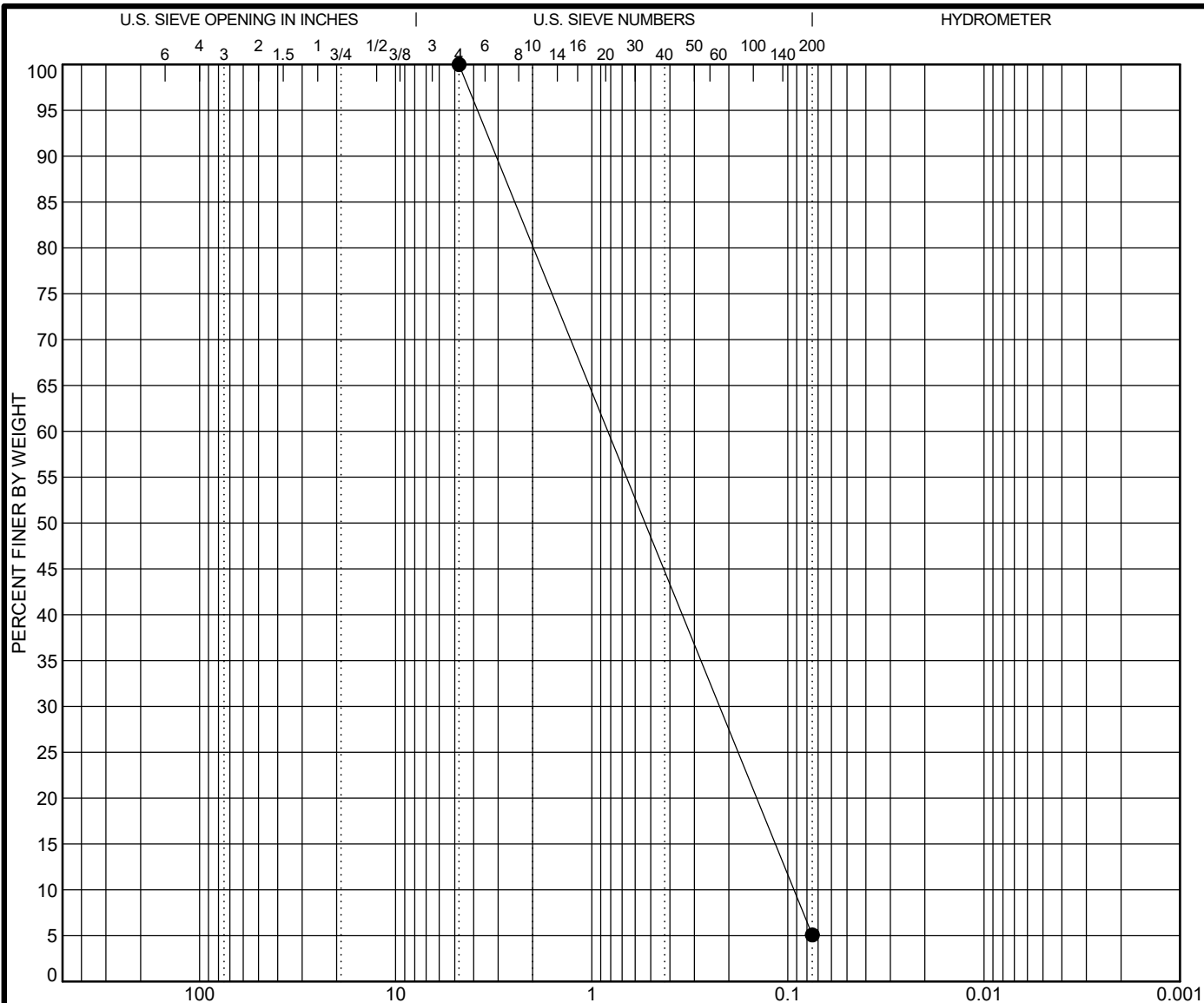
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-03 39.0 ft.	9.5	0.396	0.269	0.123	0.7	94.3	5.0	

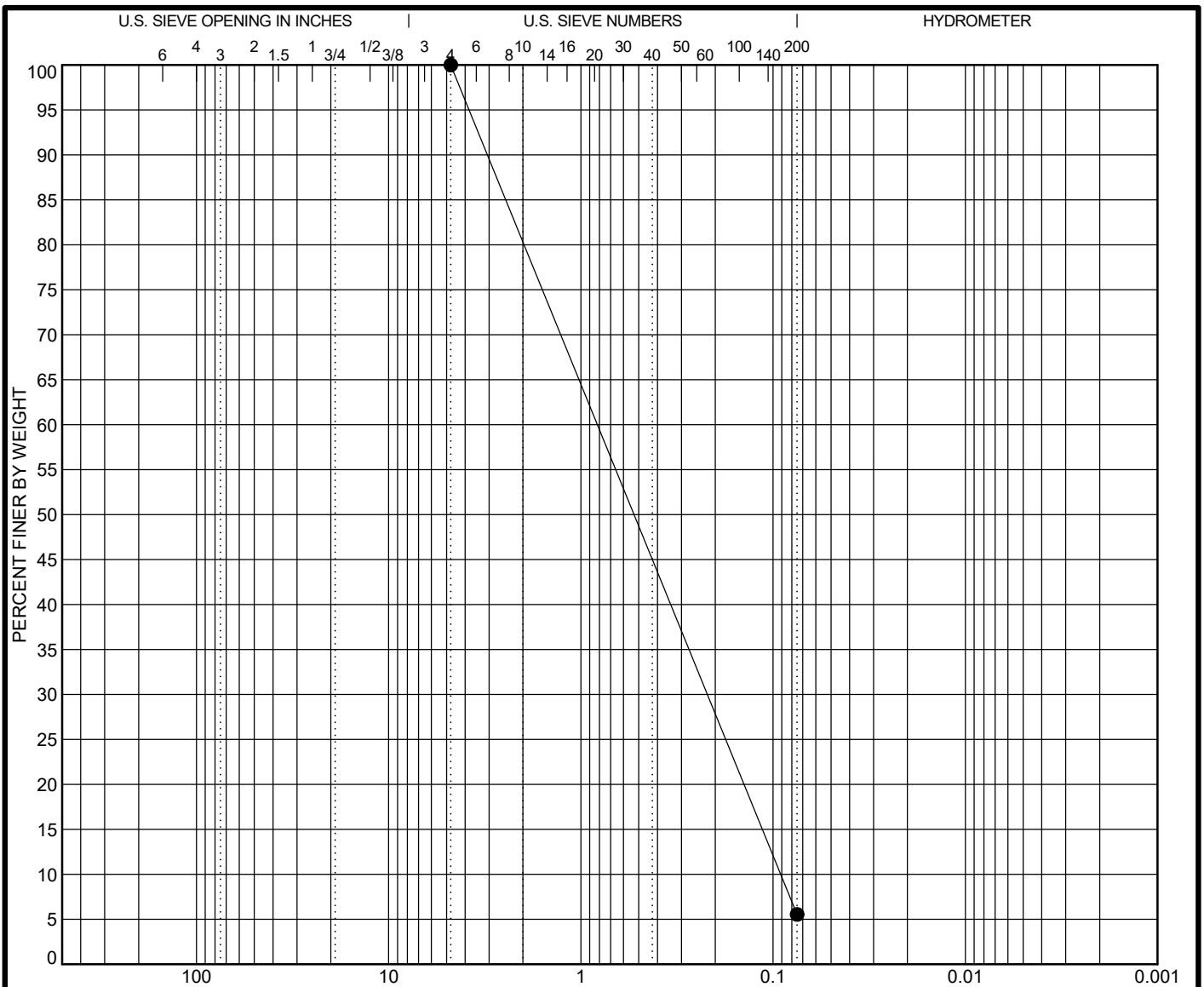
REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL





Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-3 S-20;79.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	0.64	8.99

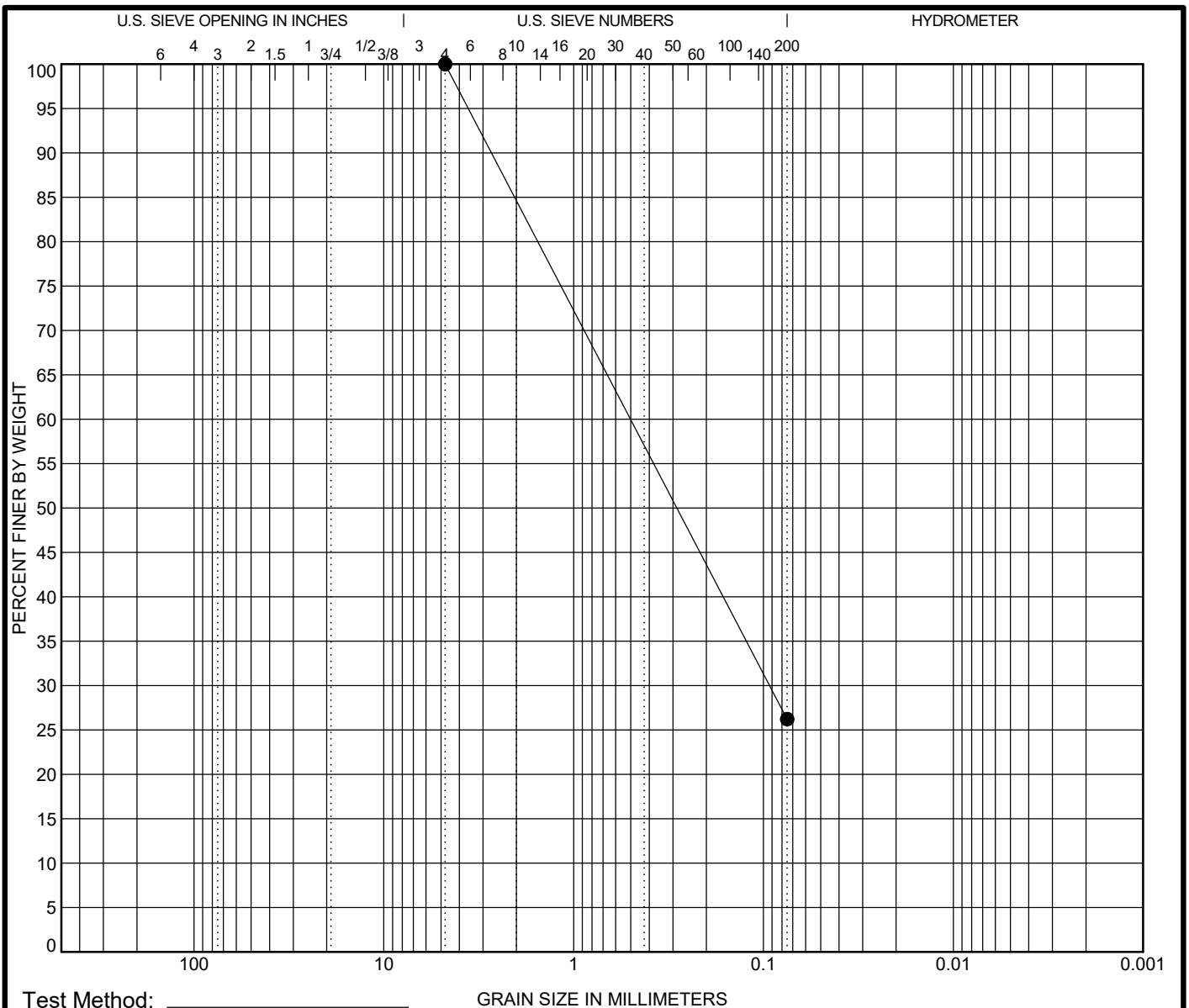
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-03 79.0 ft.	4.75	0.82	0.22	0.091	0.0	94.5	5.5	

REMARKS:

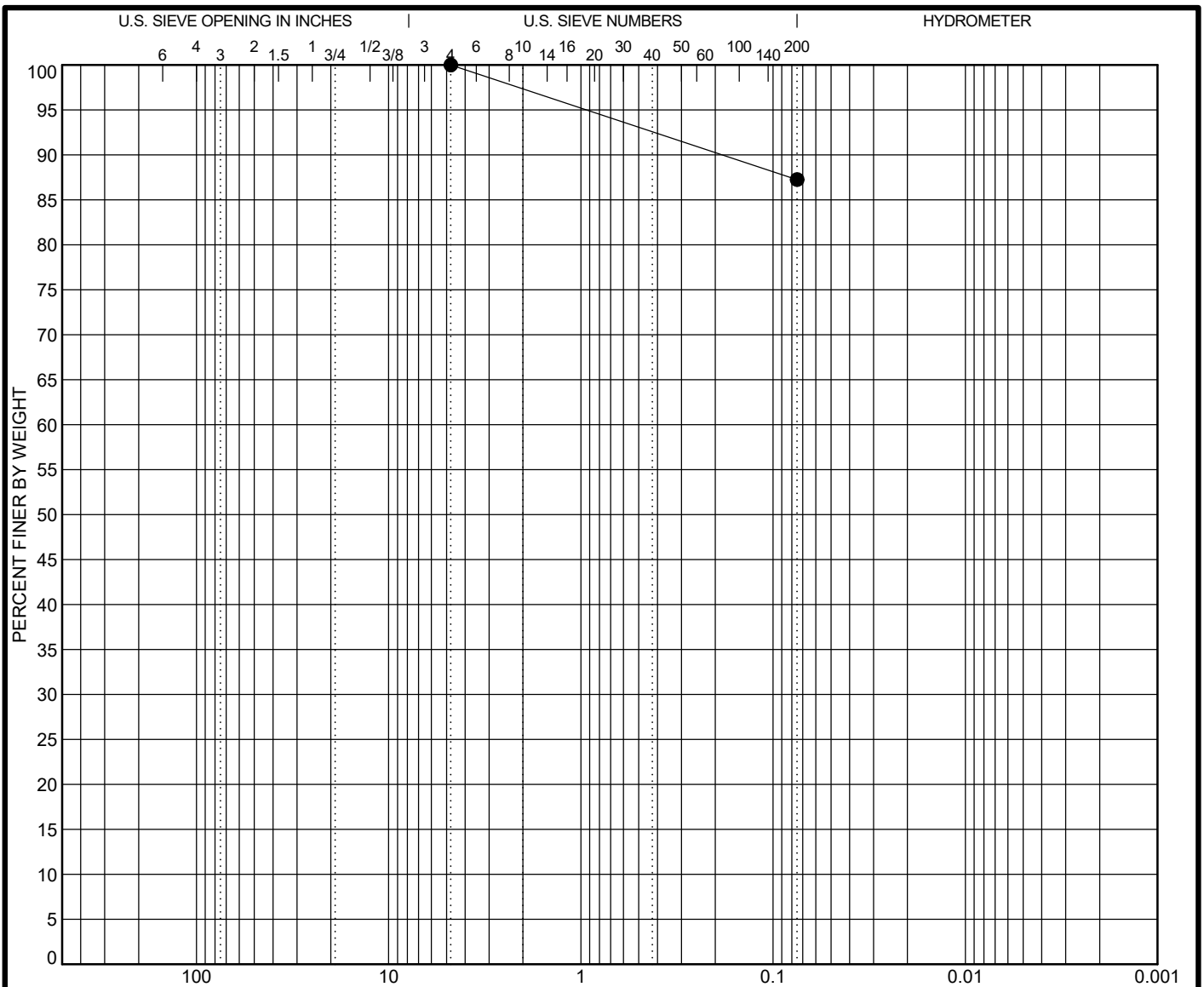


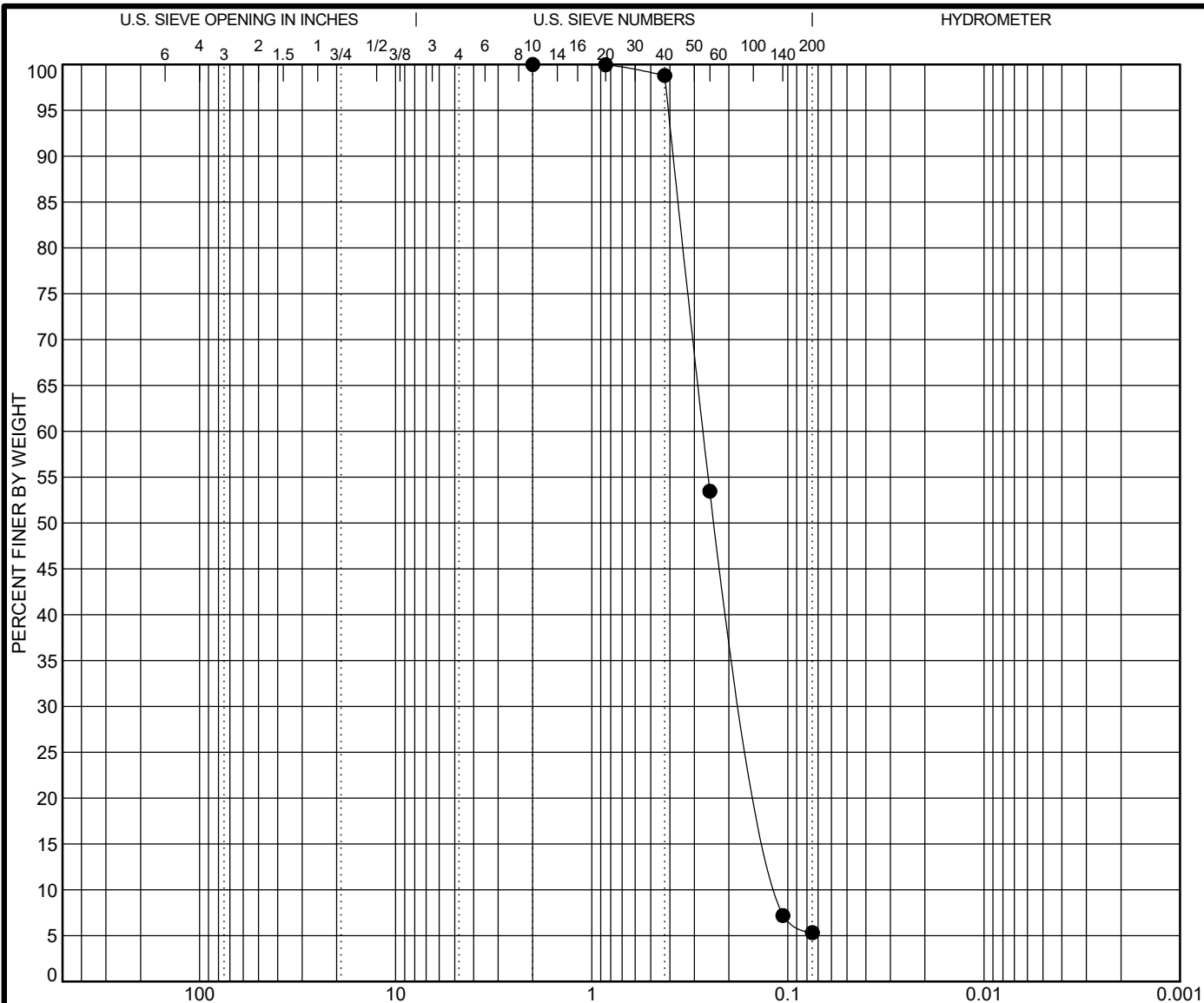
GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL









Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-4 S-11;34.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	0.87	2.42

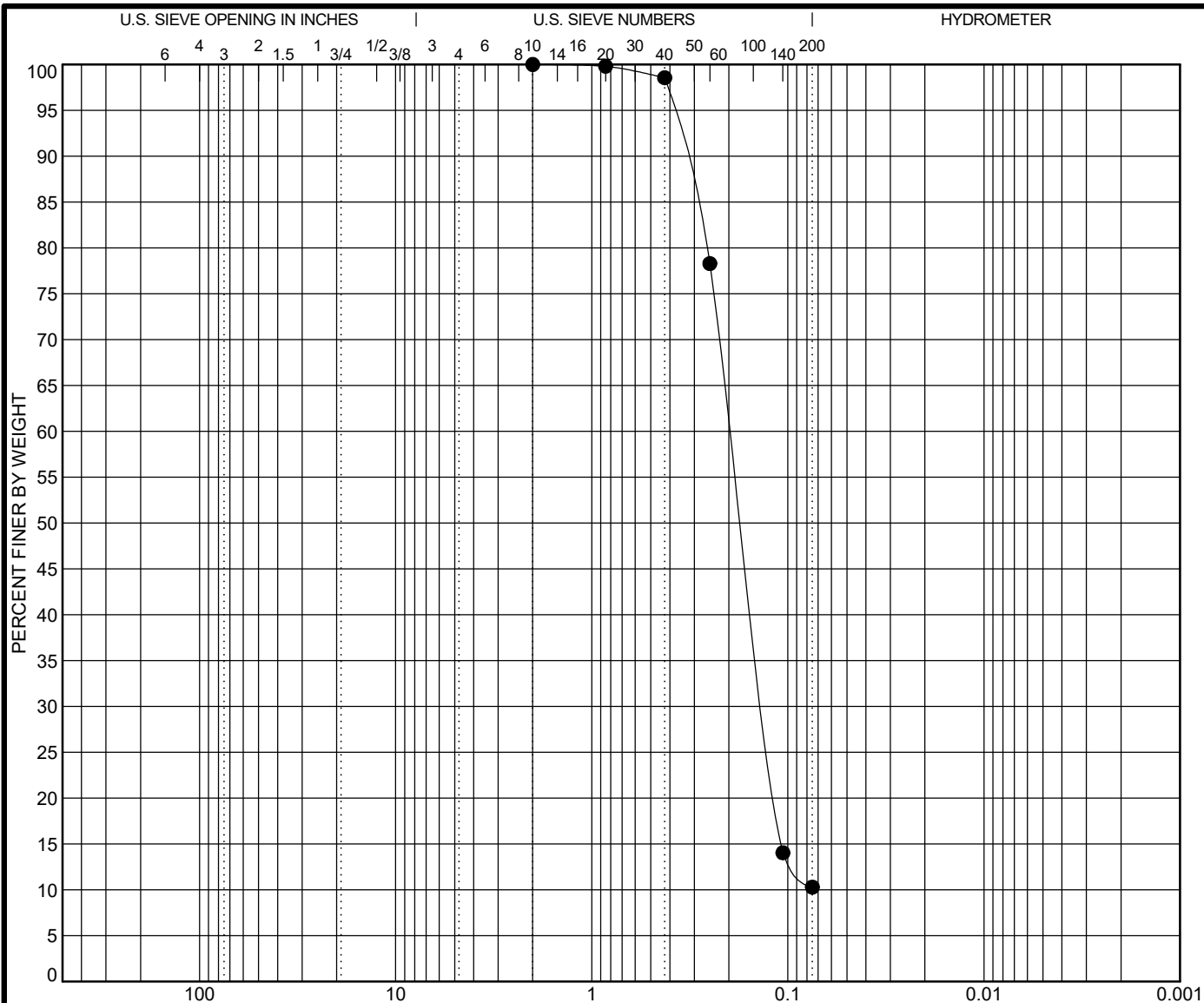
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-04 34.0 ft.	2	0.27	0.162	0.112	0.0	94.7	5.3	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-4 S-13;44.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	1.20	2.68

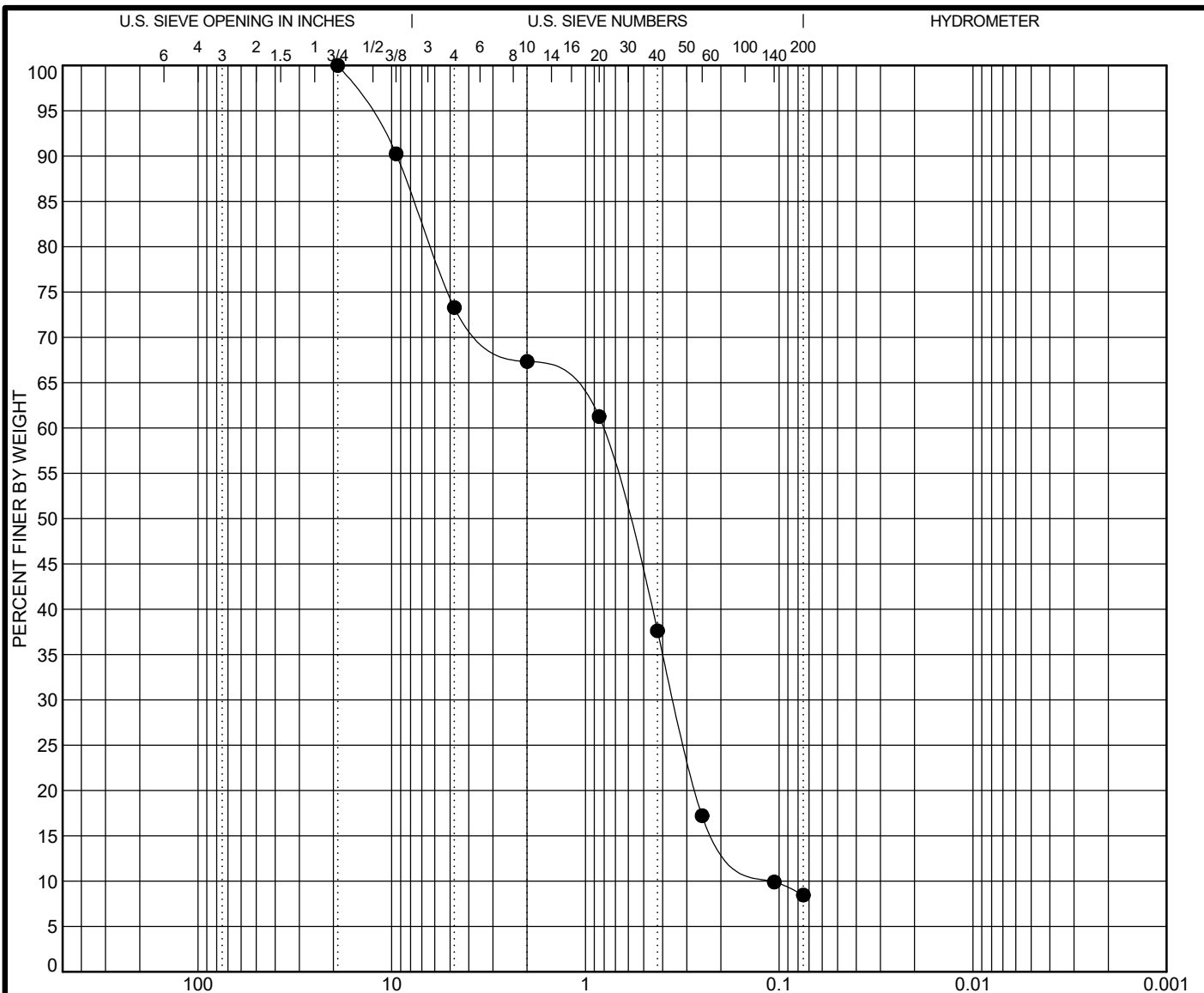
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-04 44.0 ft.	2	0.196	0.131		0.0	89.7	10.3	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-4 S-16;59.0 ft.	WELL-GRADED SAND with SILT and GRAVEL (SW-SM)					NP	NP	NP	1.38	7.64

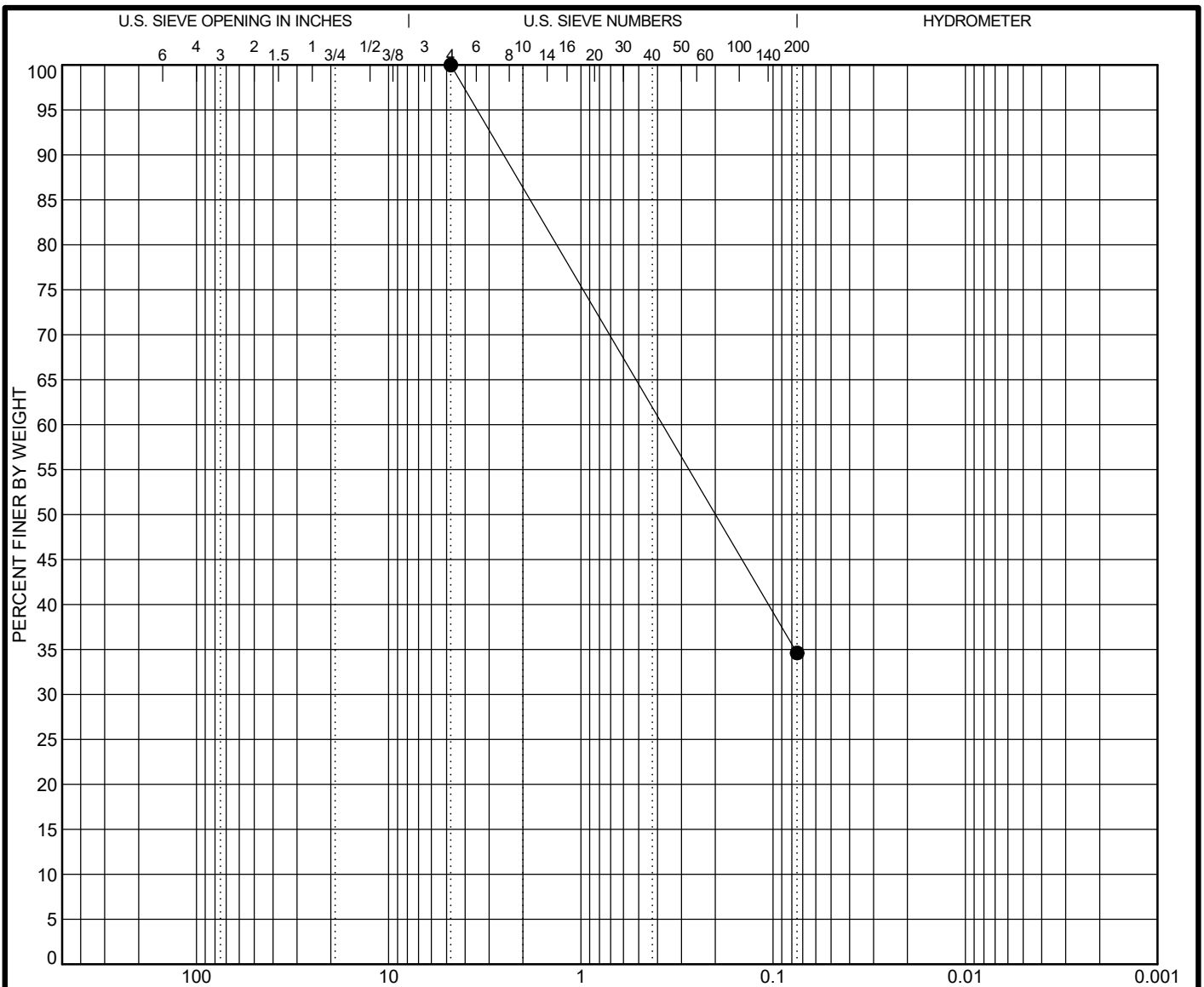
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-04 59.0 ft.	19	0.819	0.349	0.107	26.7	64.8	8.5	

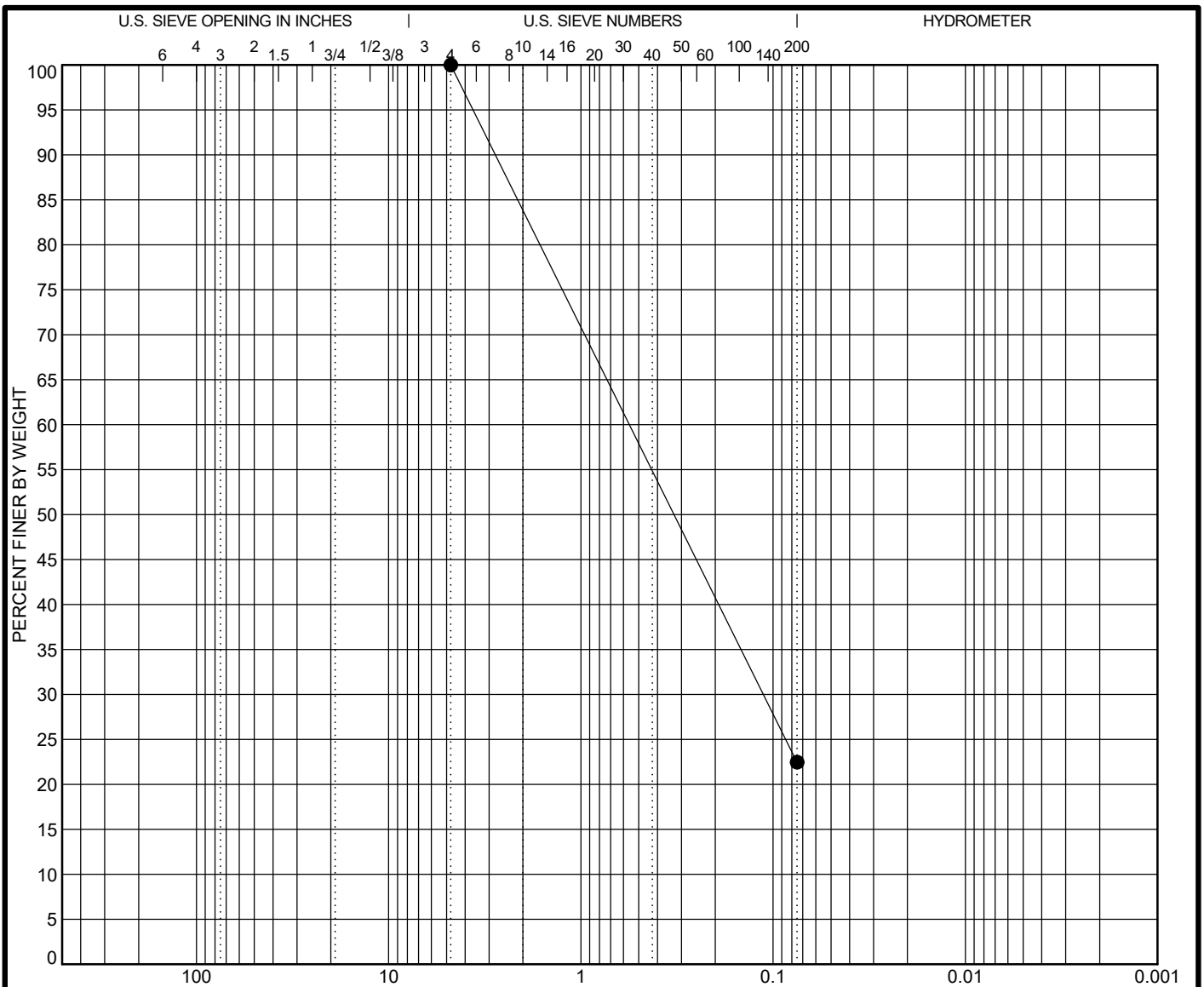
REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL





Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-5 S-6;9.0 ft.	SILTY SAND (SM)					NP	NP	NP		

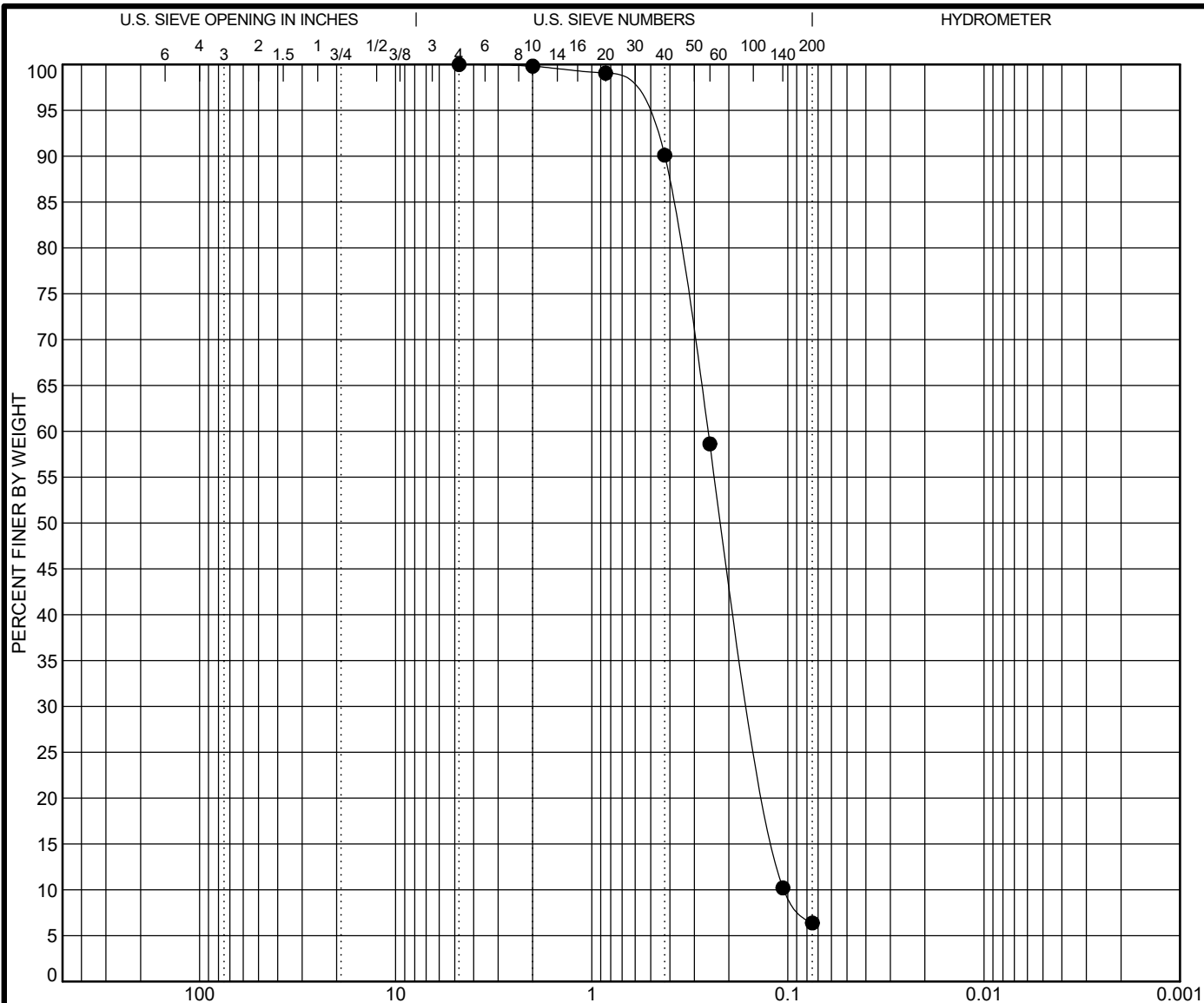
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-05 9.0 ft.	4.75	0.559	0.112		0.0	77.5	22.5	

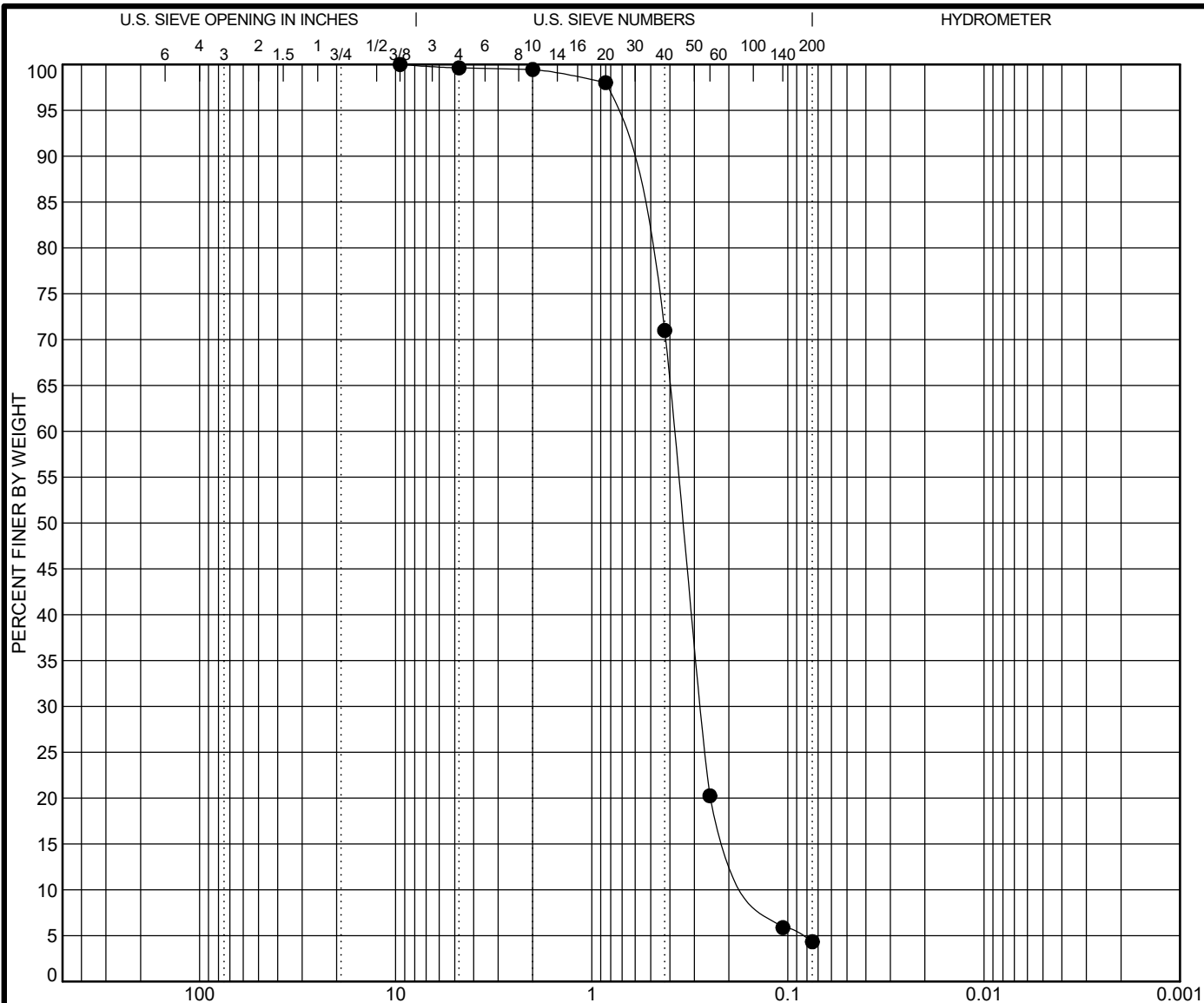
REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL





Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-5 S-14;49.0 ft.	POORLY GRADED SAND (SP)					NP	NP	NP	1.49	2.79

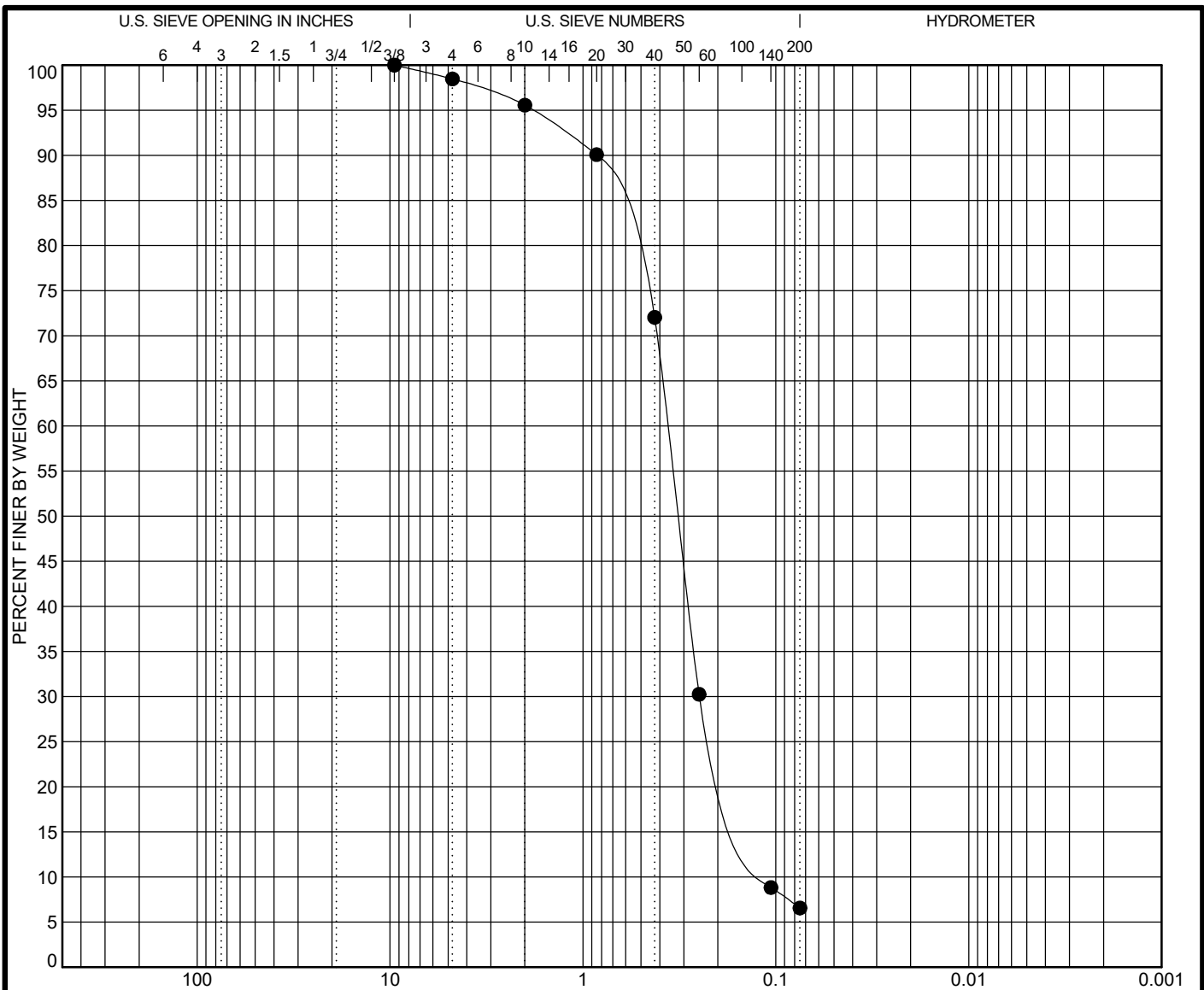
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-05 49.0 ft.	9.5	0.379	0.277	0.136	0.4	95.3	4.3	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-5 S-18;69.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	1.51	3.28

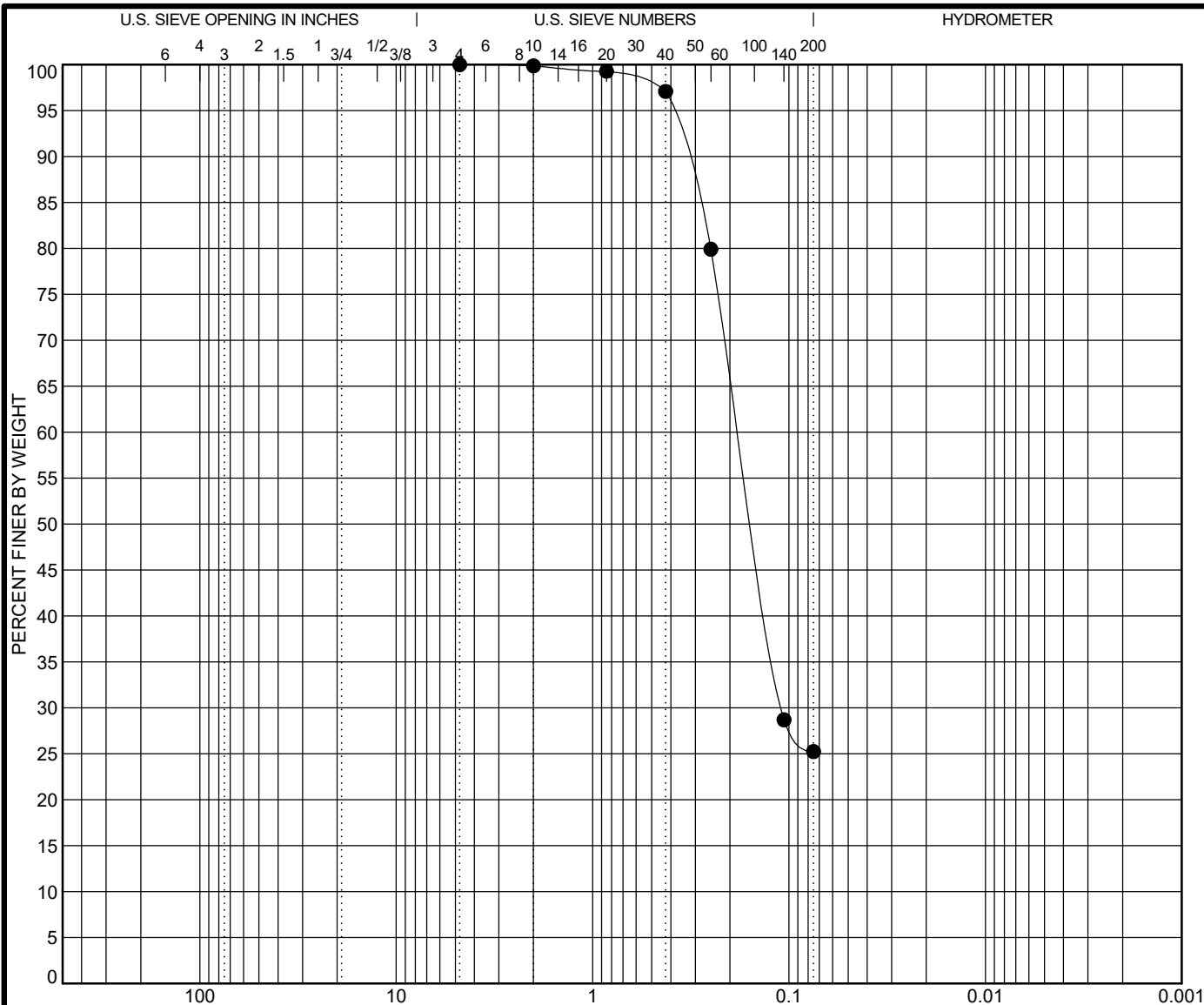
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-05 69.0 ft.	9.5	0.365	0.248	0.111	1.5	91.9	6.6	

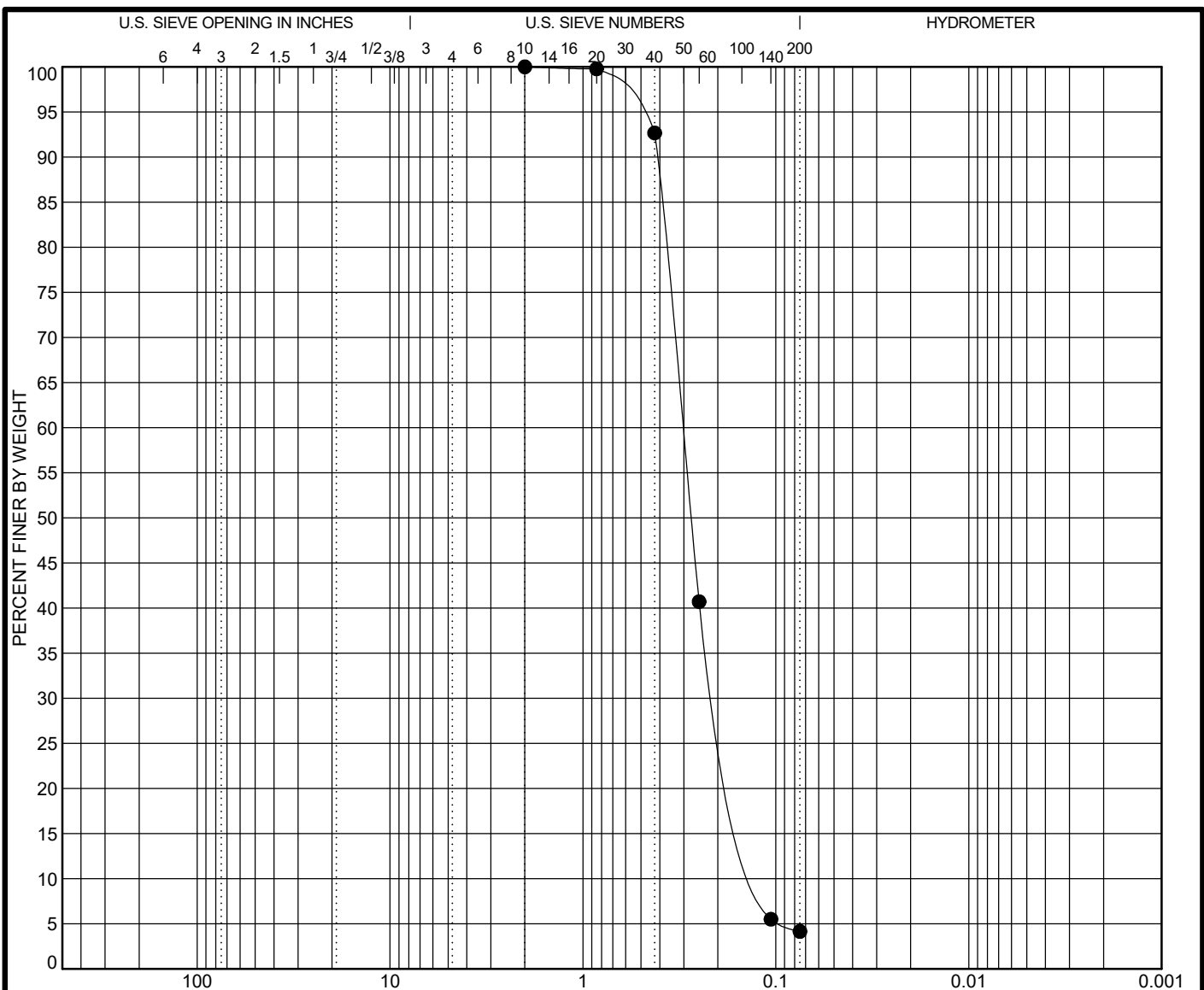
REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL





Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-6 S-10;29.0 ft.	POORLY GRADED SAND (SP)								1.03	2.57

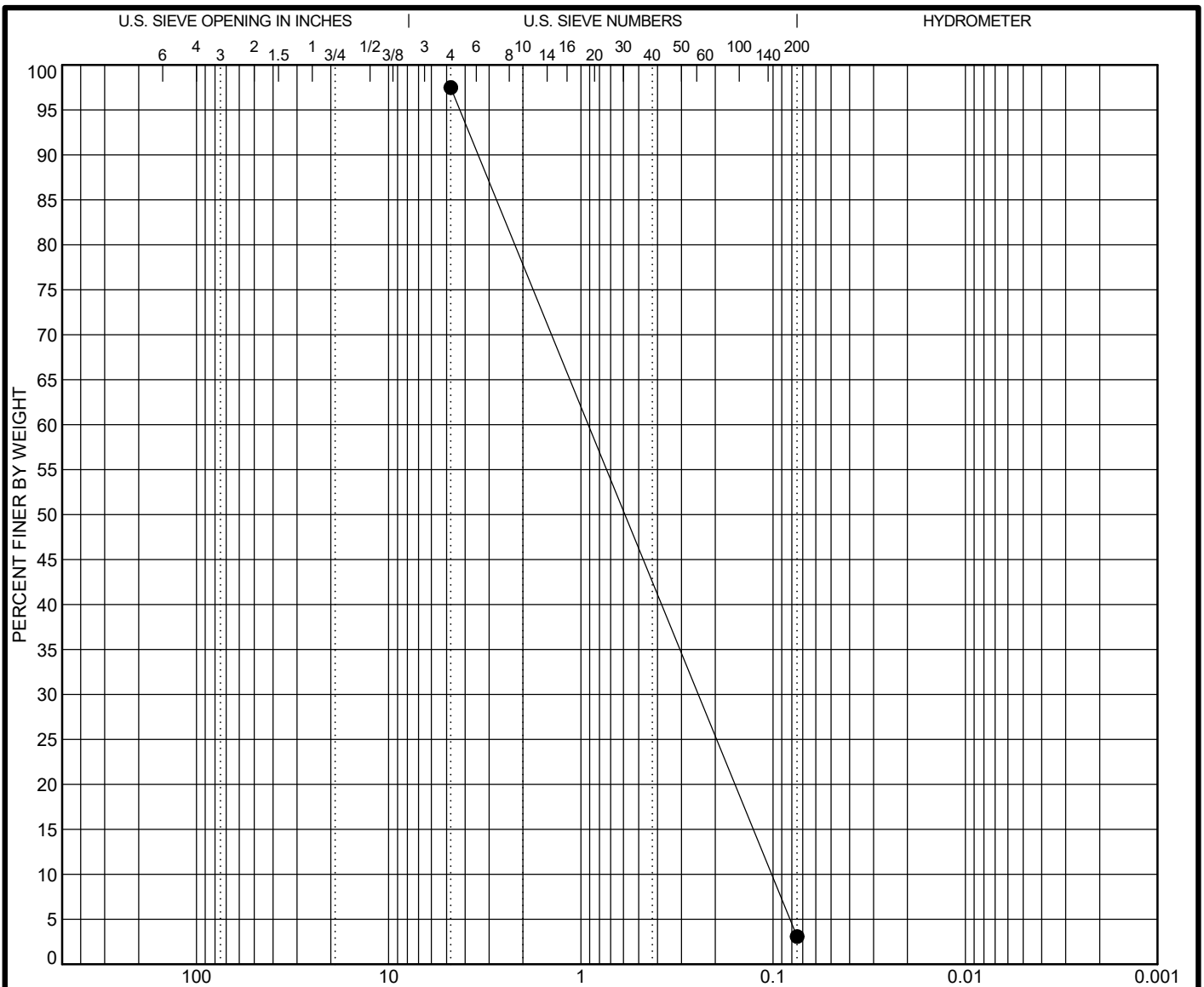
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-06 29.0 ft.	2	0.304	0.193	0.118	0.0	95.8	4.2	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-6 S-14;49.0 ft.	POORLY GRADED SAND (SP)					NP	NP	NP	0.64	9.00

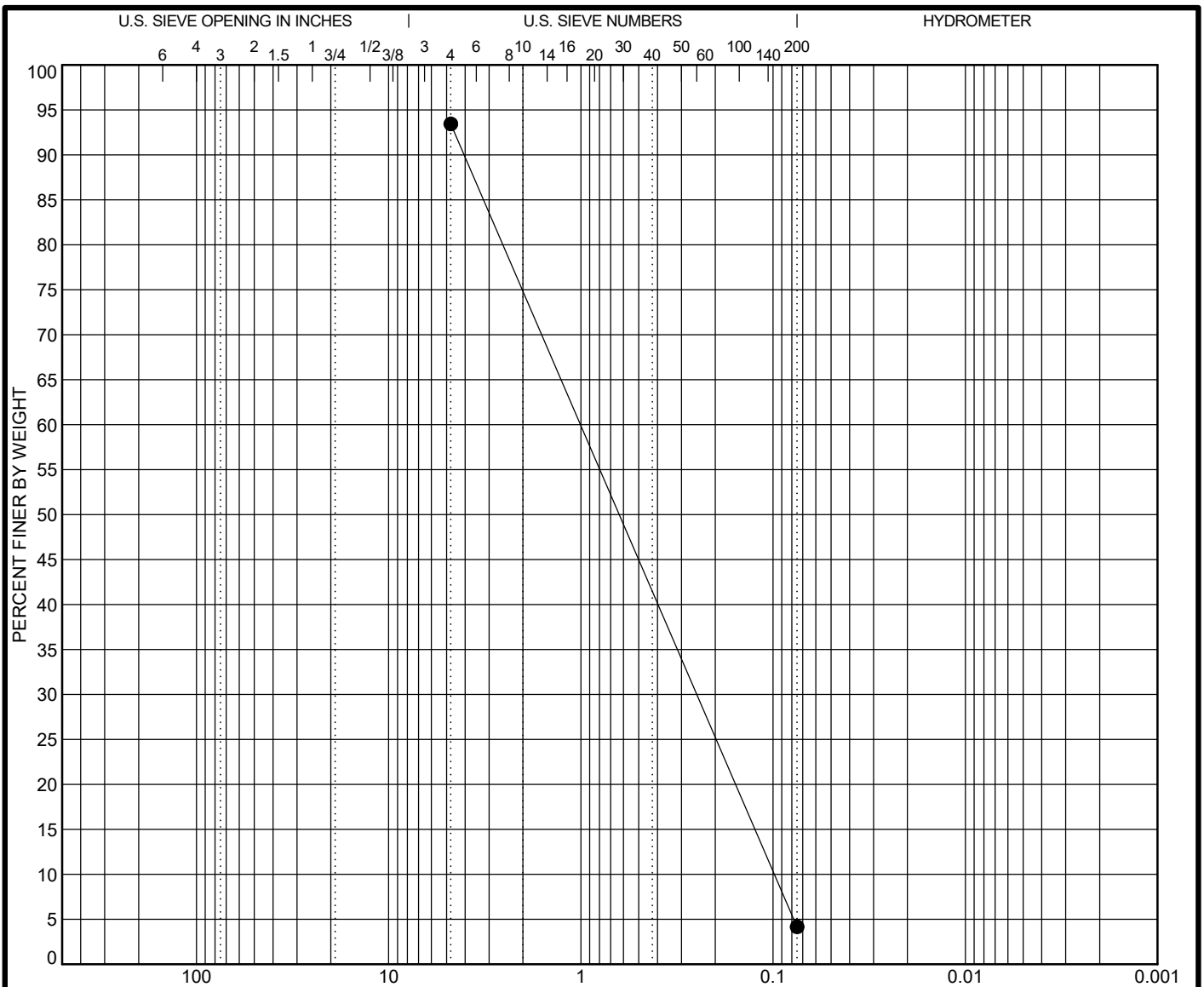
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-06 49.0 ft.	4.75	0.915	0.245	0.102	0.0	94.4	3.1	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-6 S-18;69.0 ft.	POORLY GRADED SAND (SP)								0.63	10.21

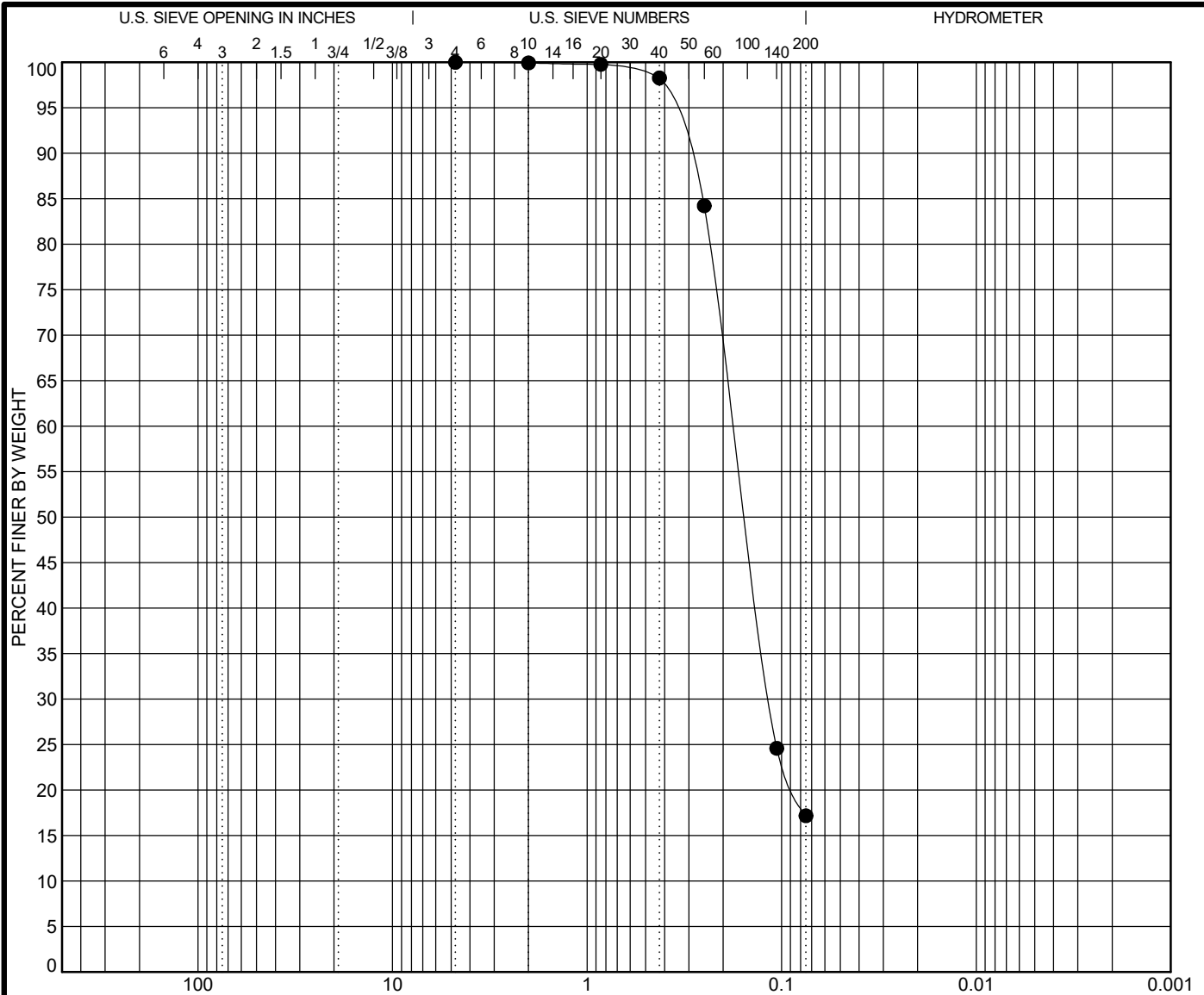
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-06 69.0 ft.	4.75	1.004	0.249	0.098	0.0	89.3	4.2	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● B-7 S-5; 6.5 ft.						

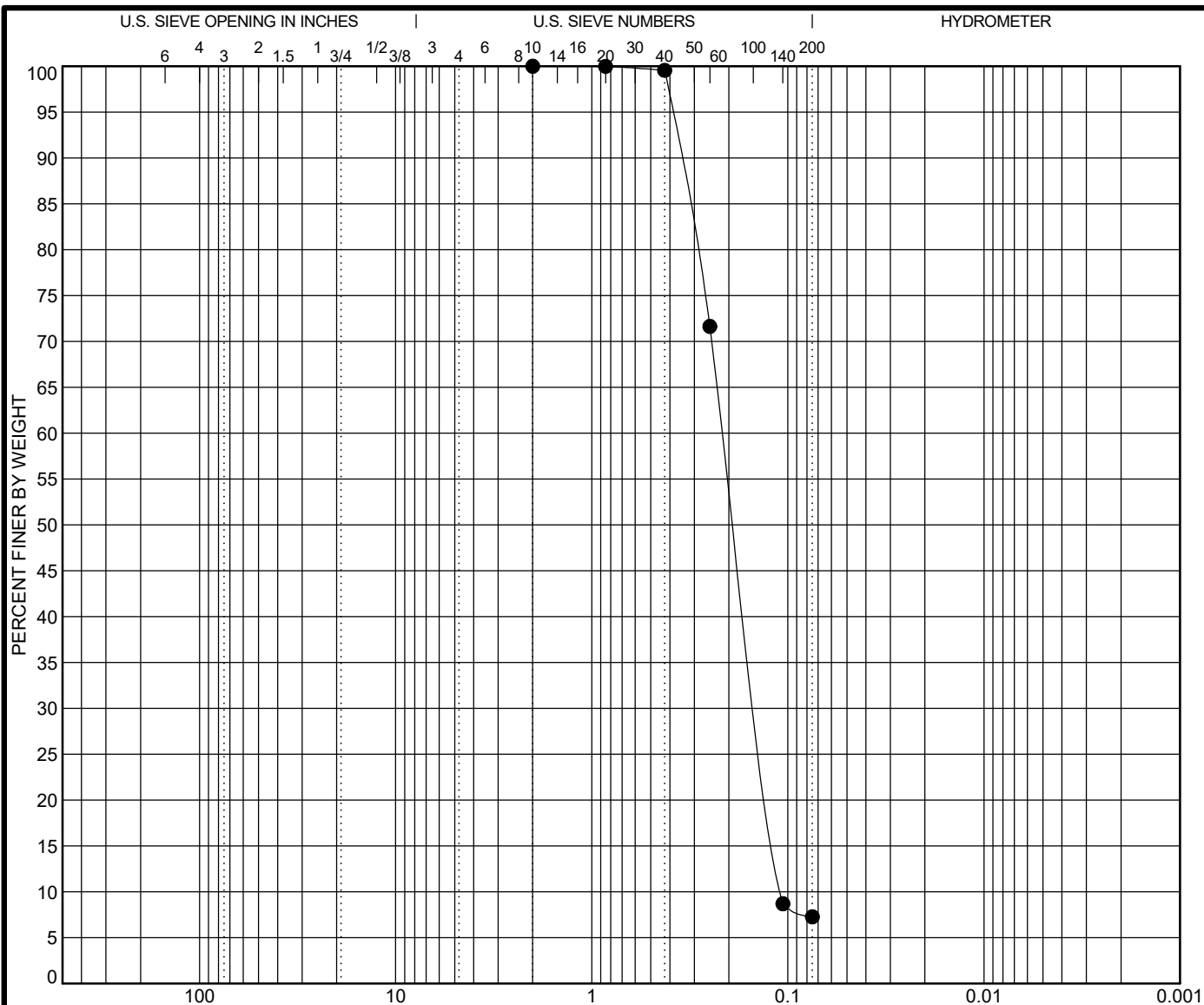
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-07 6.5 ft.	4.75	0.176	0.115		0.0	82.8	17.2	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-7 S-11;34.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	0.87	1.98

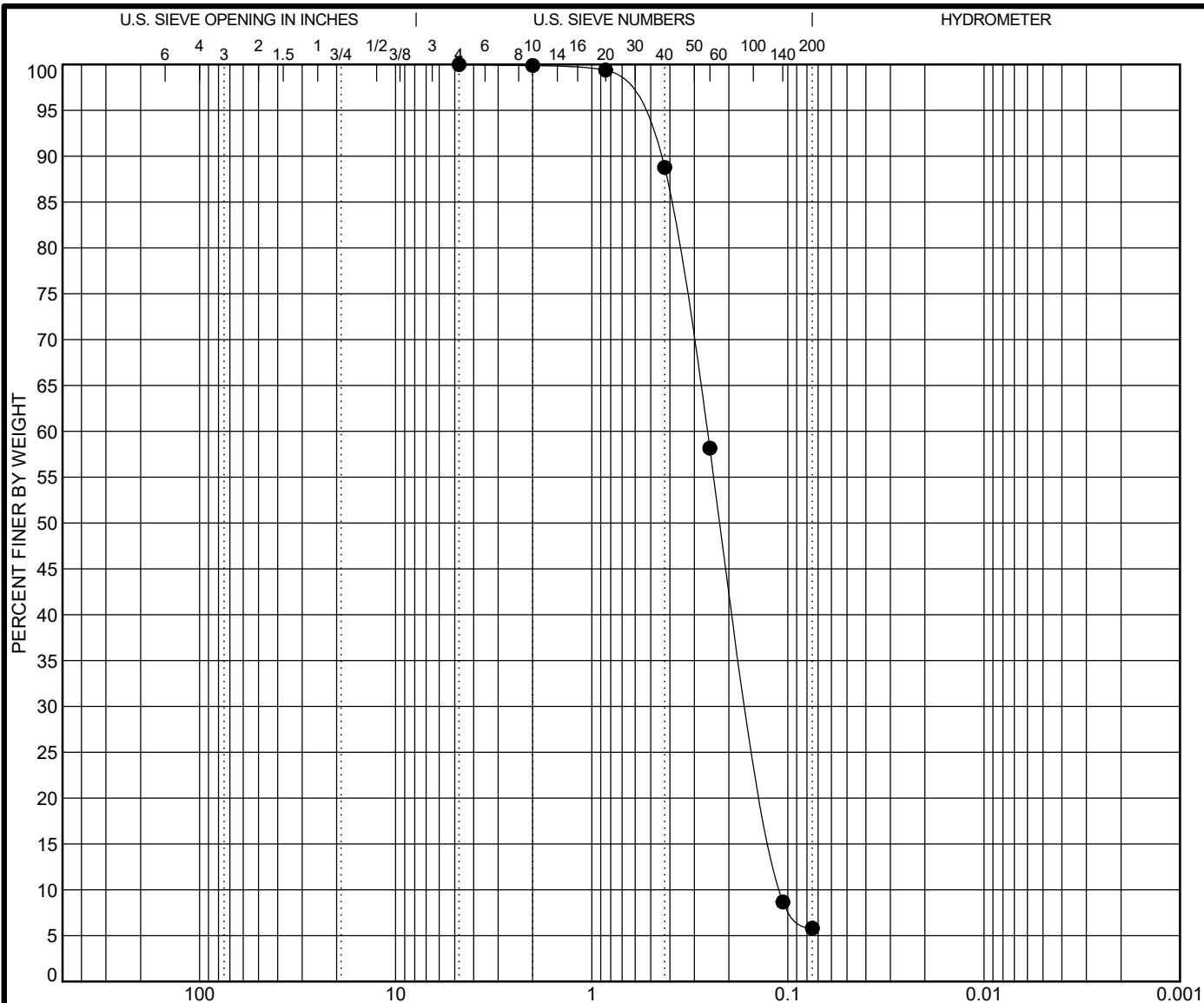
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-07 34.0 ft.	2	0.213	0.142	0.108	0.0	92.7	7.3	

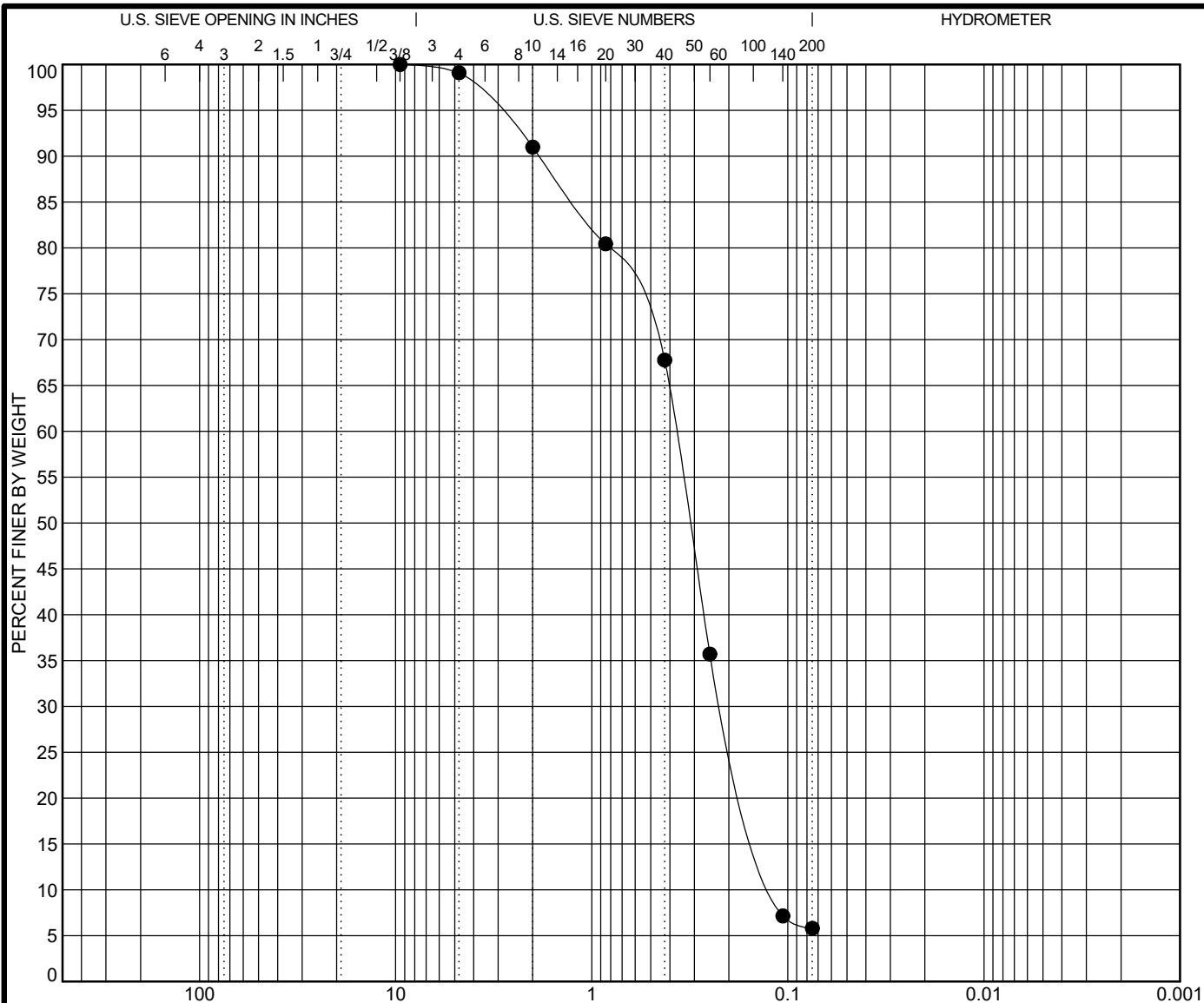
REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL





Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-7 S-18;69.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	1.03	3.24

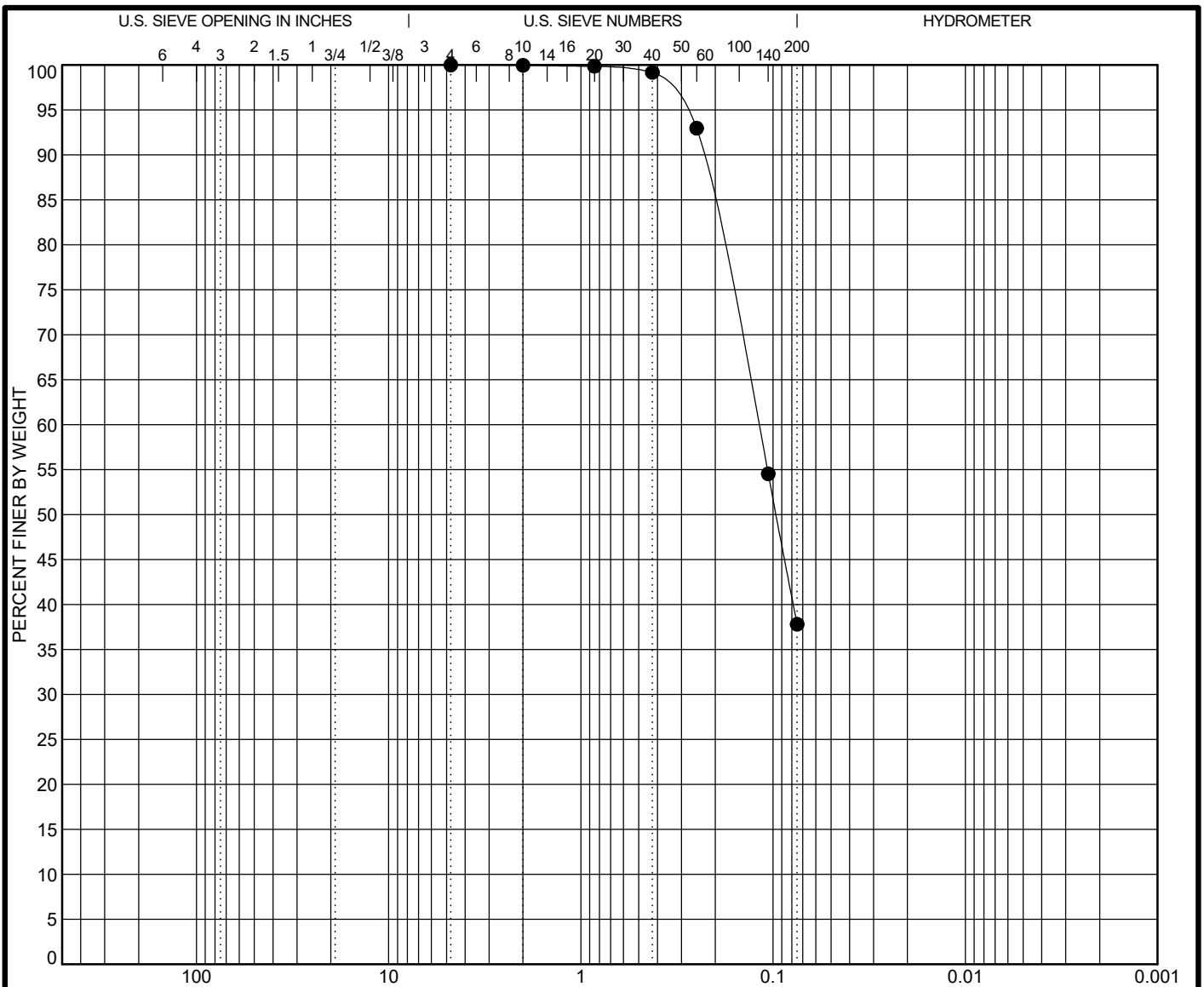
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-07 69.0 ft.	9.5	0.374	0.211	0.115	0.9	93.3	5.8	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-8 S-6;9.0 ft.	CLAYEY SAND (SC)					34	21	13		

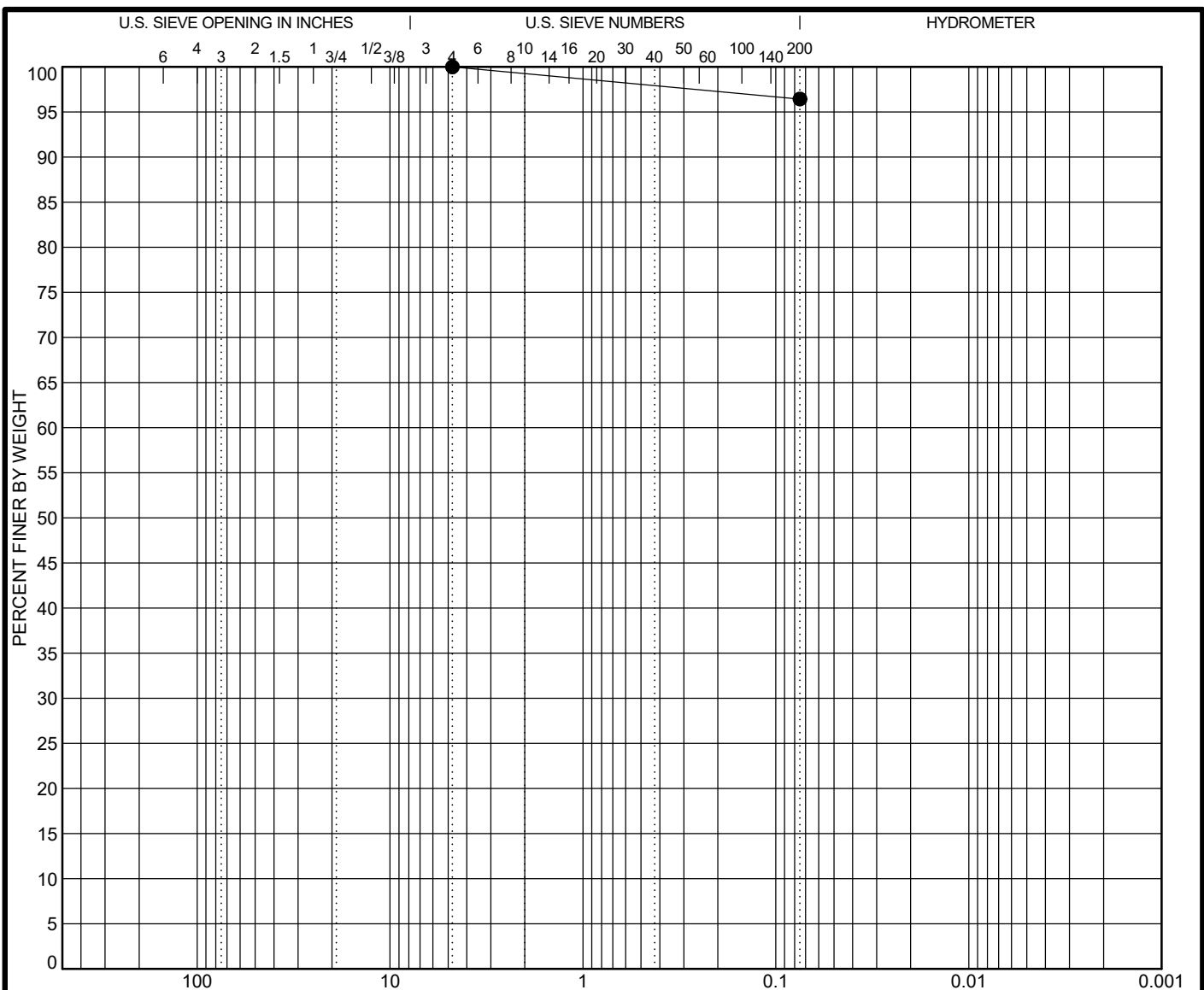
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-08 9.0 ft.	4.75	0.12			0.0	62.2	37.8	

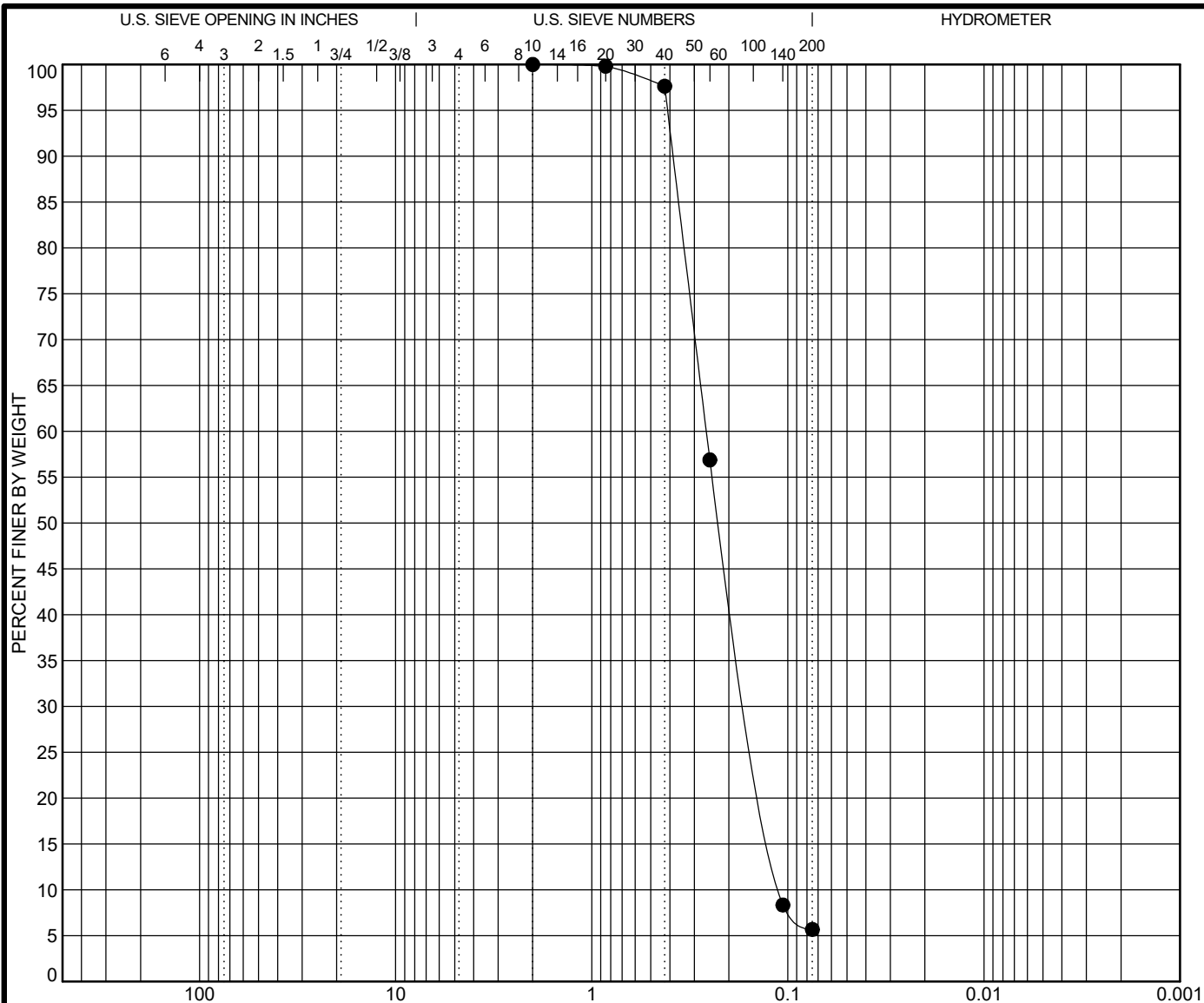
REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL





Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-8 S-12;39.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	0.85	2.39

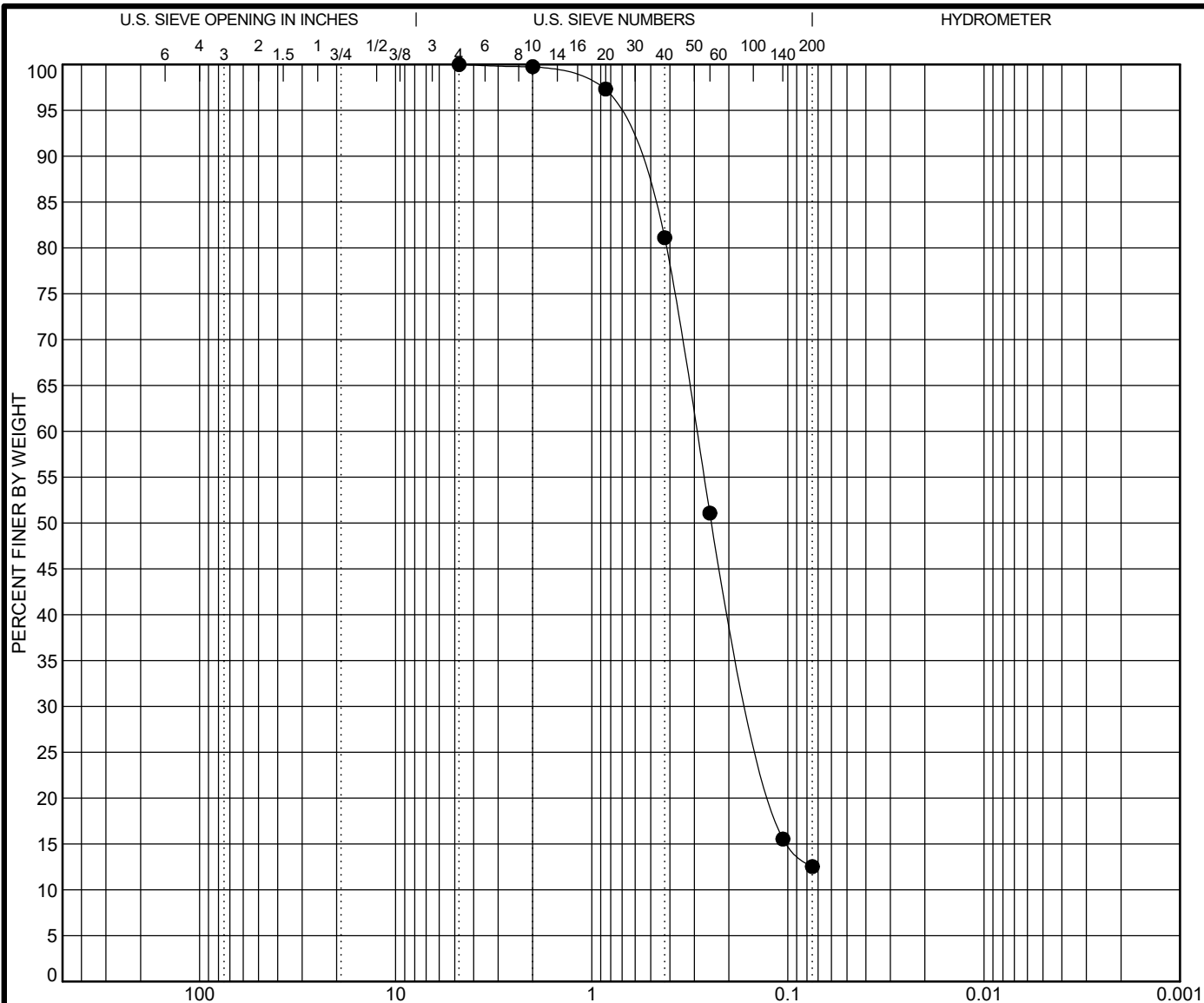
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-08 39.0 ft.	2	0.26	0.155	0.109	0.0	94.3	5.7	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● B-8 S-15;54.0 ft.	SILTY SAND (SM)	NP	NP	NP		

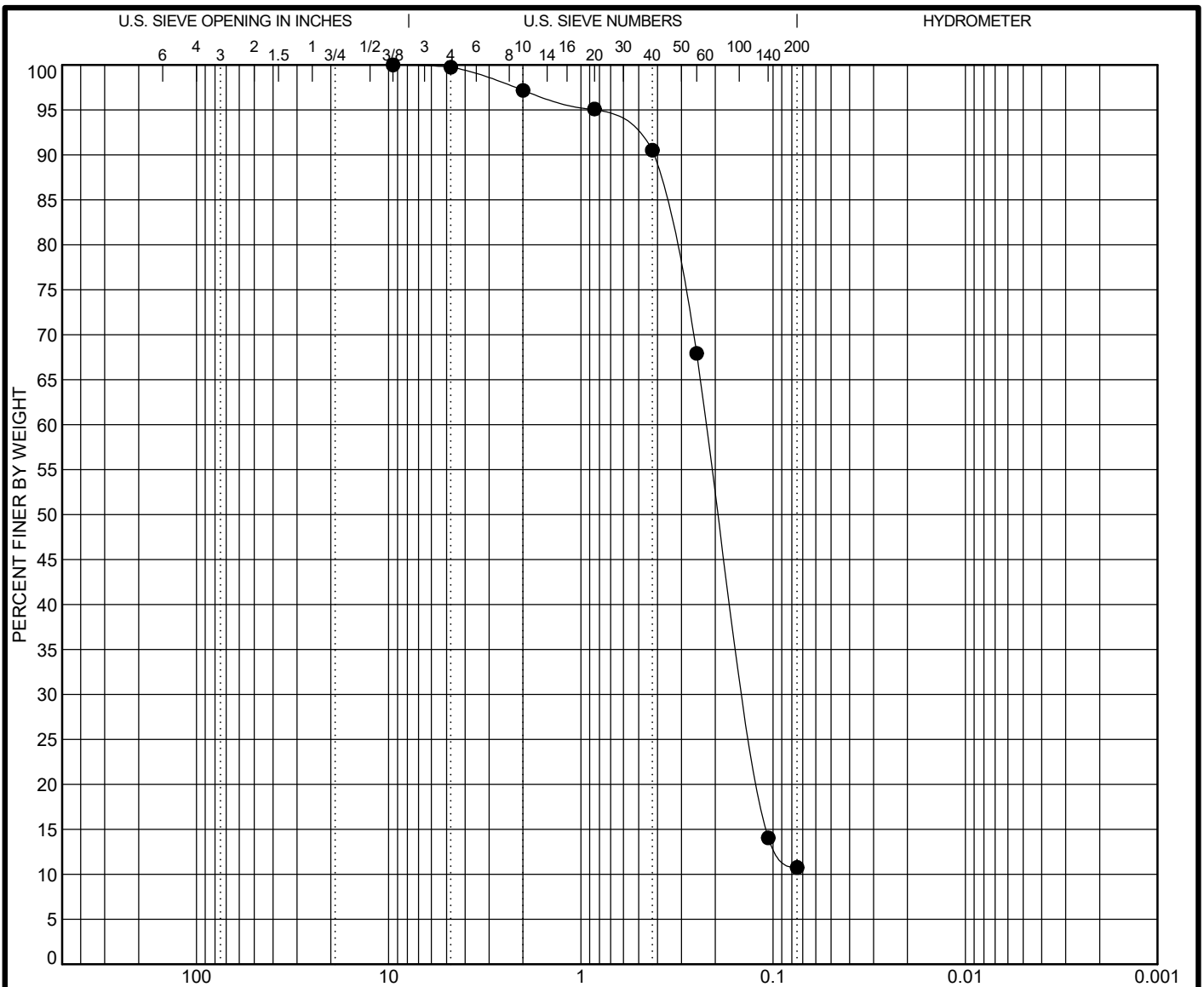
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-08 54.0 ft.	4.75	0.293	0.15		0.0	87.5	12.5	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-8 S-18;69.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	1.22	3.18

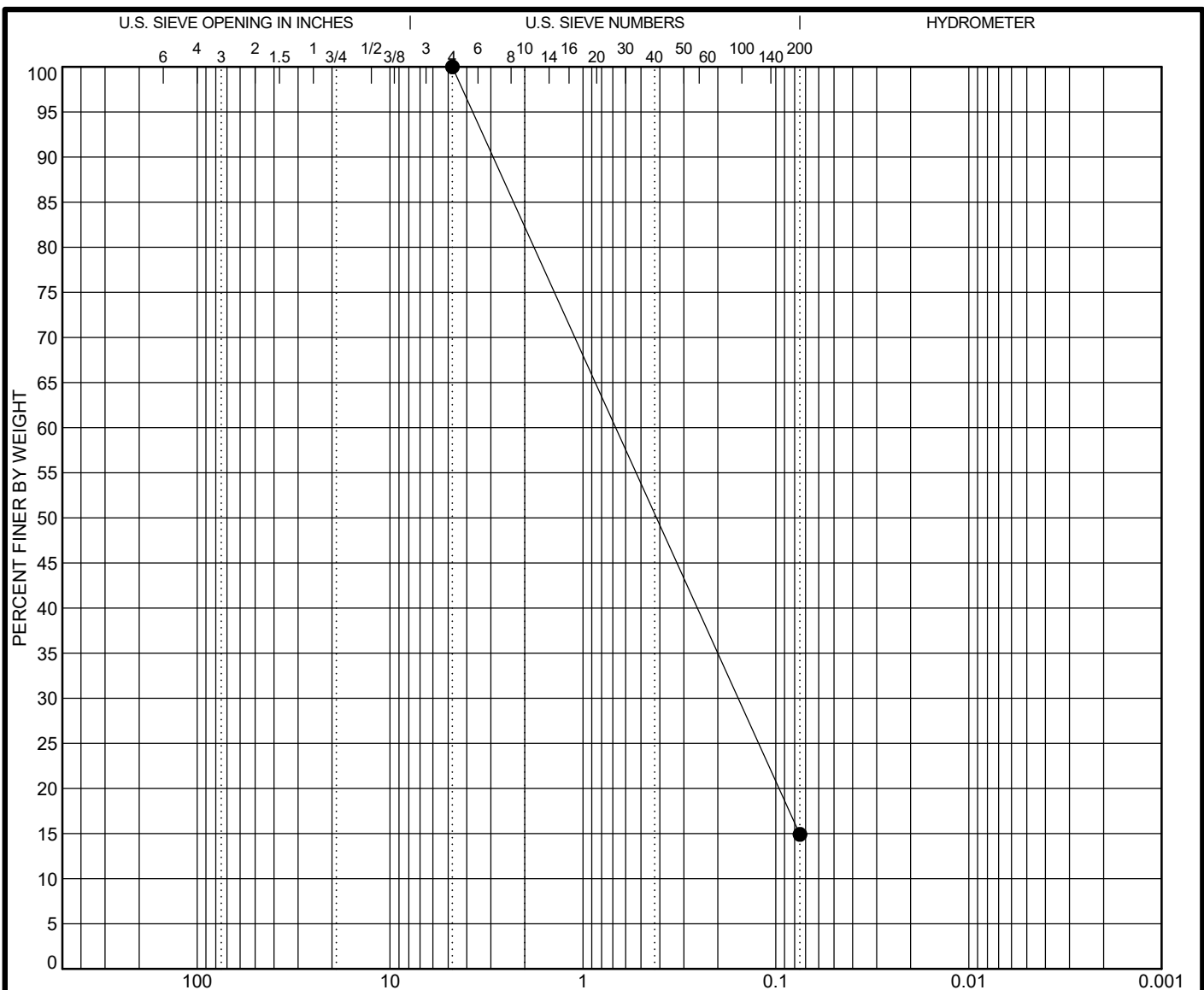
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-08 69.0 ft.	9.5	0.22	0.137		0.2	89.0	10.8	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● B-9 S-5;6.5 ft.	SILTY SAND (SM)	NP	NP	NP		

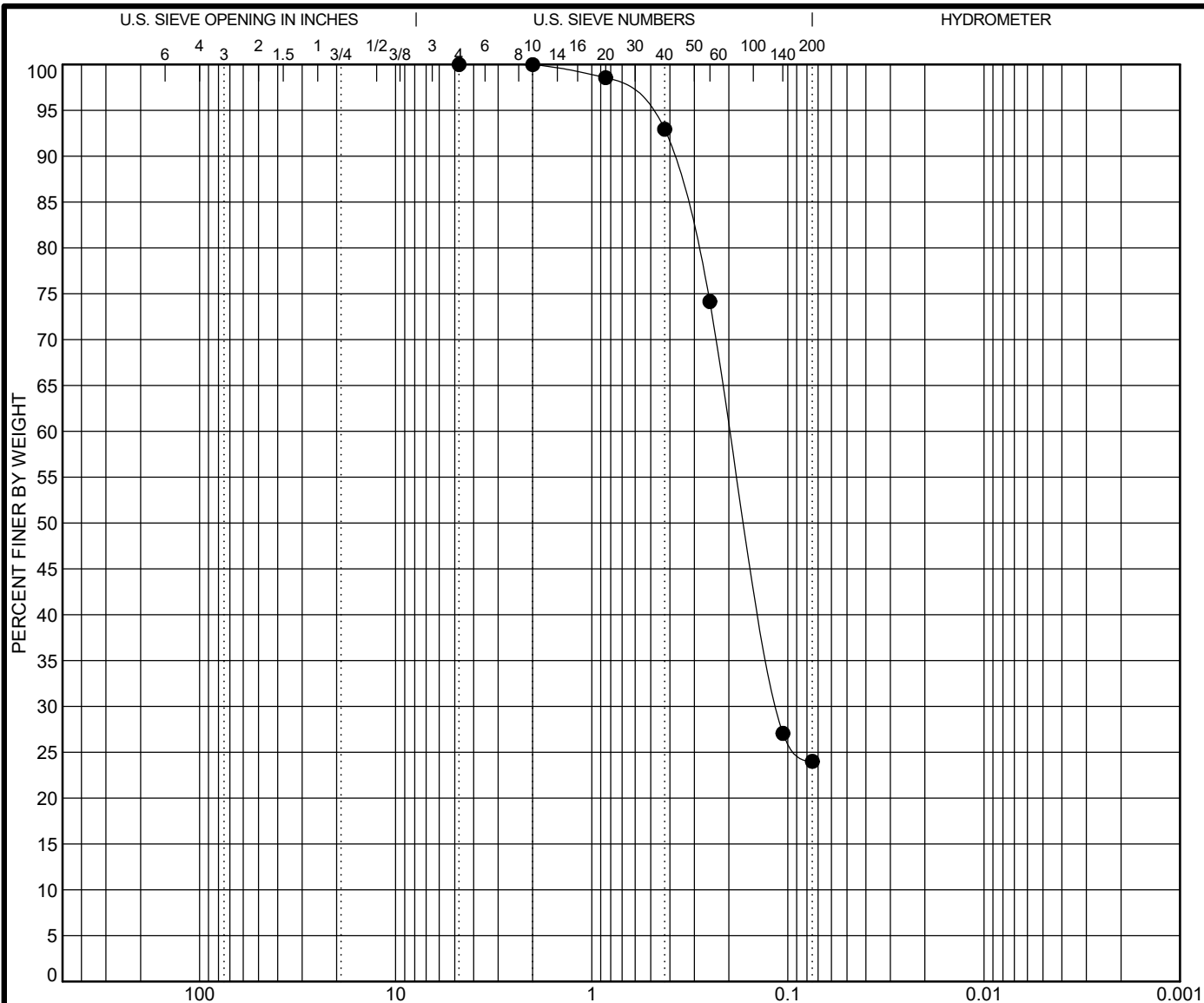
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-09 6.5 ft.	4.75	0.676	0.157		0.0	85.1	14.9	

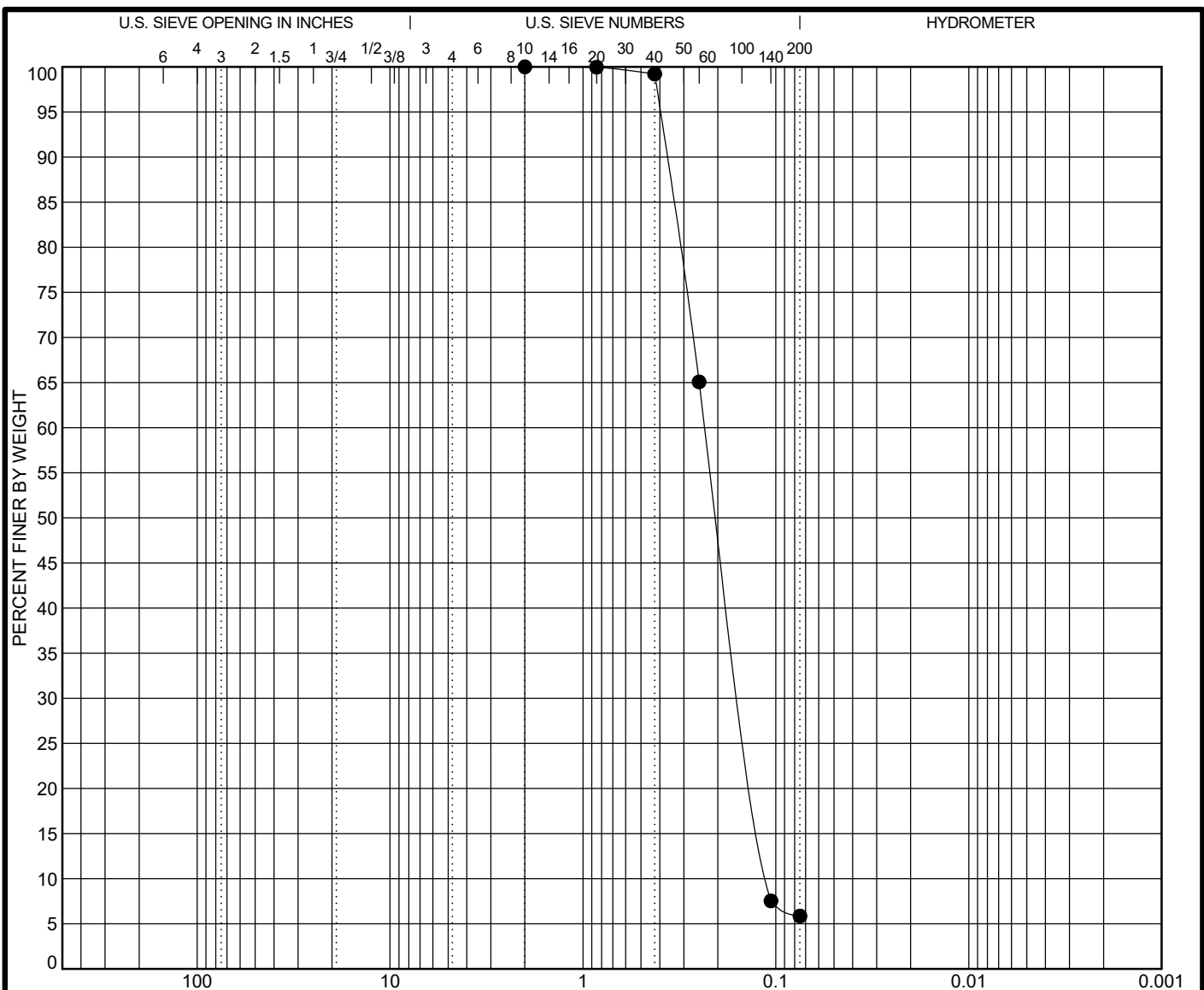
REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL





Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-9 S-10;29.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	0.86	2.11

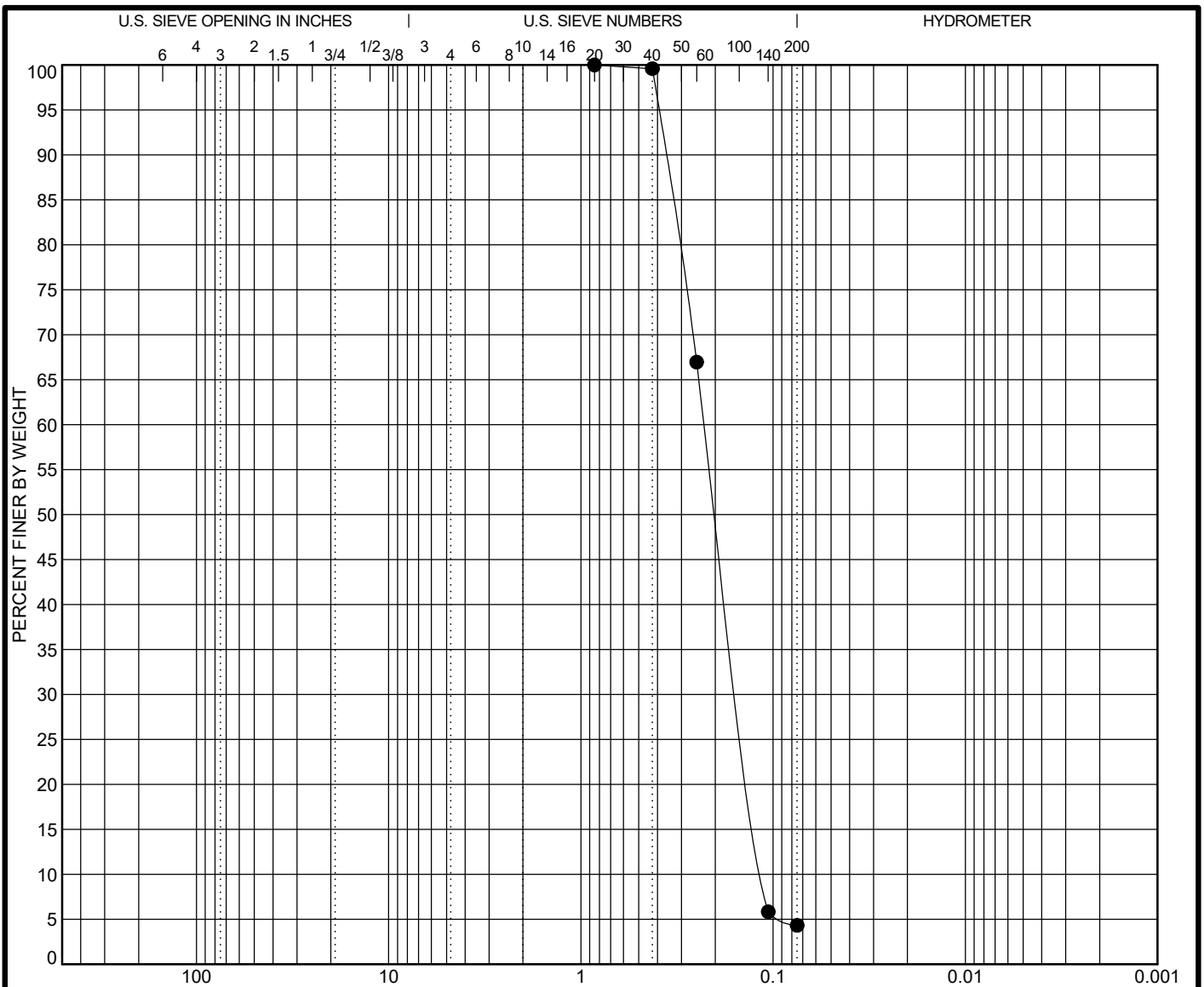
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-09 29.0 ft.	2	0.232	0.148	0.11	0.0	94.1	5.9	

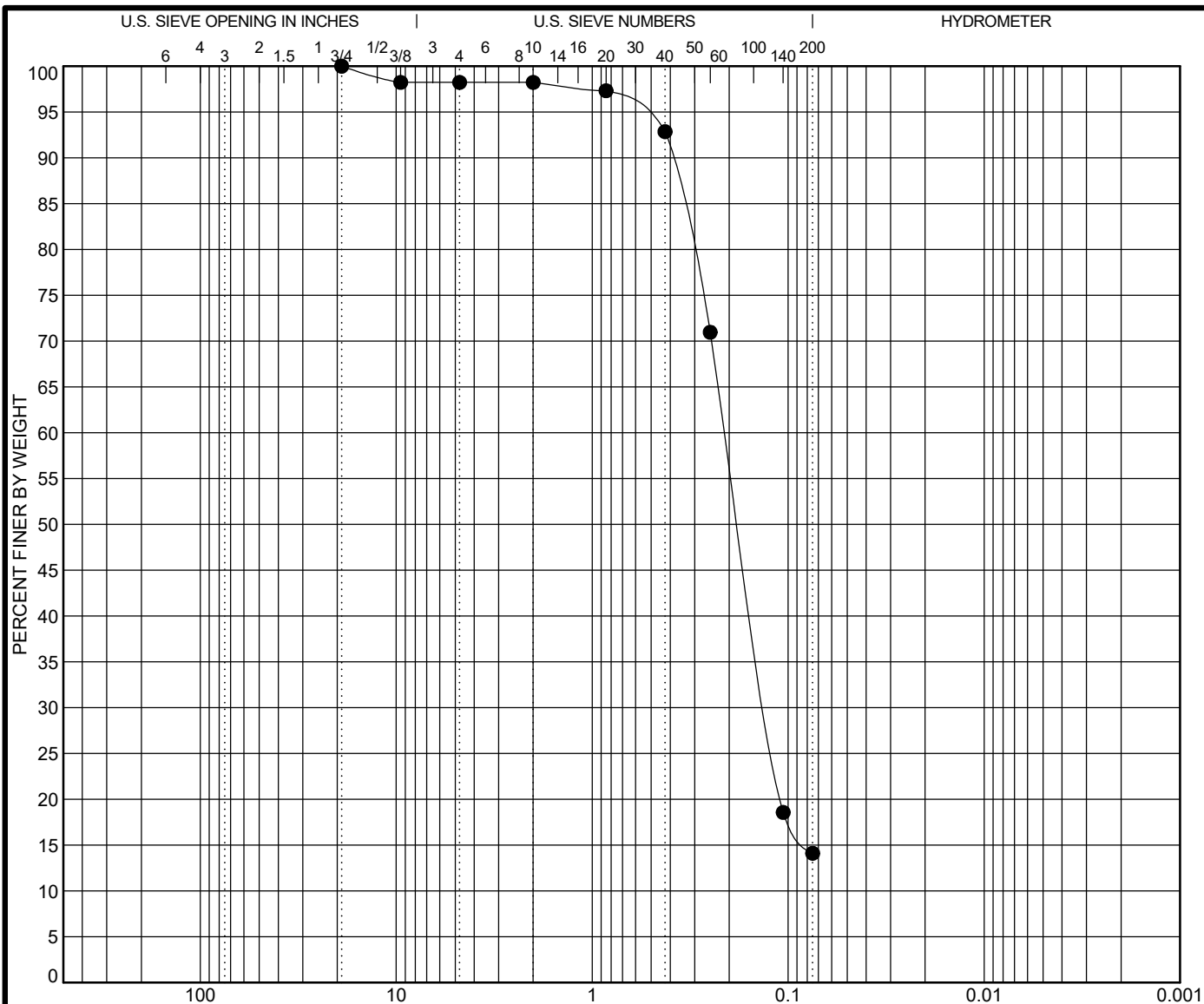
REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL





Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● B-9 S-15;54.0 ft.	SILTY SAND (SM)	NP	NP	NP		

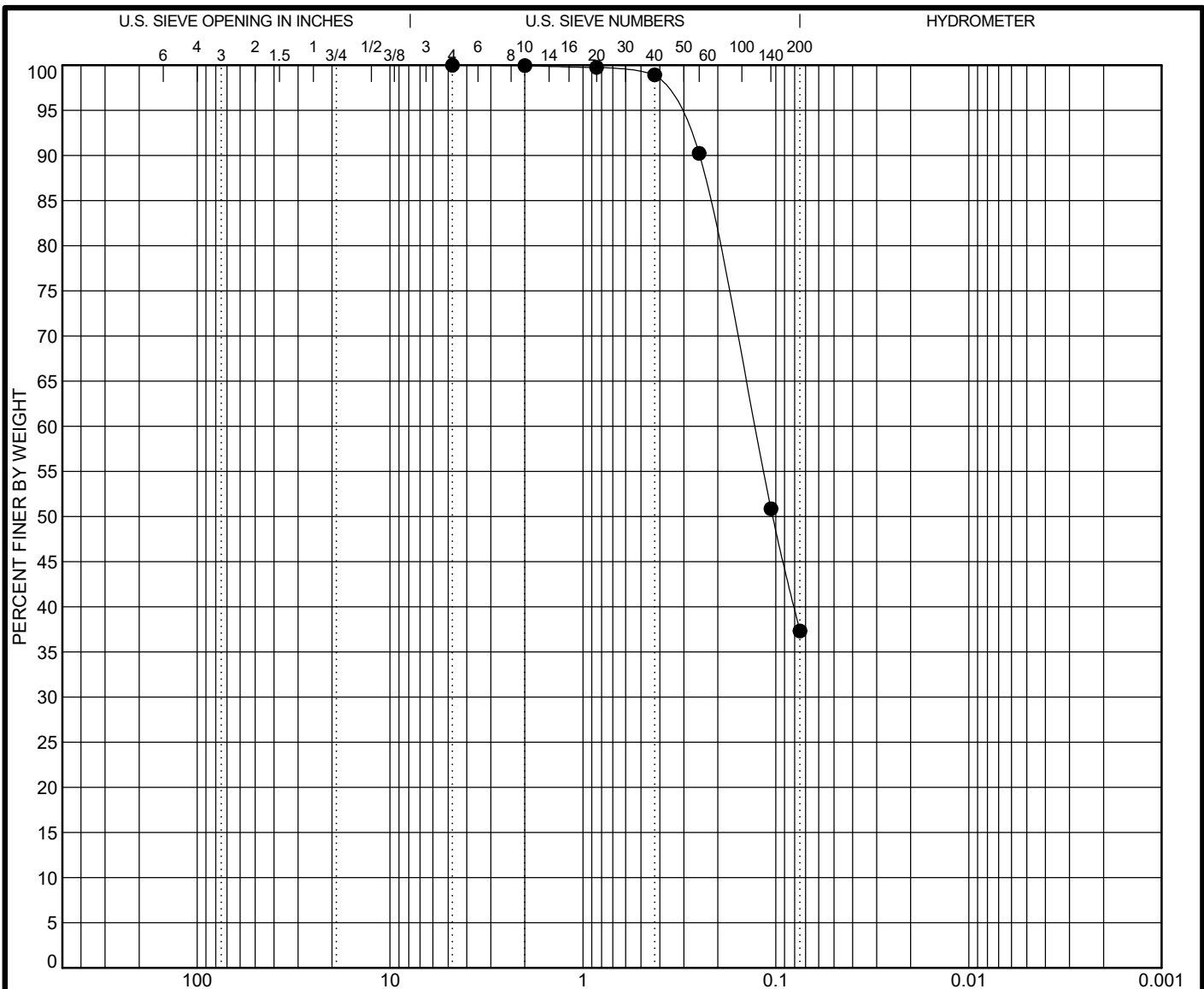
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-09 54.0 ft.	19	0.209	0.128		1.8	84.1	14.1	

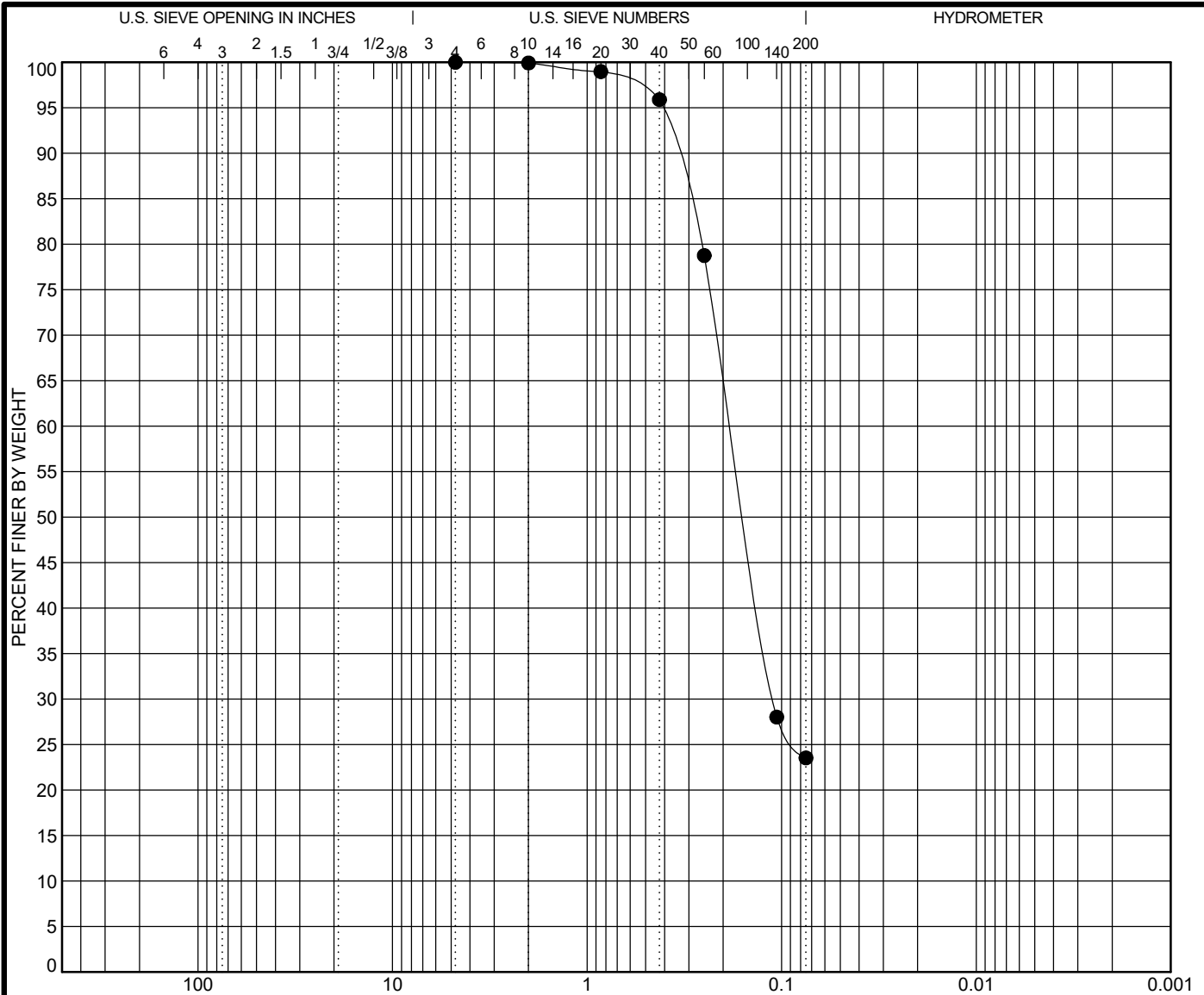
REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL





Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● B-10 S-7;14.0 ft.						

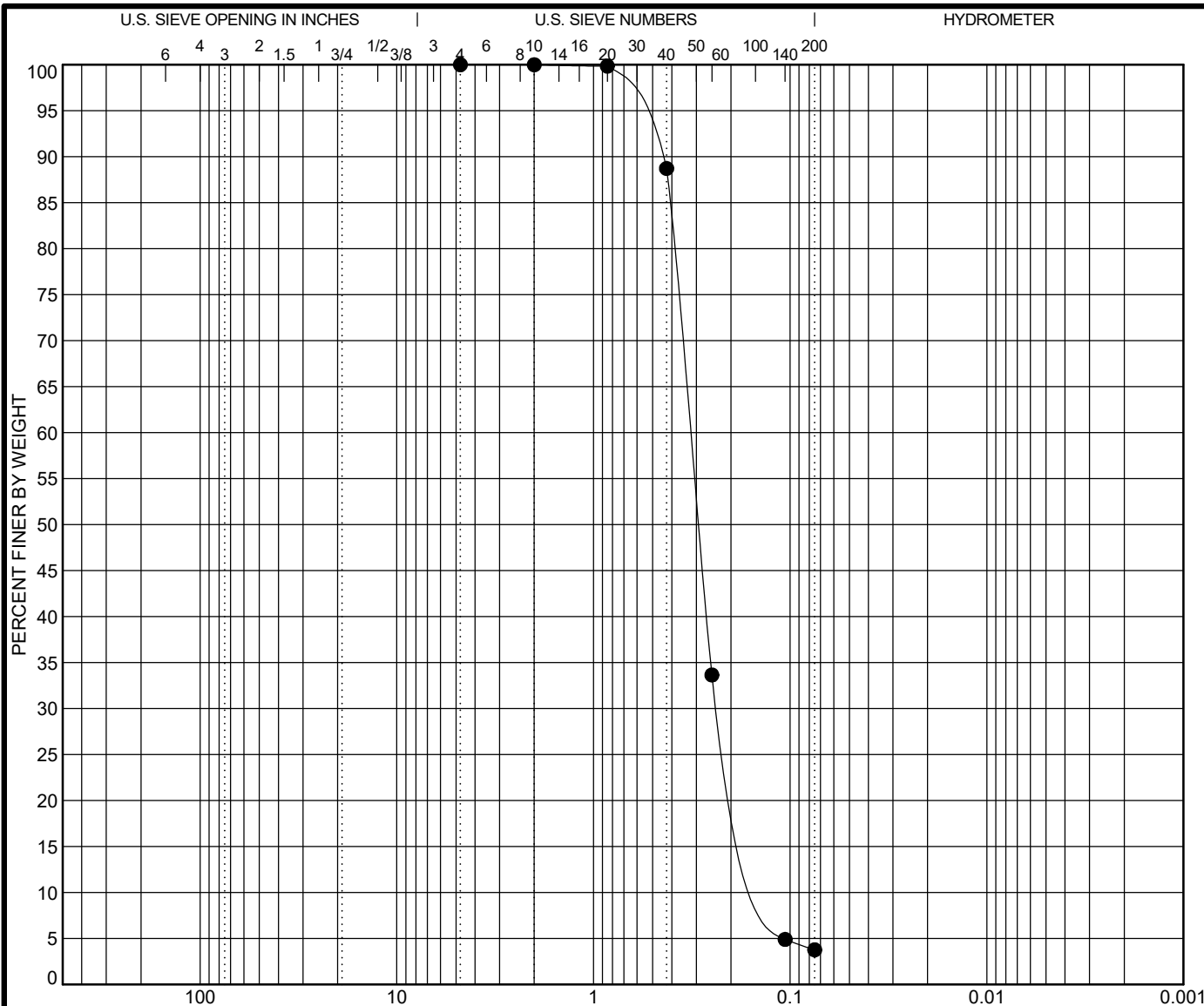
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-10 14.0 ft.	4.75	0.182	0.11		0.0	76.5	23.5	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-10 S-9;24.0 ft.	POORLY GRADED SAND (SP)					NP	NP	NP	1.26	2.61

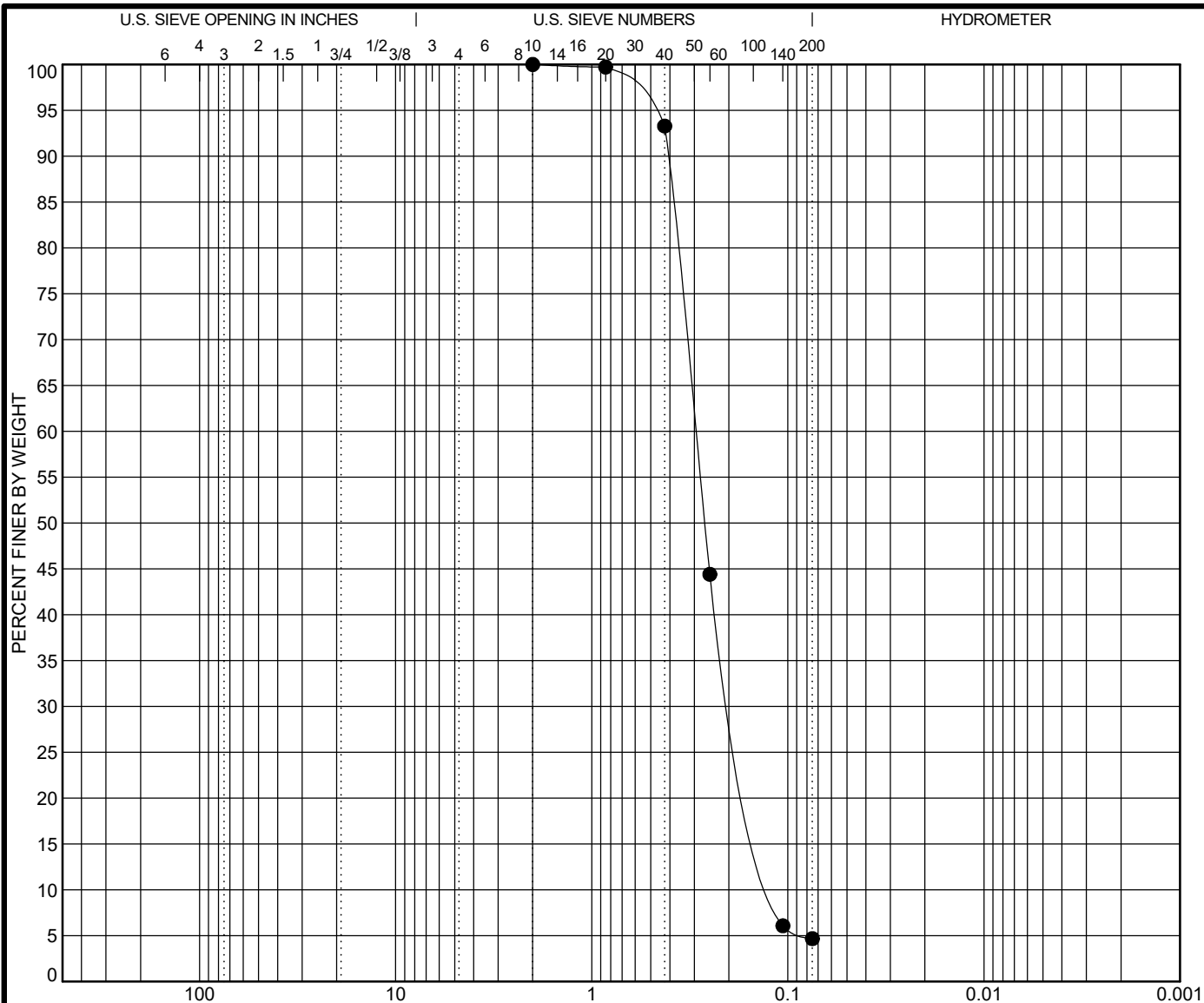
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-10 24.0 ft.	4.75	0.322	0.224	0.123	0.0	96.2	3.8	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● B-10 S-11;34.0 ft.	POORLY GRADED SAND (SP)	NP	NP	NP	0.96	2.56

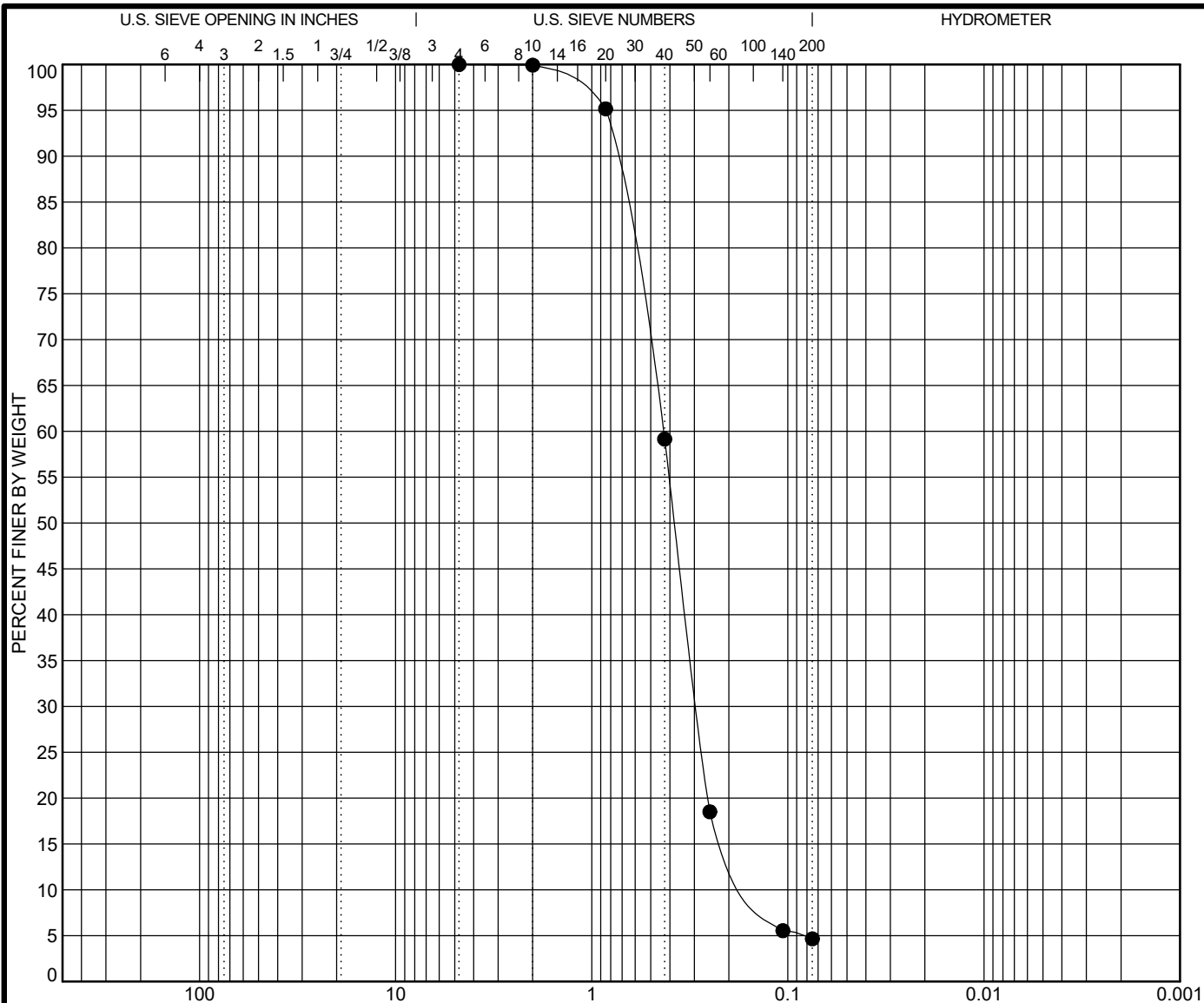
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-10 34.0 ft.	2	0.296	0.181	0.116	0.0	95.3	4.7	

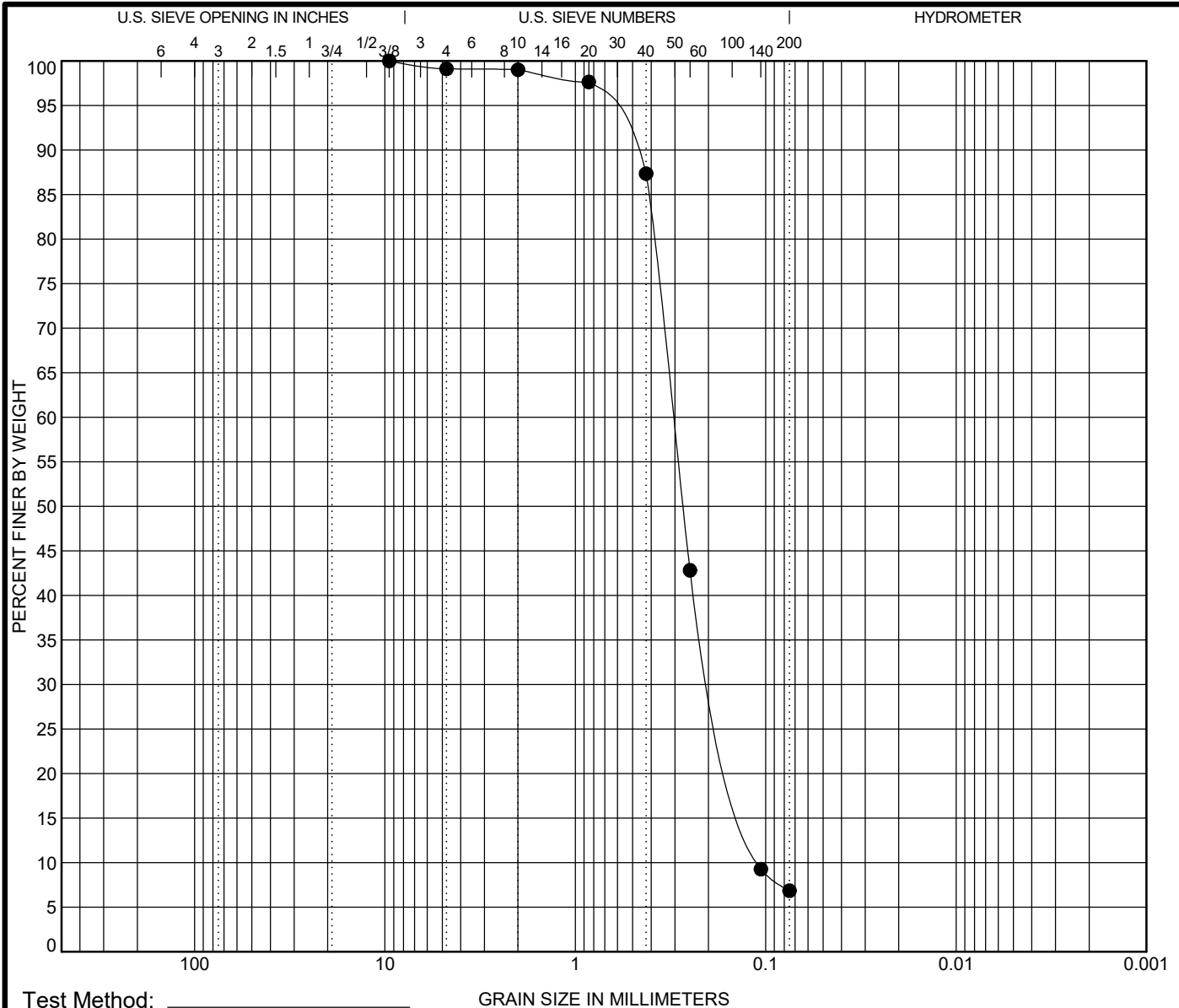
REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL





COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification			Classification	LL	PL	PI	Cc	Cu
●	B-10	S-15; 54.0 ft.	POORLY GRADED SAND with SILT (SP-SM)	NP	NP	NP	0.98	2.84

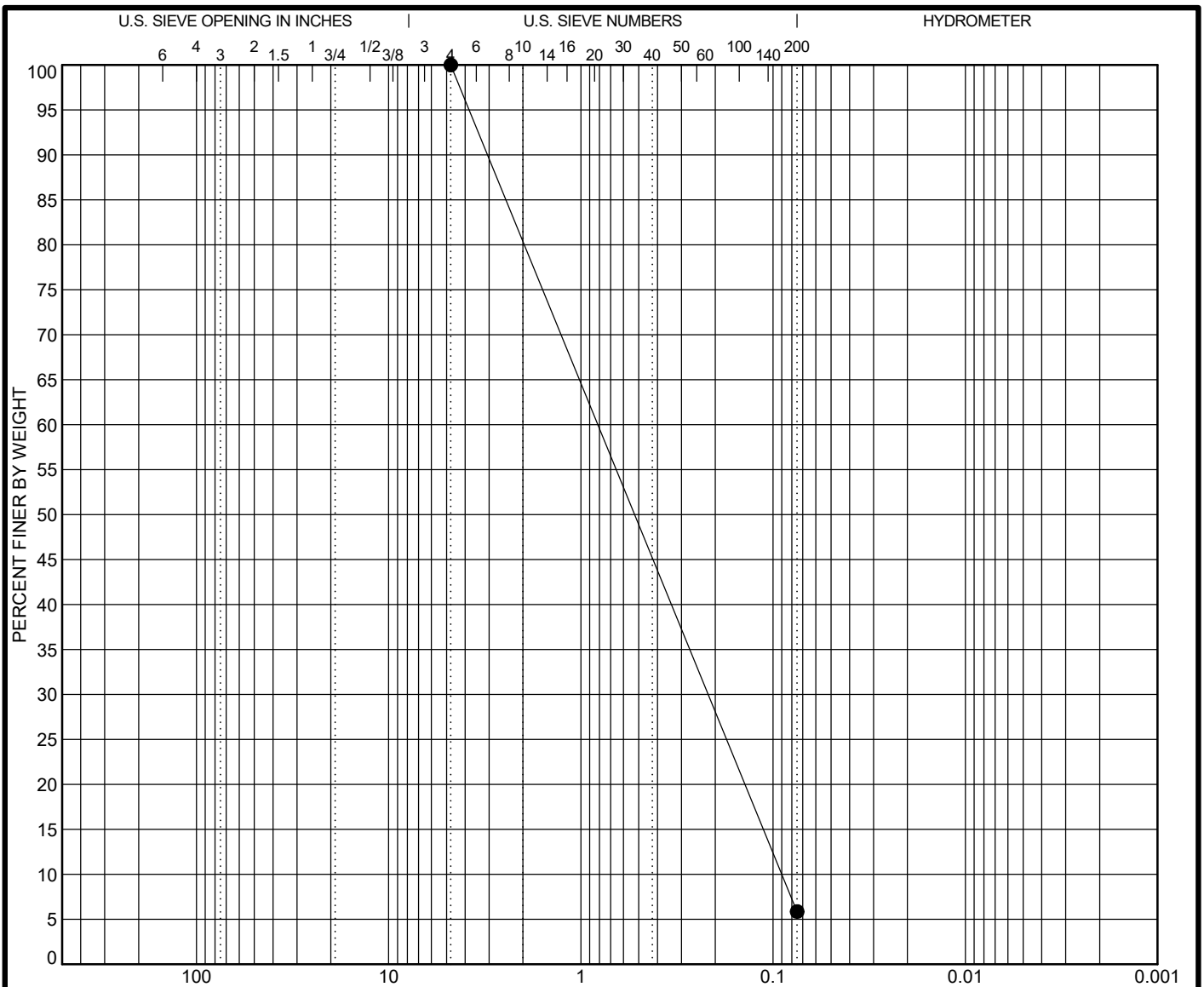
Specimen Identification		D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
	B-10	54.0 ft.	9.5	0.307	0.18	0.108	0.9	92.3	6.8

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-10 S-17;64.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	0.64	9.05

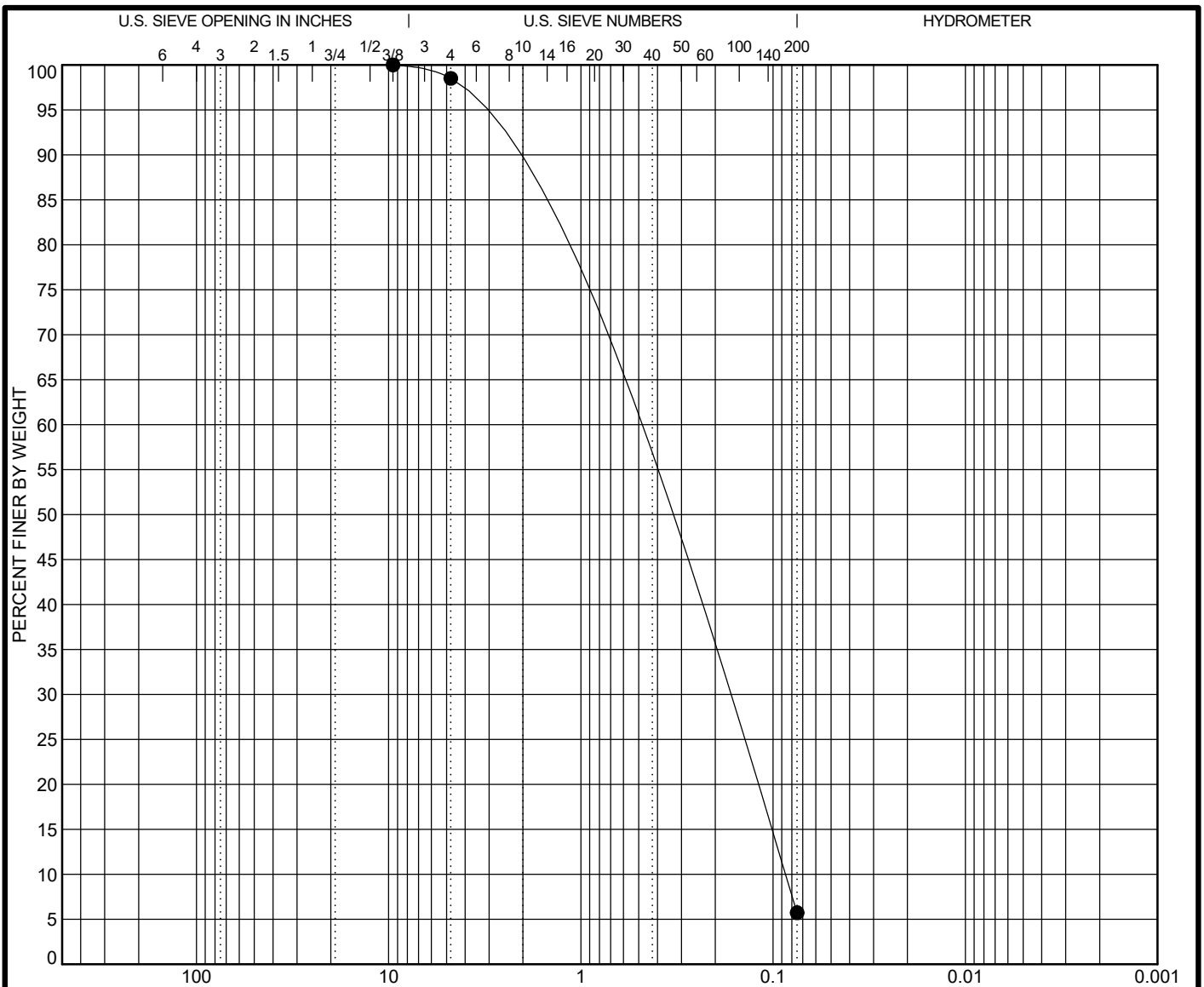
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-10 64.0 ft.	4.75	0.815	0.217	0.09	0.0	94.1	5.9	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-10 S-22;89.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	0.64	9.35

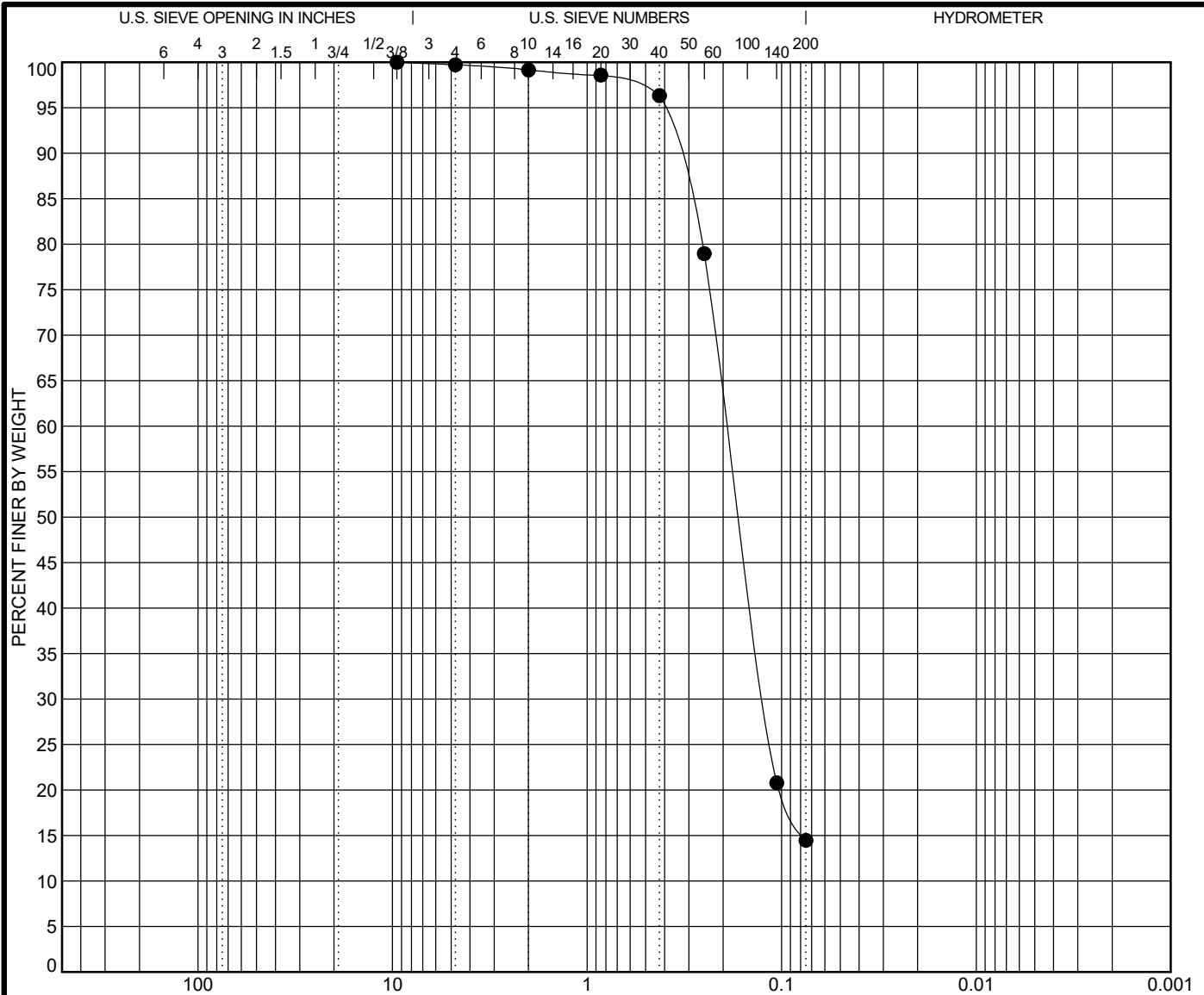
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-10 89.0 ft.	9.5	0.849	0.222	0.091	1.5	92.8	5.7	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● B-11 S-4;5.0 ft.	SILTY SAND (SM)	NP	NP	NP		

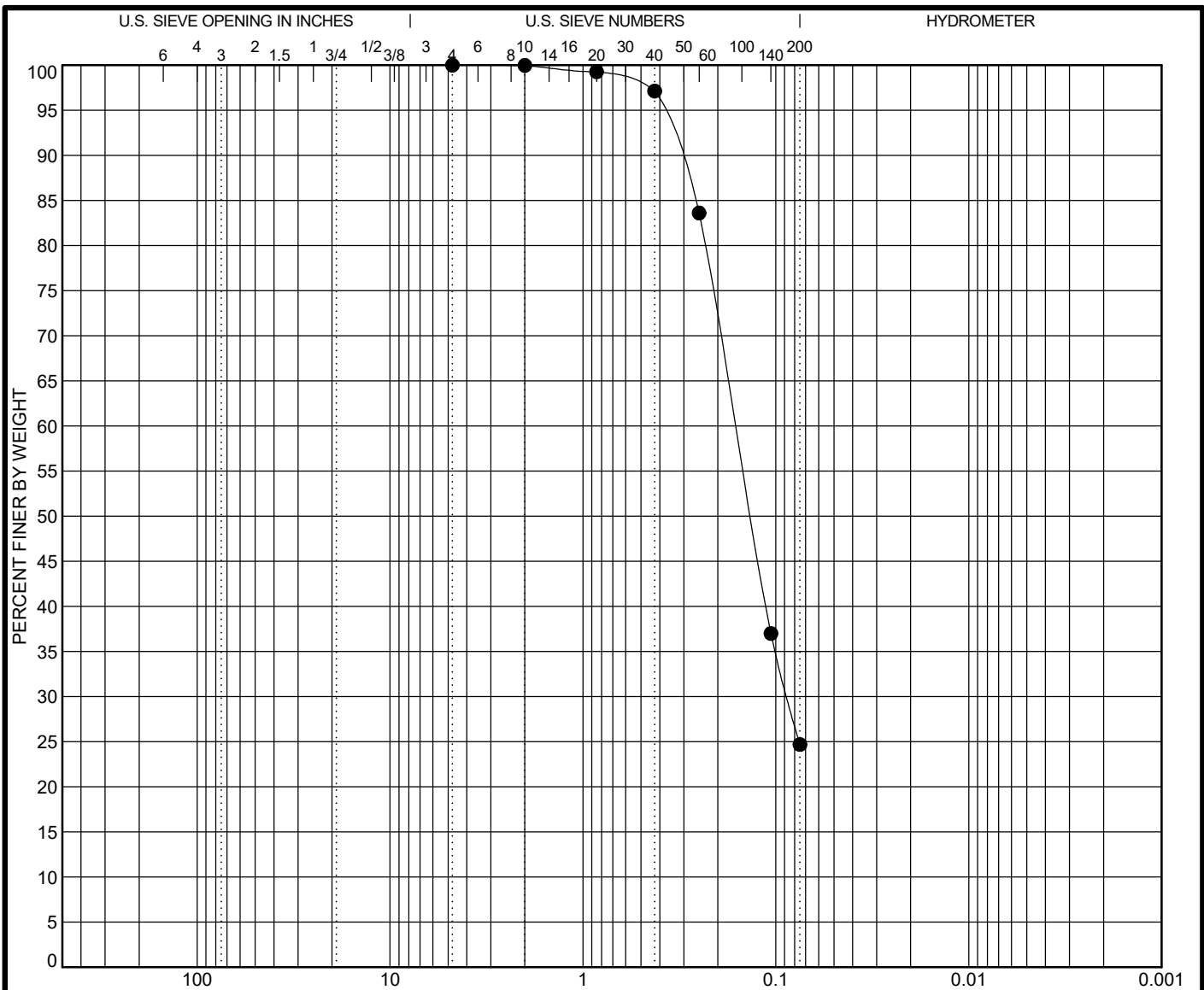
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-11 5.0 ft.	9.5	0.189	0.121		0.3	85.3	14.5	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● B-11 S-7;14.0 ft.	CLAYEY SAND (SC)	25	17	8		

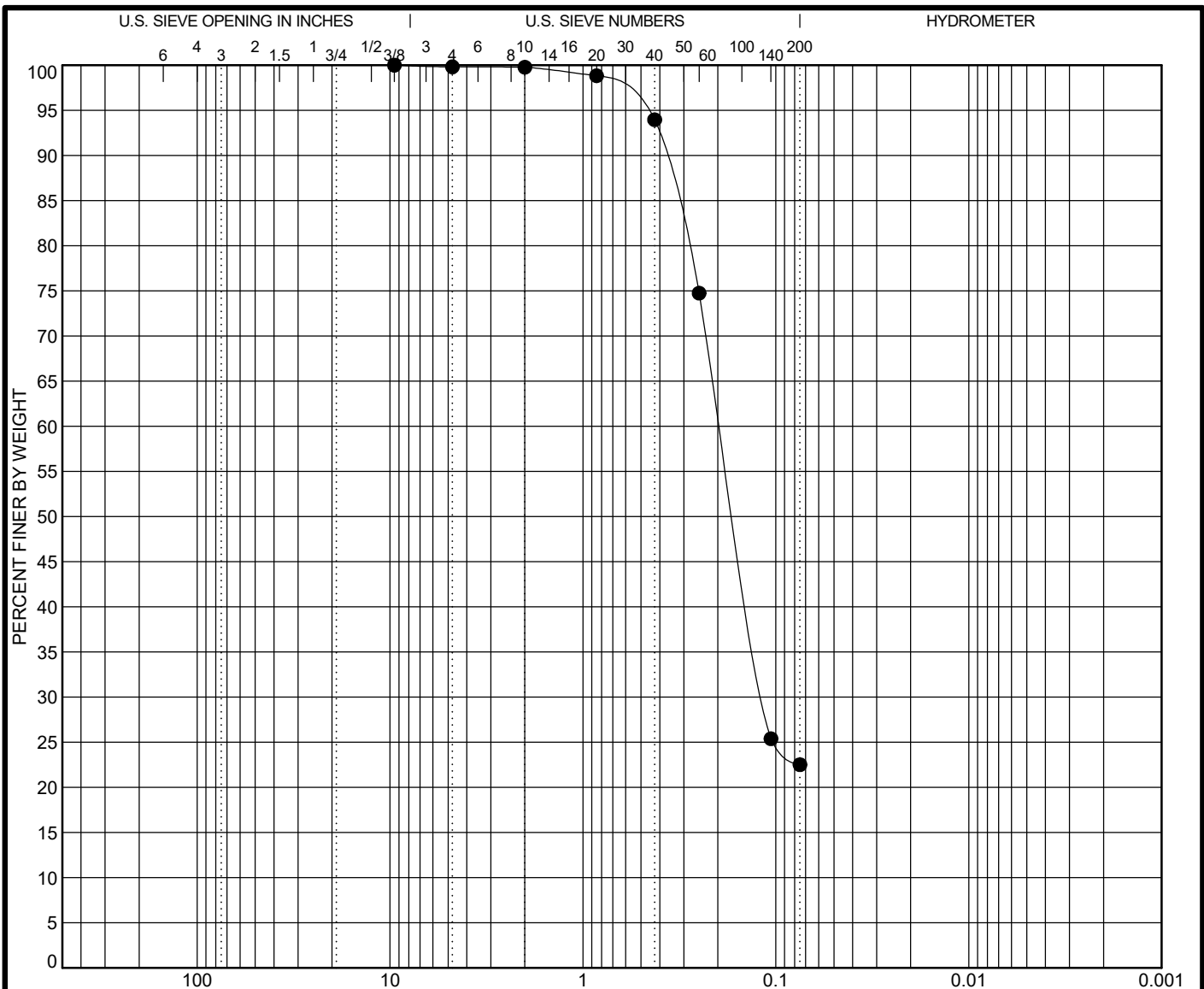
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-11 14.0 ft.	4.75	0.162	0.087		0.0	75.3	24.7	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● B-11 S-8;19.0 ft.	CLAYEY SAND (SC)	28	17	11		

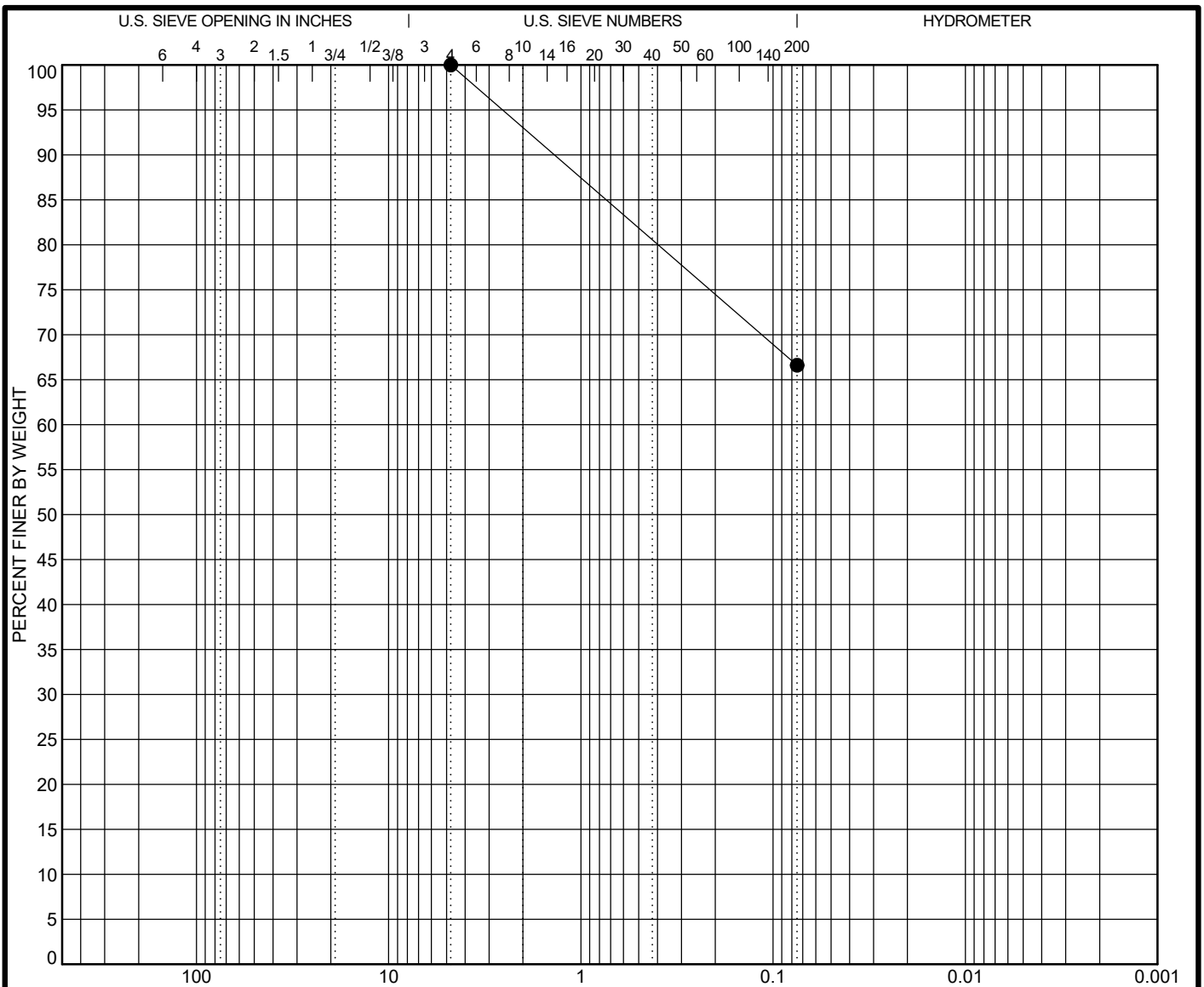
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-11 19.0 ft.	9.5	0.193	0.115		0.2	77.3	22.5	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● B-11 T-1;26.5 ft.	SANDY ELASTIC SILT (MH)	150	68	82		

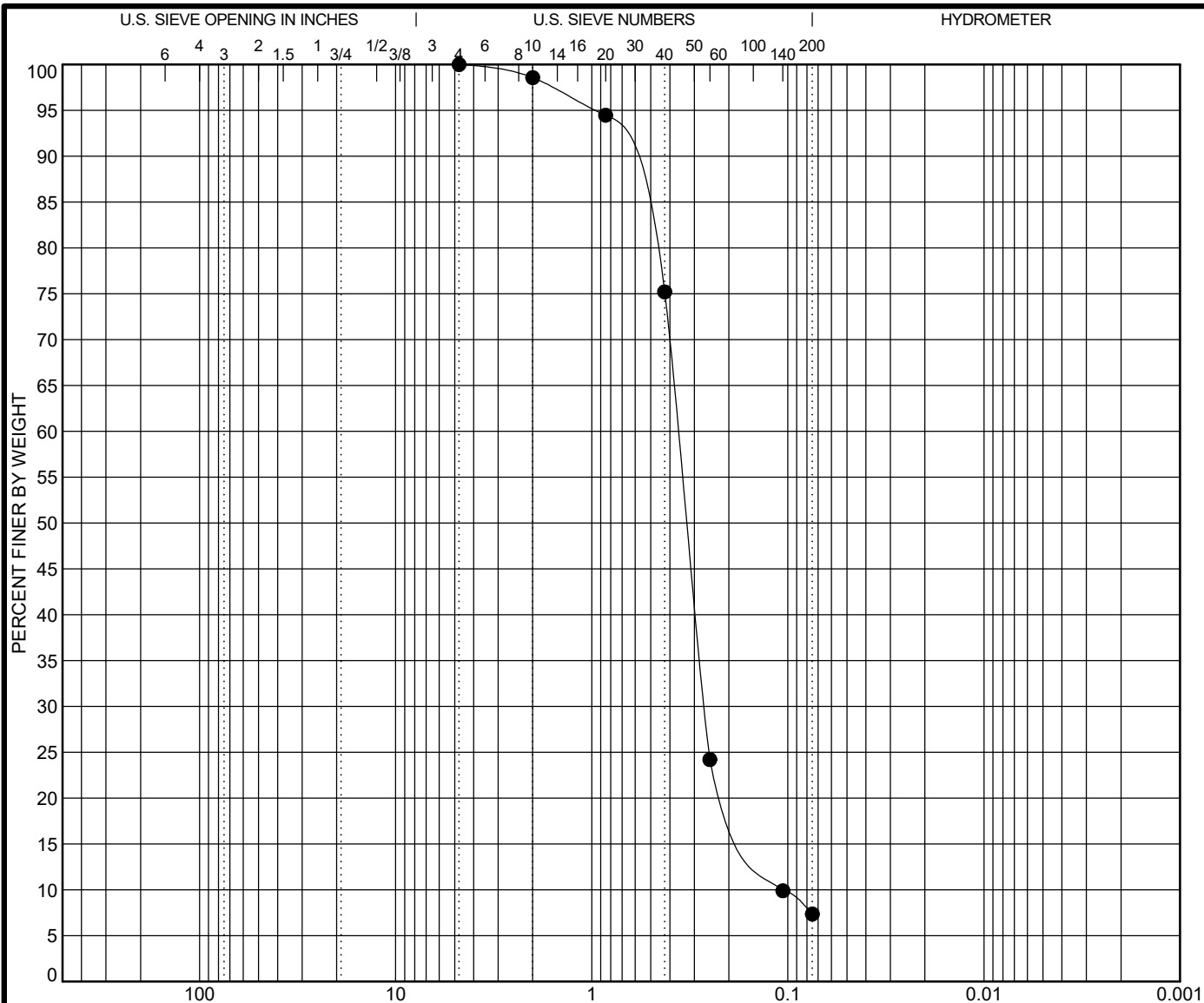
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-11 26.5 ft.	4.75				0.0	33.4	66.6	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-11 S-16;59.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	1.82	3.40

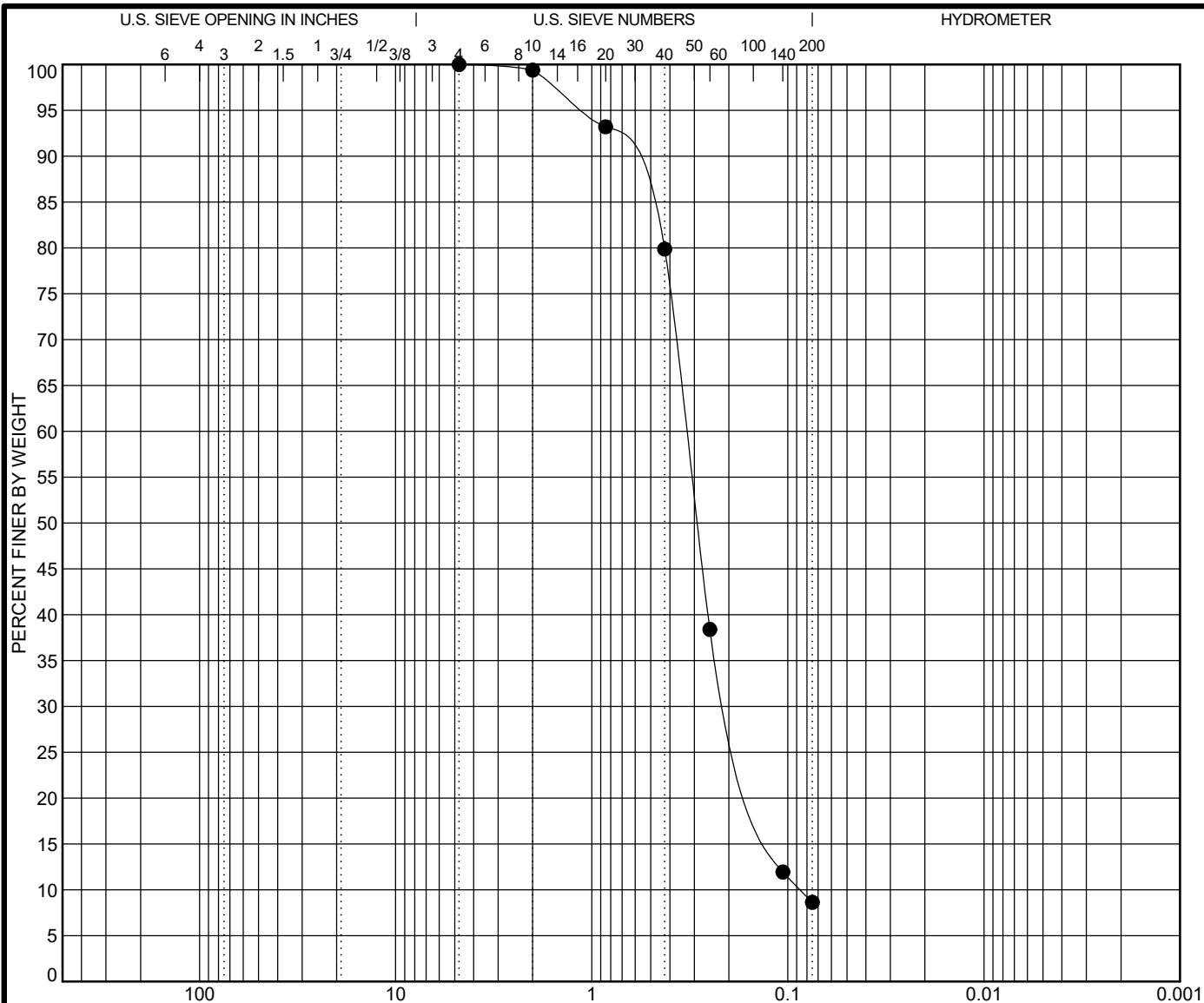
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-11 59.0 ft.	4.75	0.363	0.266	0.107	0.0	92.6	7.4	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-11 S-20;79.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	1.27	3.81

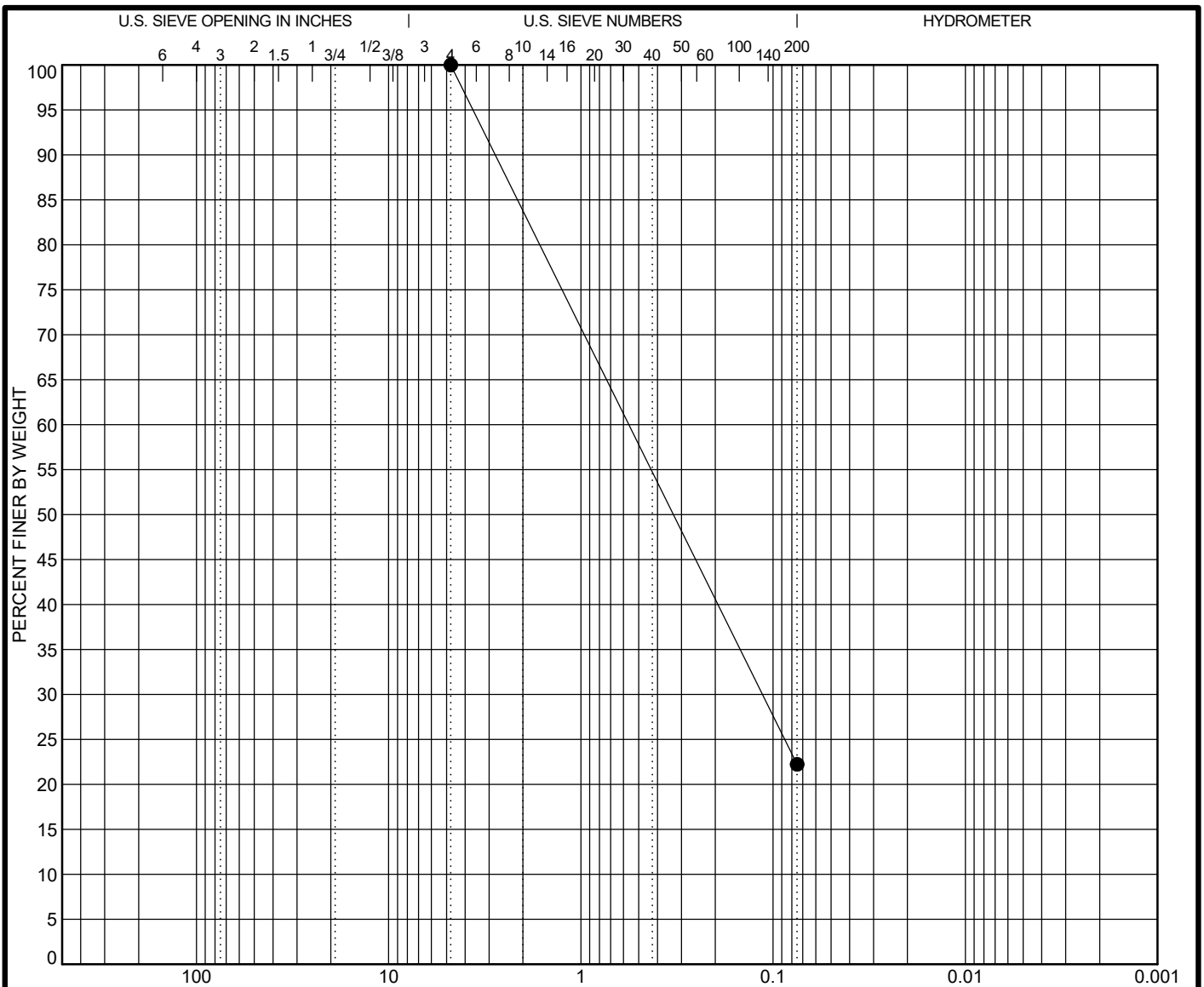
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-11 79.0 ft.	4.75	0.33	0.19	0.087	0.0	91.4	8.6	

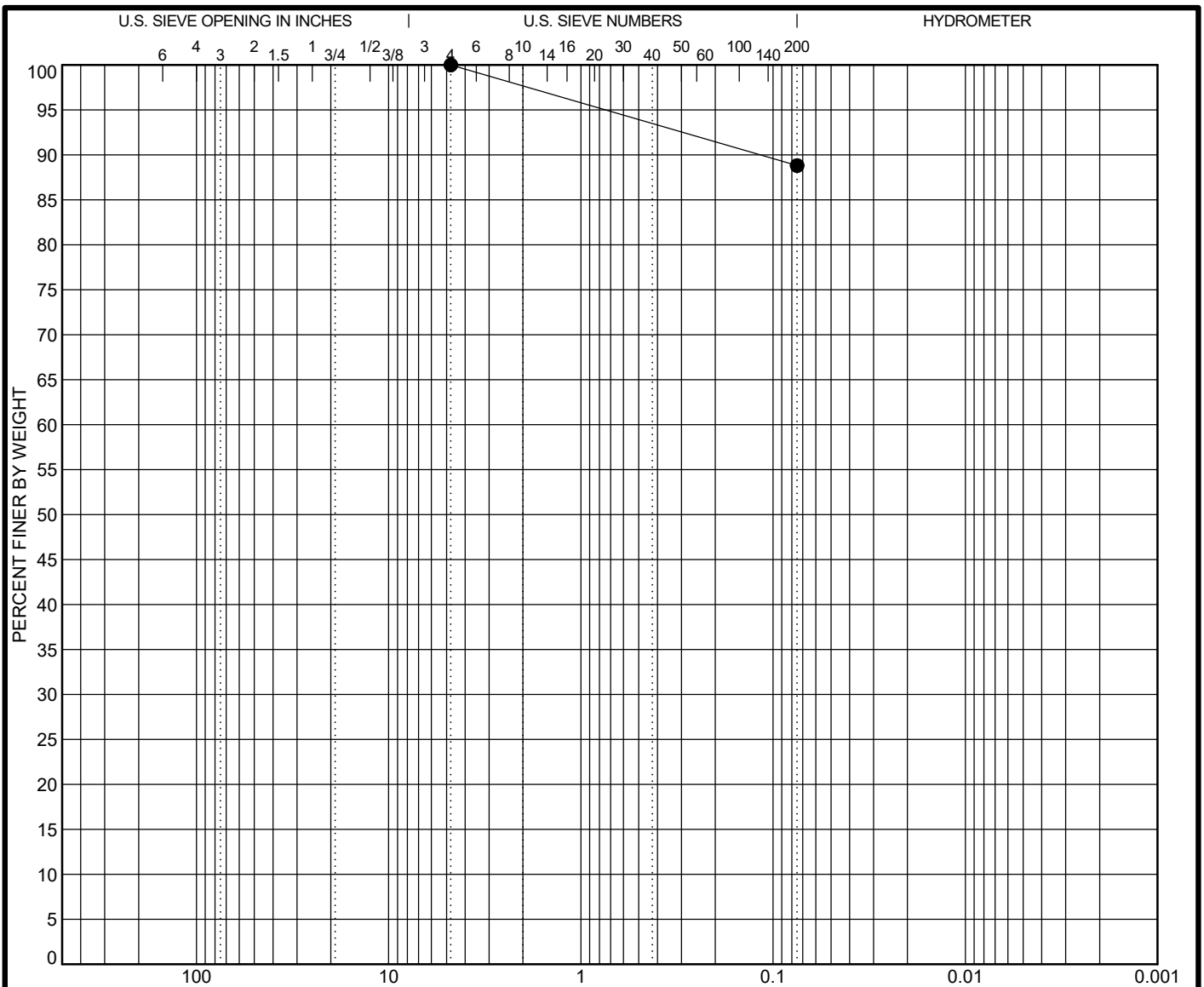
REMARKS:

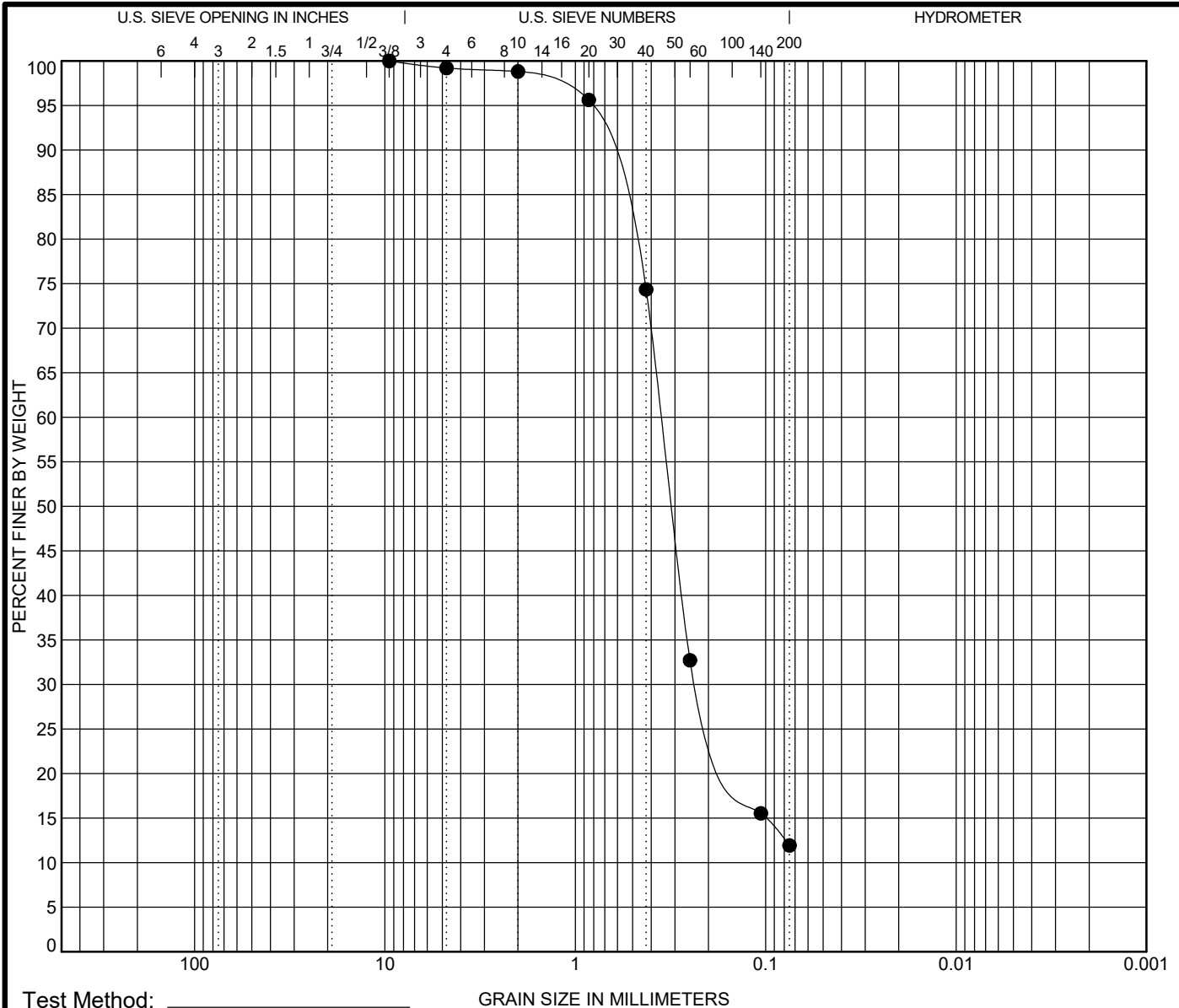


GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL







COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification			Classification	LL	PL	PI	Cc	Cu
●	B-13	S-12; 39.0 ft.	POORLY GRADED SAND with SILT (SP-SM)	NP	NP	NP	2.16	5.68

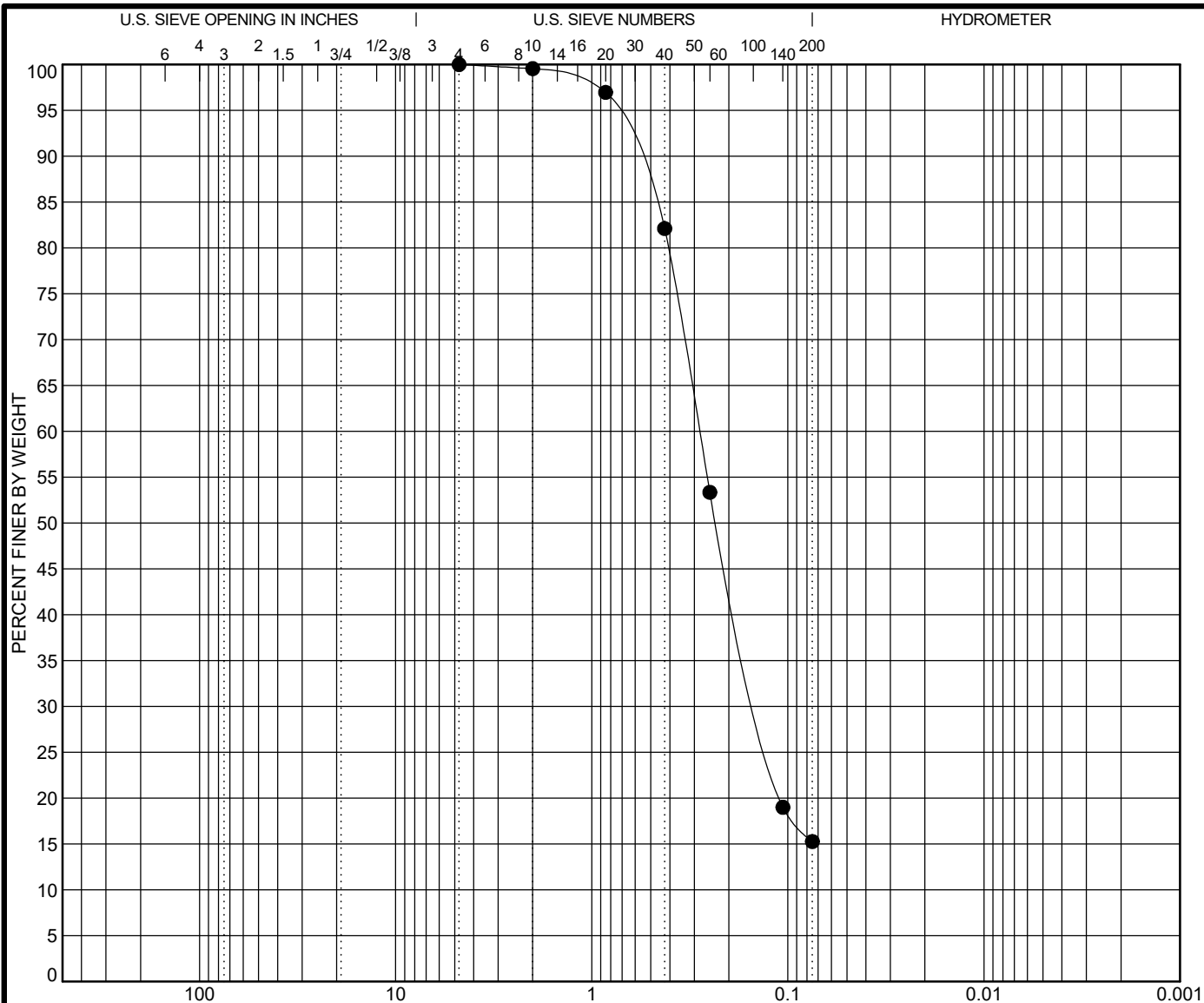
Specimen Identification		D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
	B-13	39.0 ft.	9.5	0.354	0.218	0.8	87.3	11.9	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● B-13 S-15;54.0 ft.	SILTY SAND (SM)	NP	NP	NP		

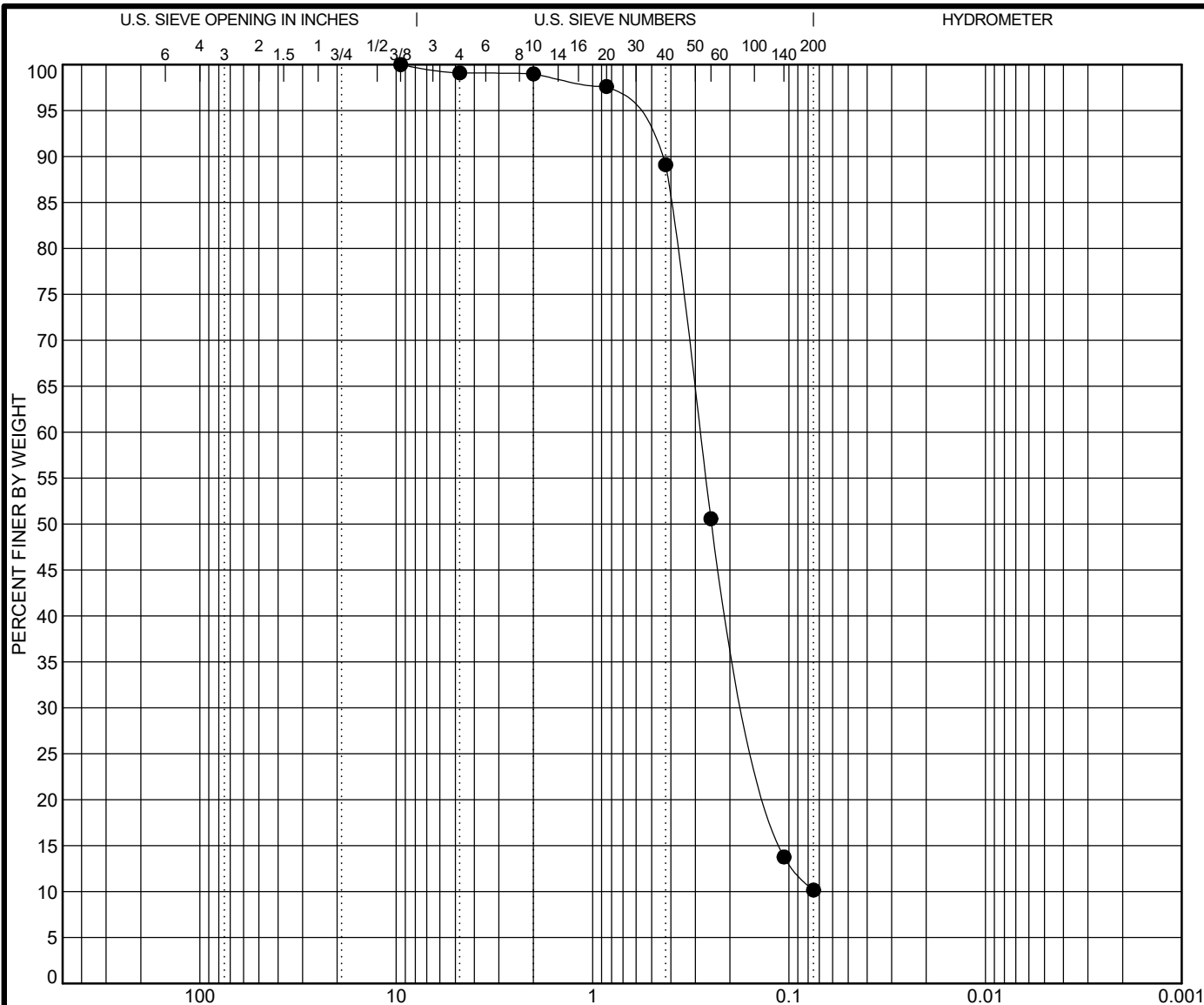
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-13 54.0 ft.	4.75	0.283	0.14		0.0	84.7	15.3	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-13 S-19;74.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	1.14	3.85

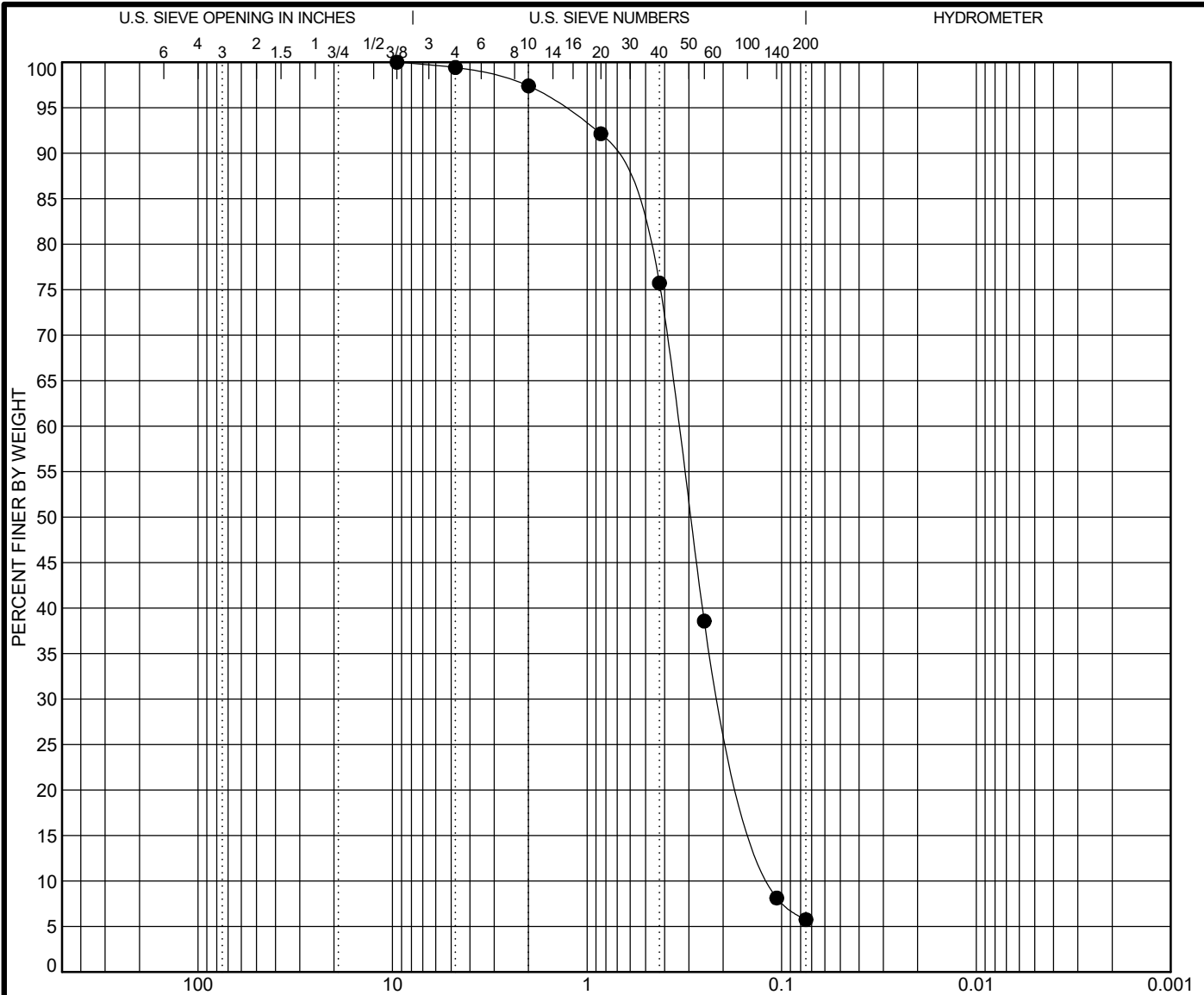
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-13 74.0 ft.	9.5	0.285	0.155		0.9	88.9	10.2	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-13 S-22;89.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	1.02	3.04

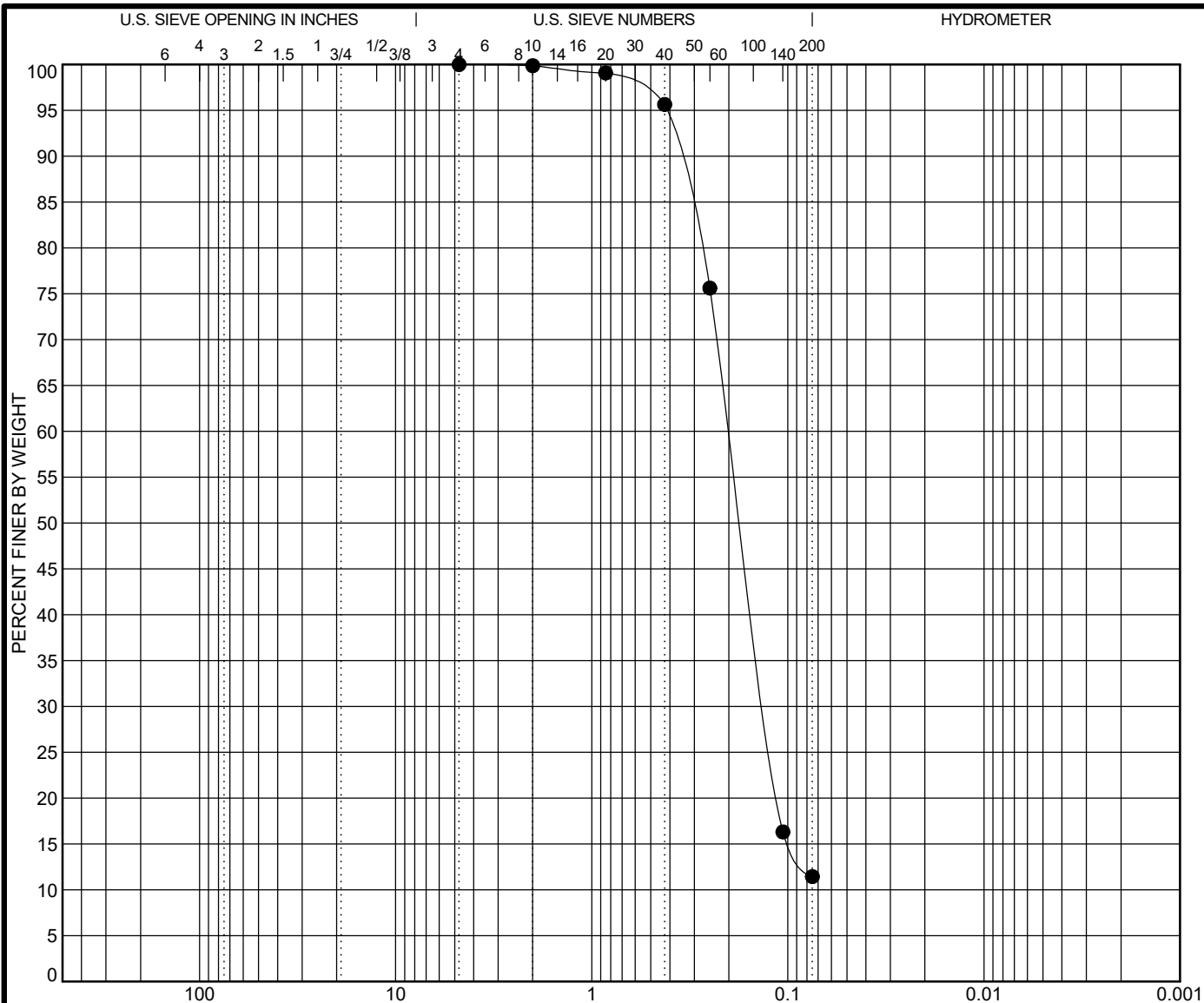
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-13 89.0 ft.	9.5	0.339	0.196	0.112	0.6	93.7	5.7	

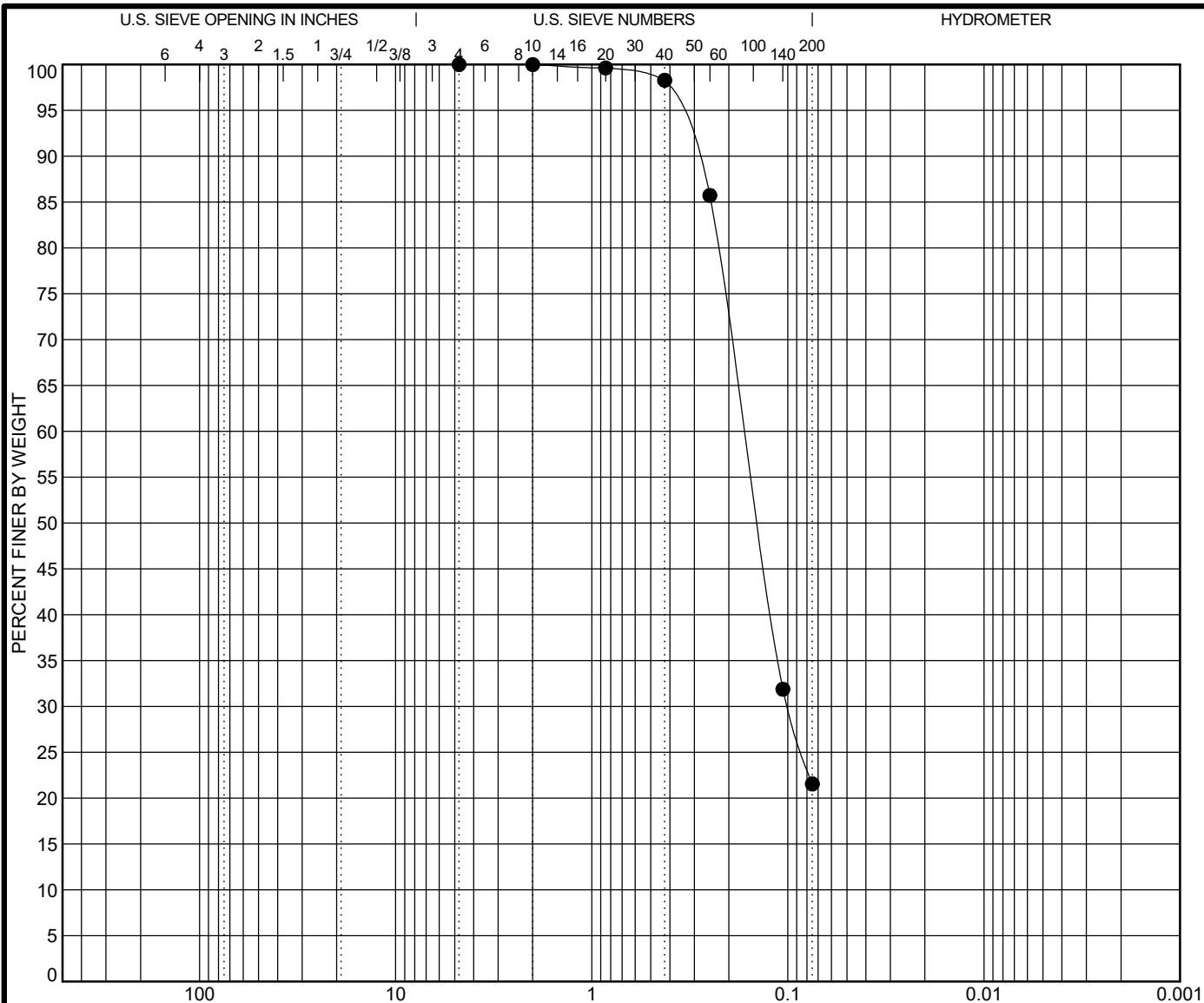
REMARKS:

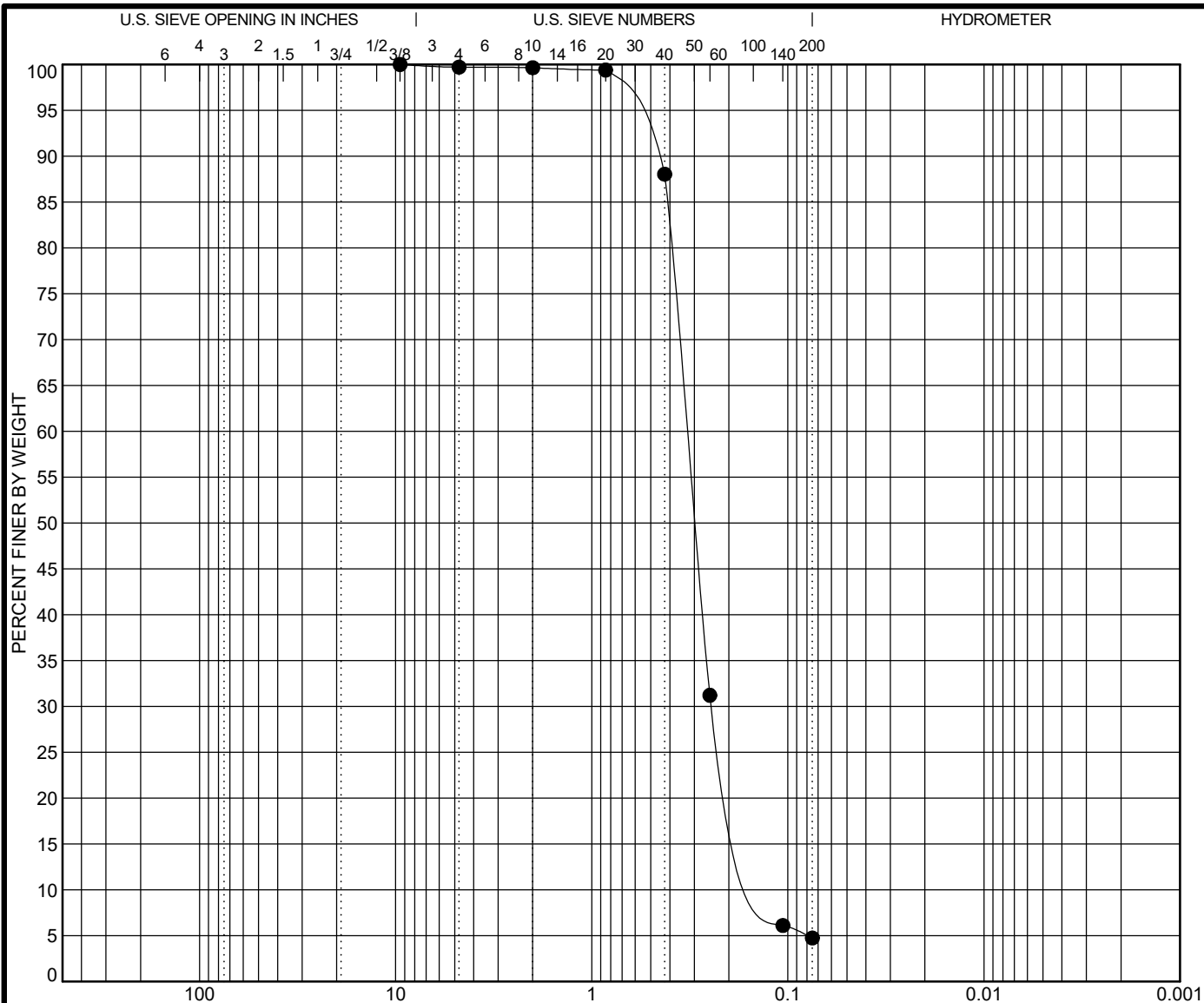


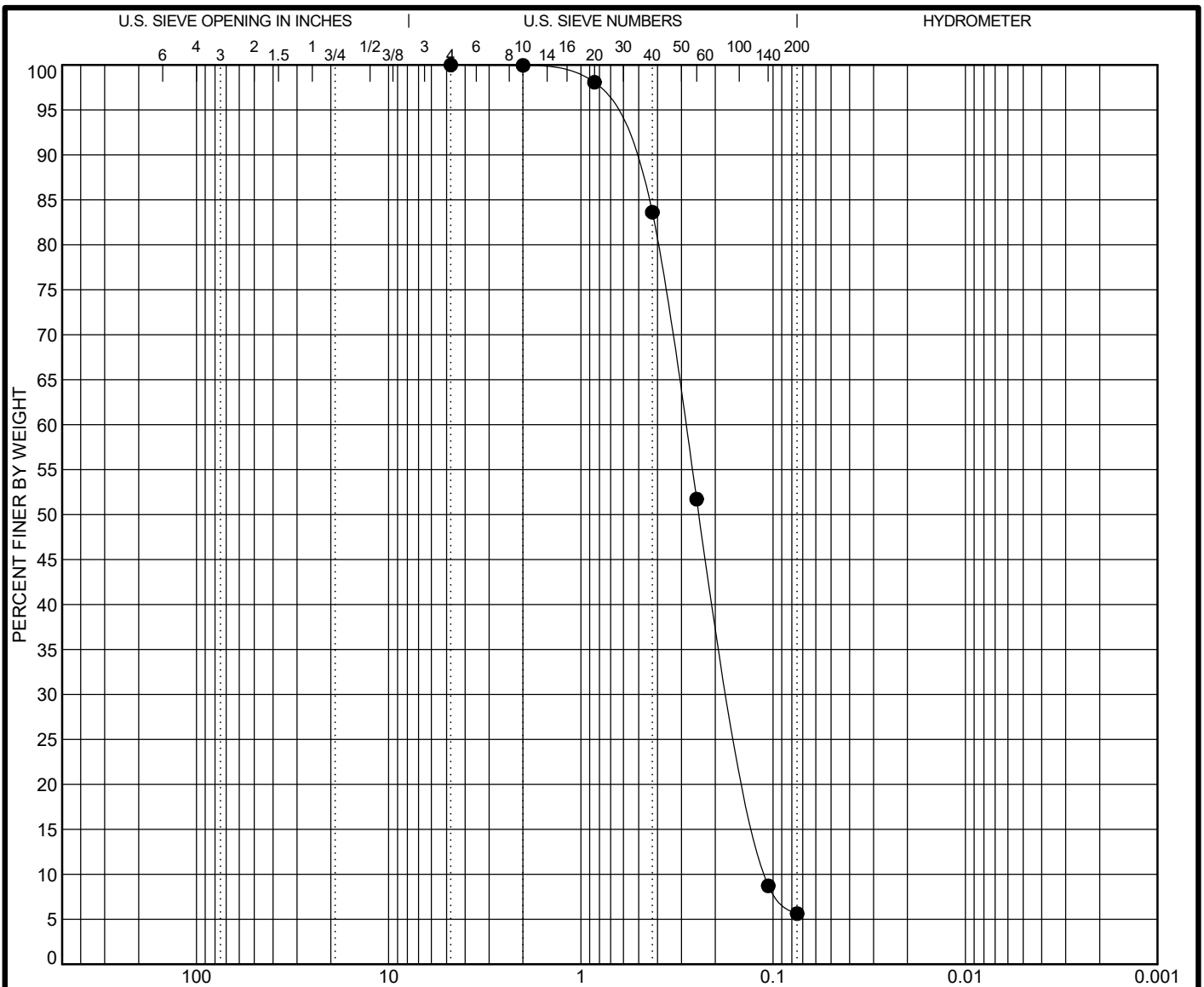
GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL









Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-14 S-14;49.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	0.84	2.64

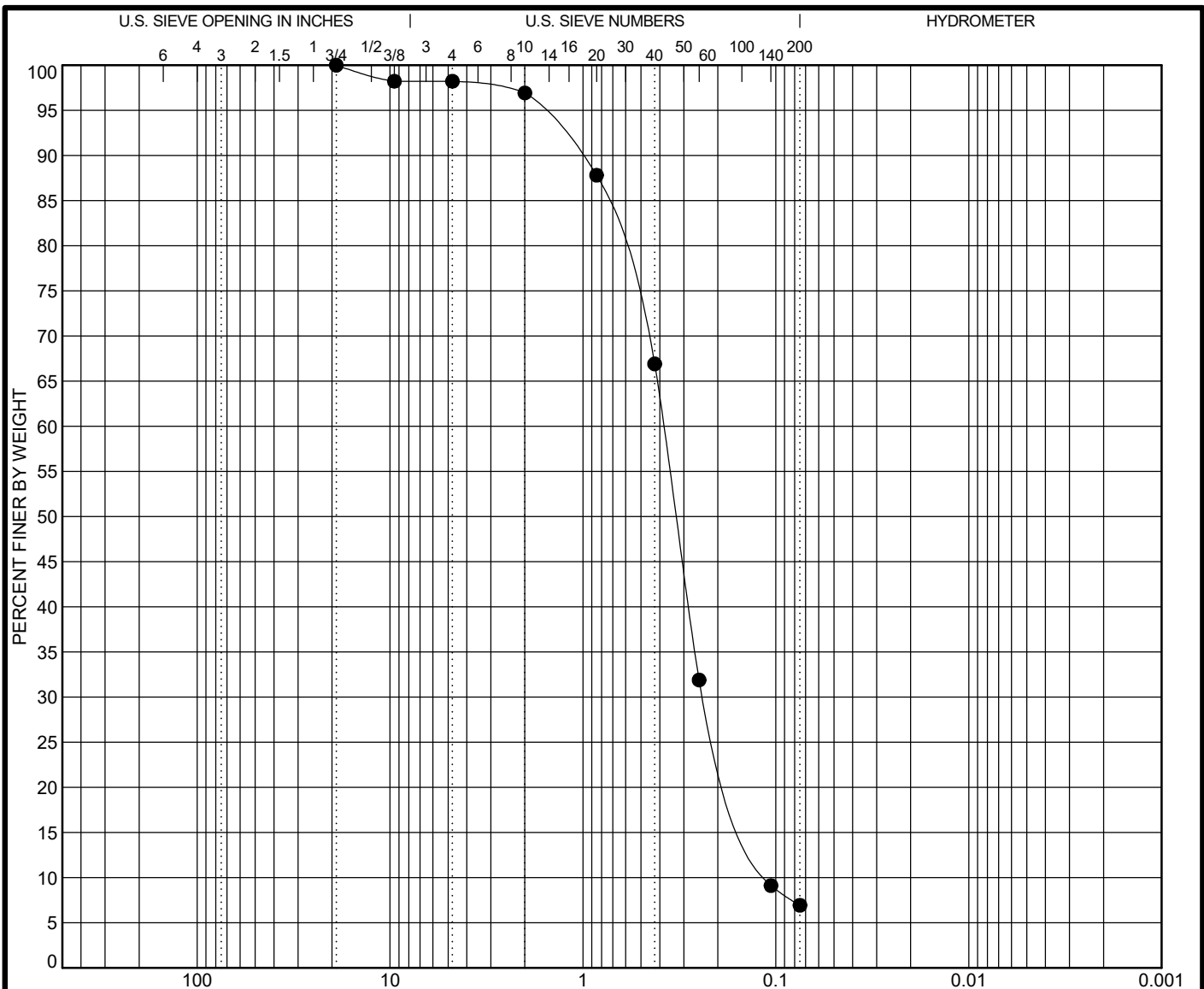
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-14 49.0 ft.	4.75	0.287	0.162	0.109	0.0	94.4	5.6	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-14 S-17;64.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	1.29	3.49

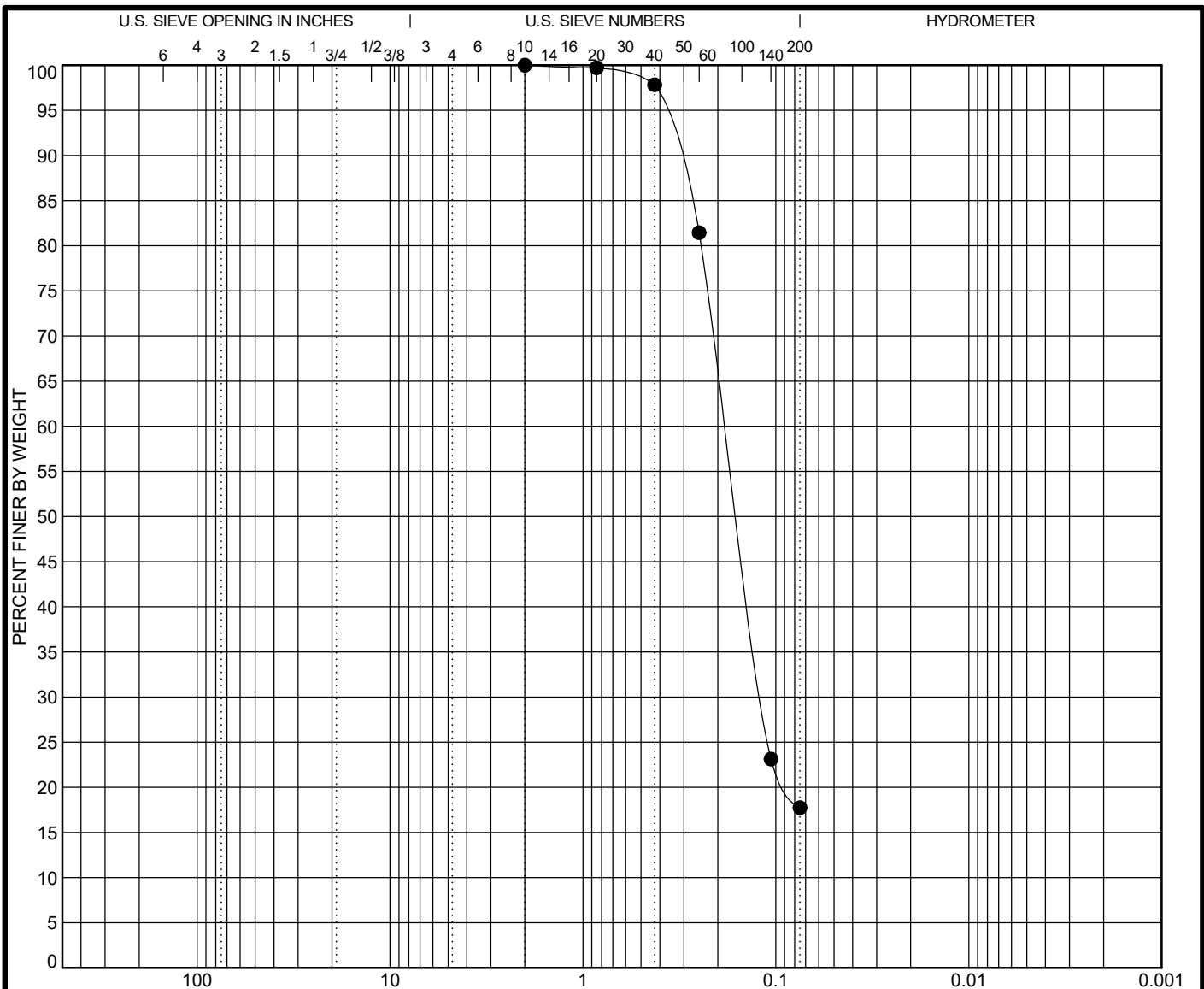
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-14 64.0 ft.	19	0.383	0.233	0.11	1.8	91.3	6.9	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● B-15 S-6;9.0 ft.						

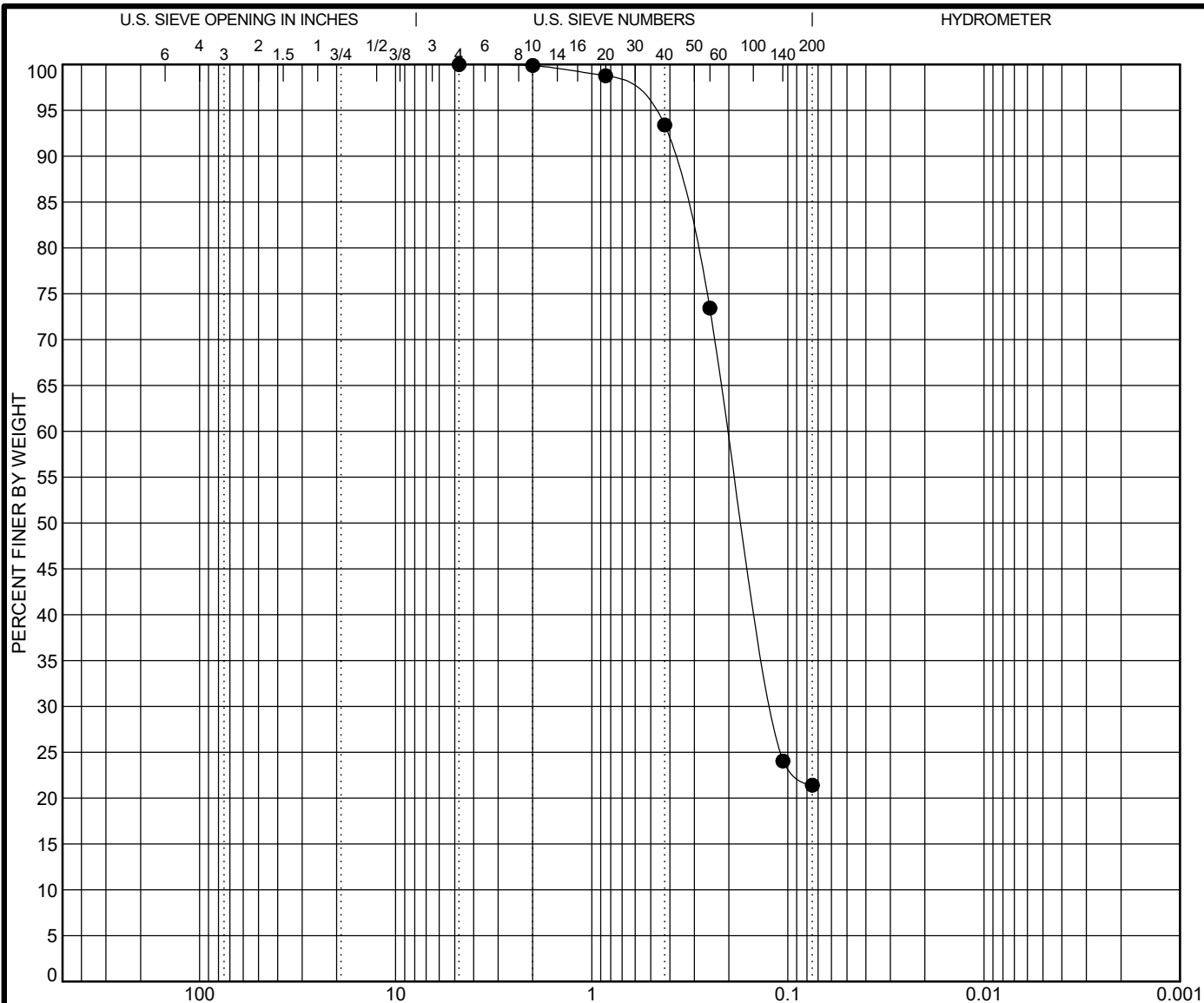
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-15 9.0 ft.	2	0.182	0.117		0.0	82.3	17.7	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● B-15 S-8;19.0 ft.	SILTY SAND (SM)	NP	NP	NP		

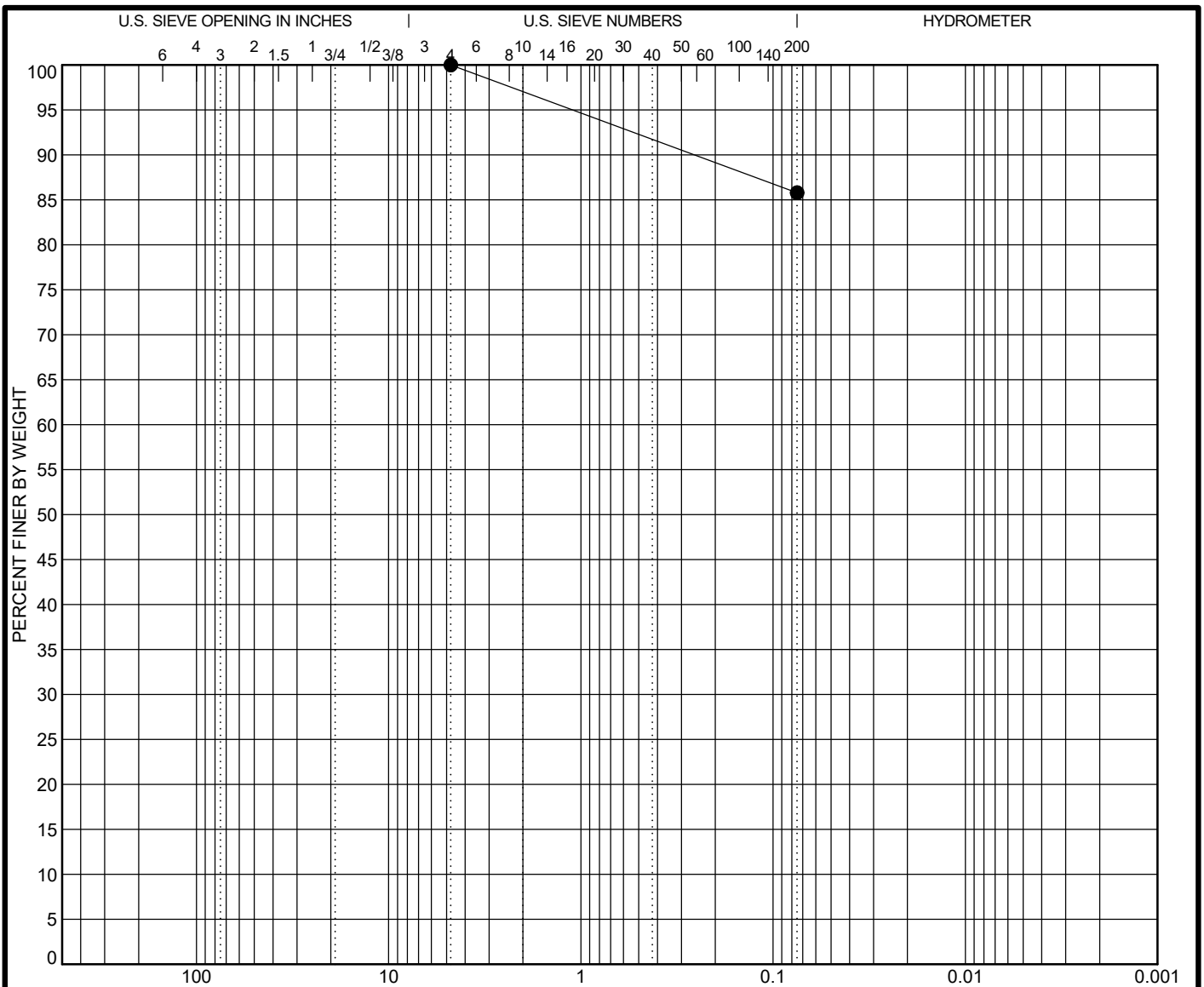
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-15 19.0 ft.	4.75	0.198	0.118		0.0	78.6	21.4	

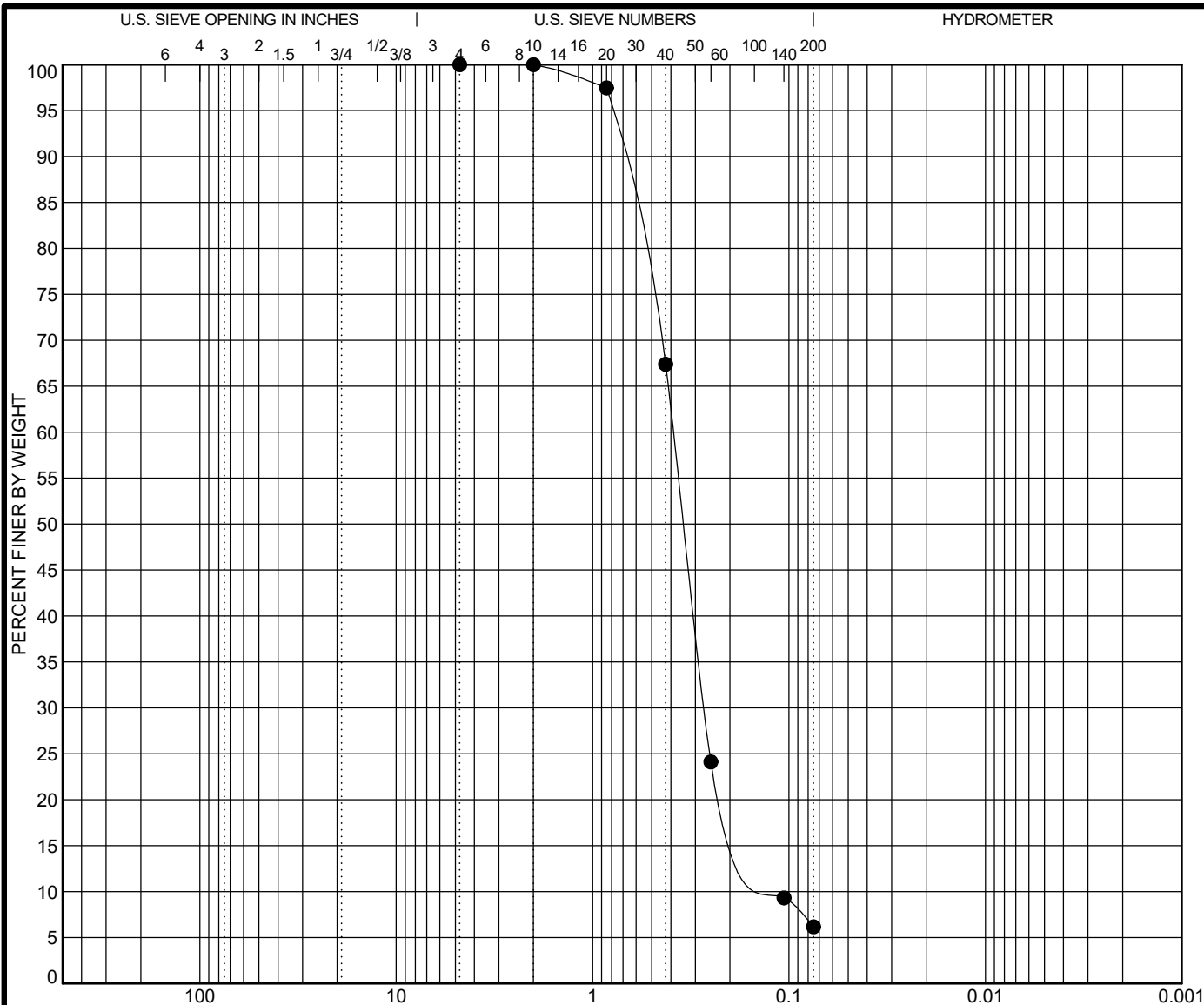
REMARKS:

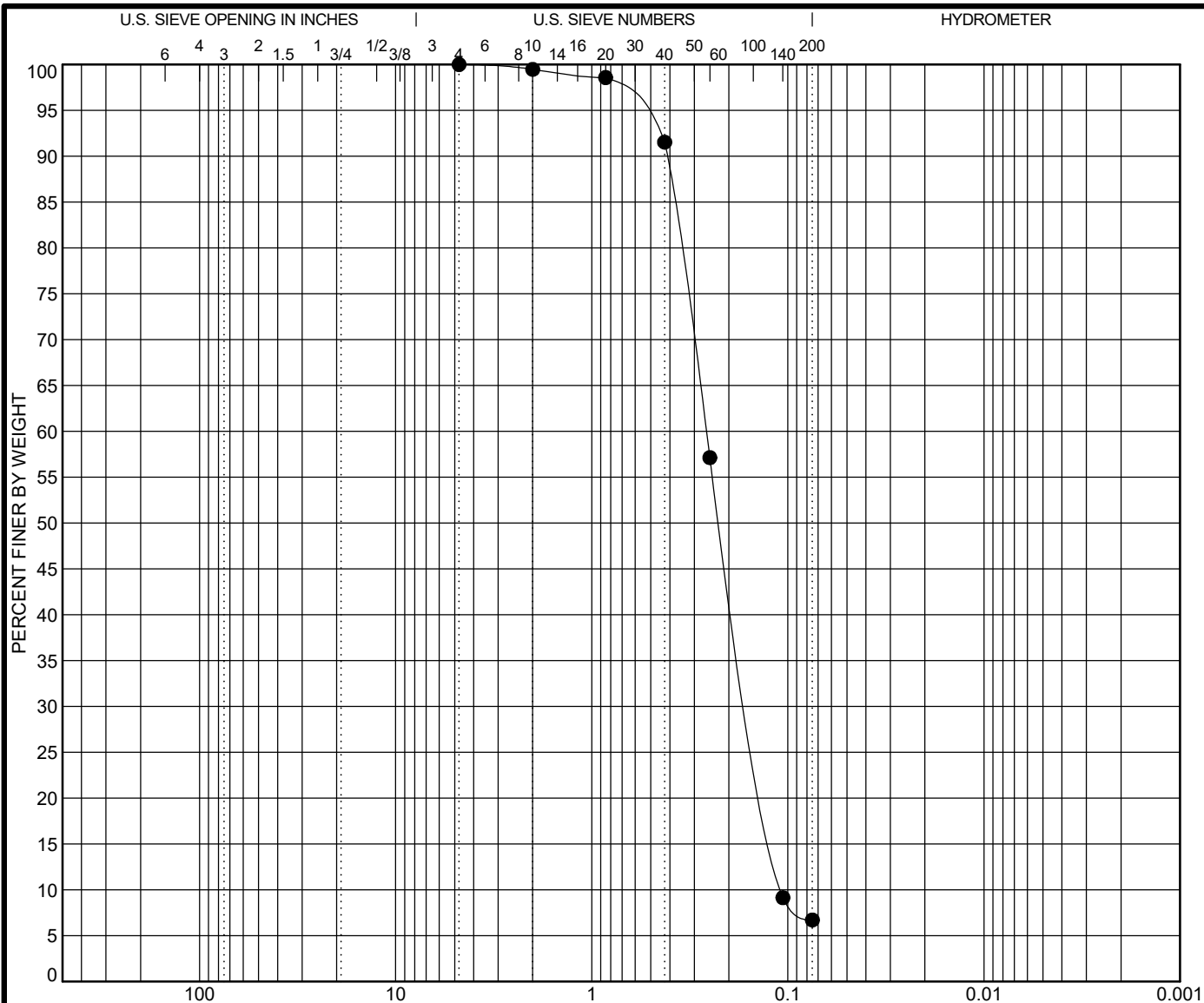


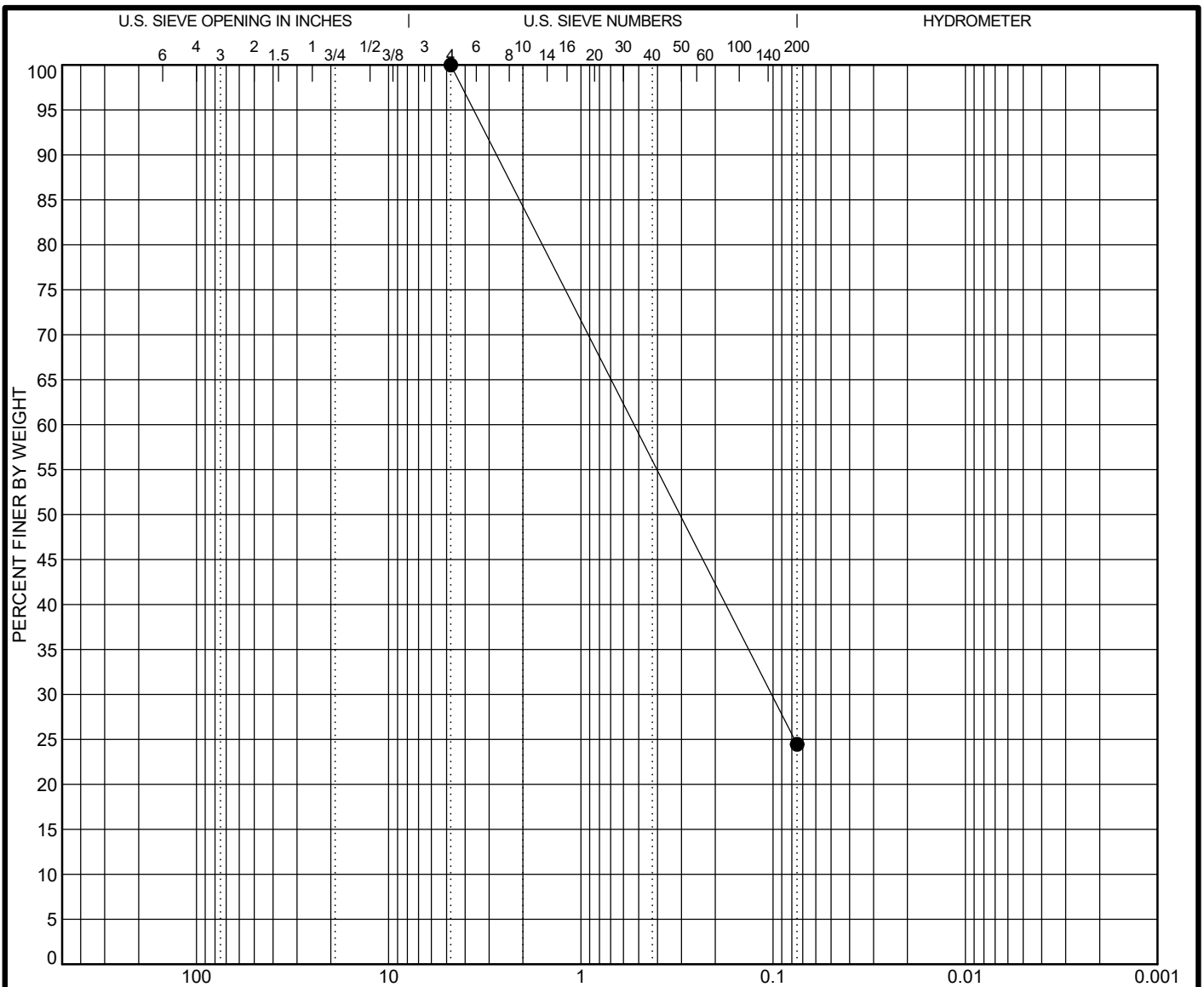
GRAIN SIZE DISTRIBUTION

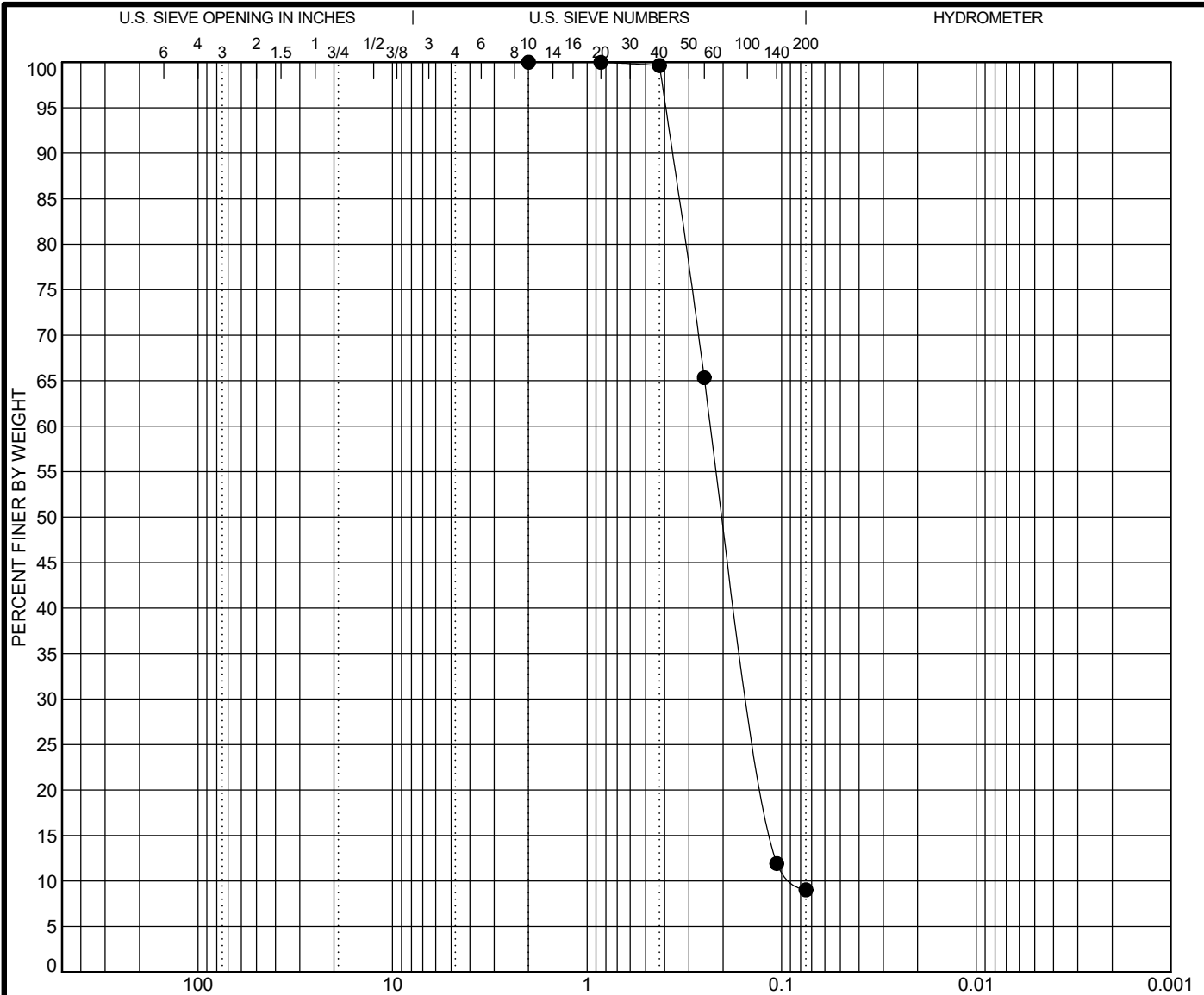
PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL











Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● B-16 S-8;19.0 ft.					1.04	2.72

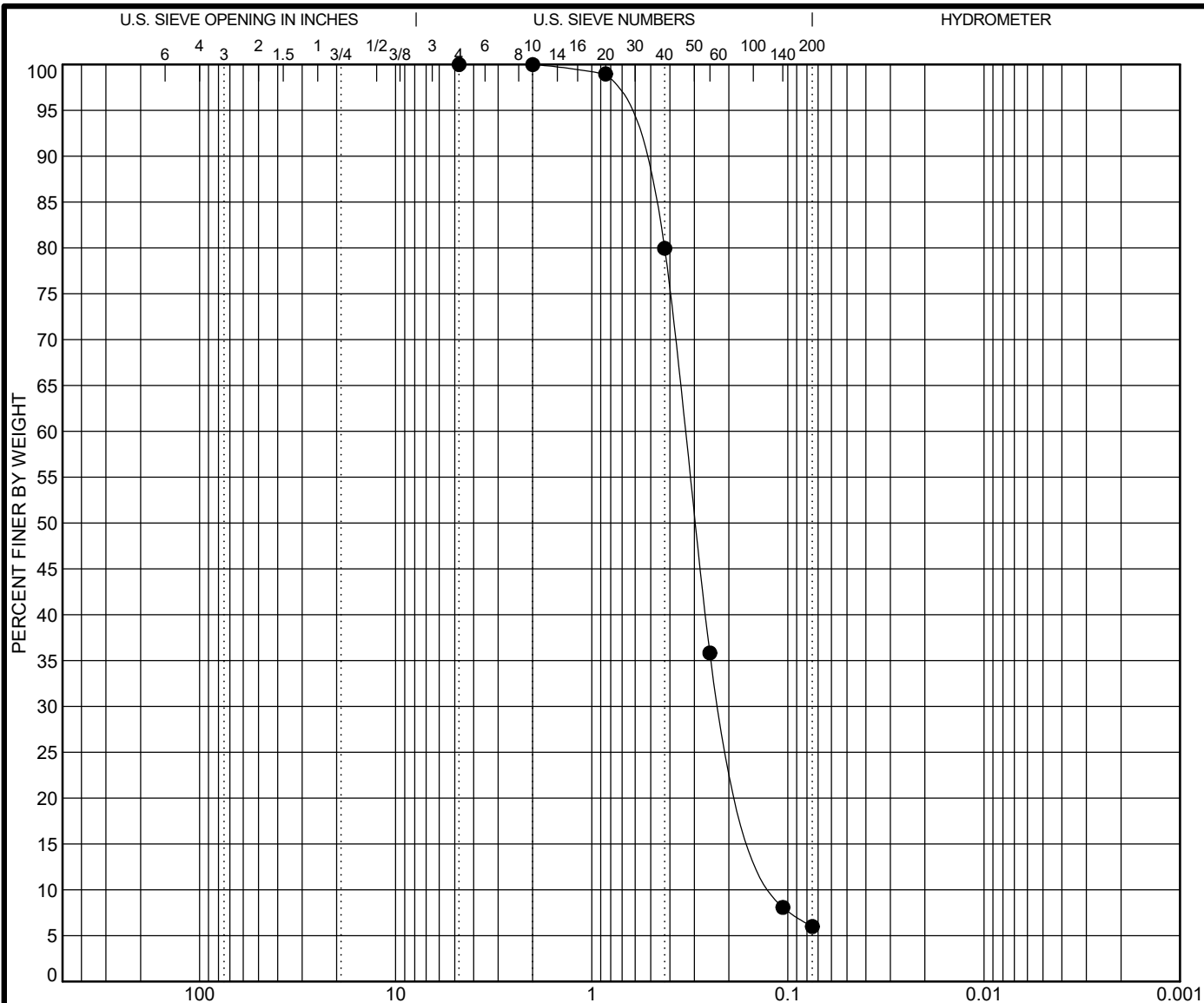
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-16 19.0 ft.	2	0.23	0.142	0.084	0.0	91.0	9.0	

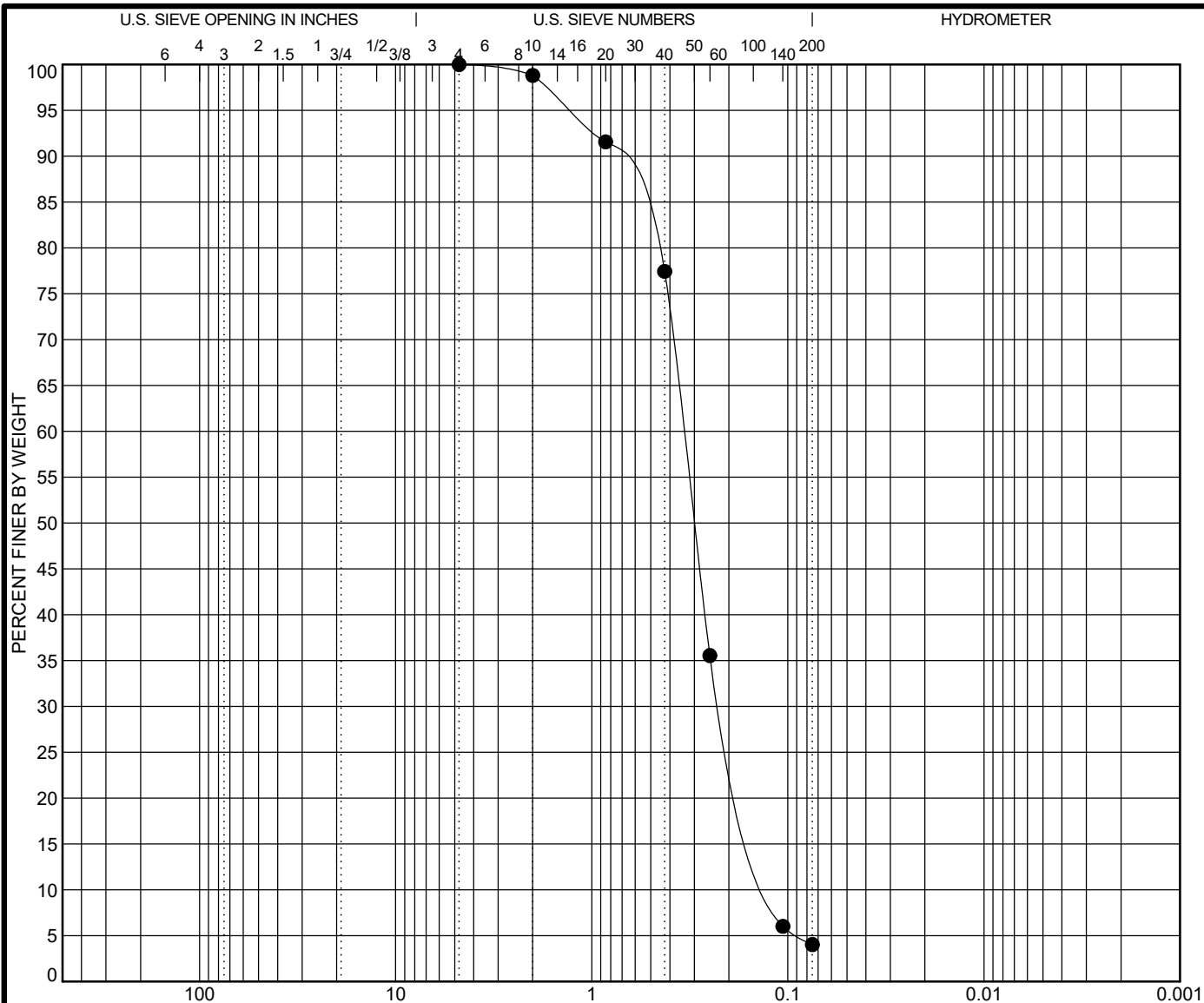
REMARKS:

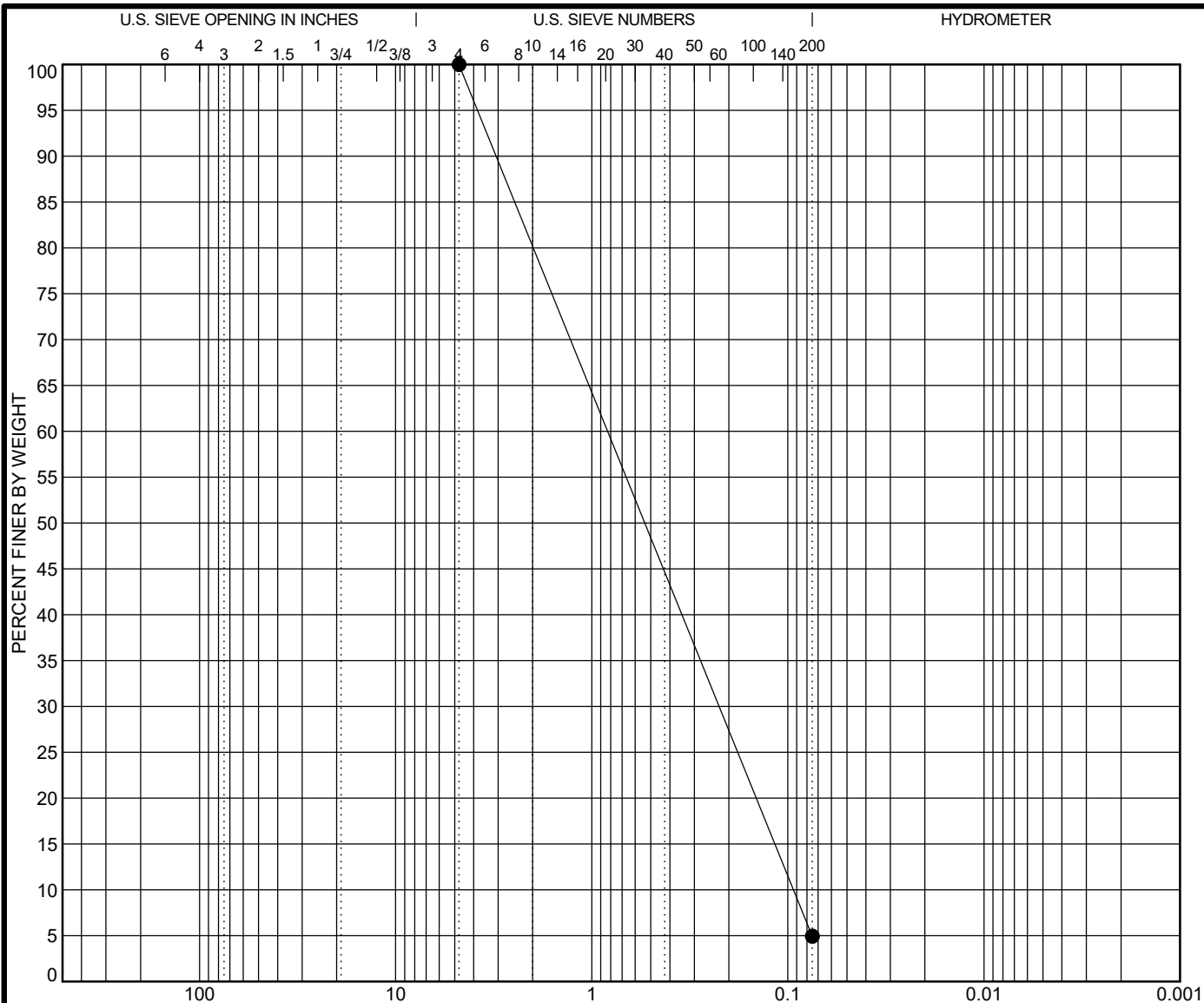


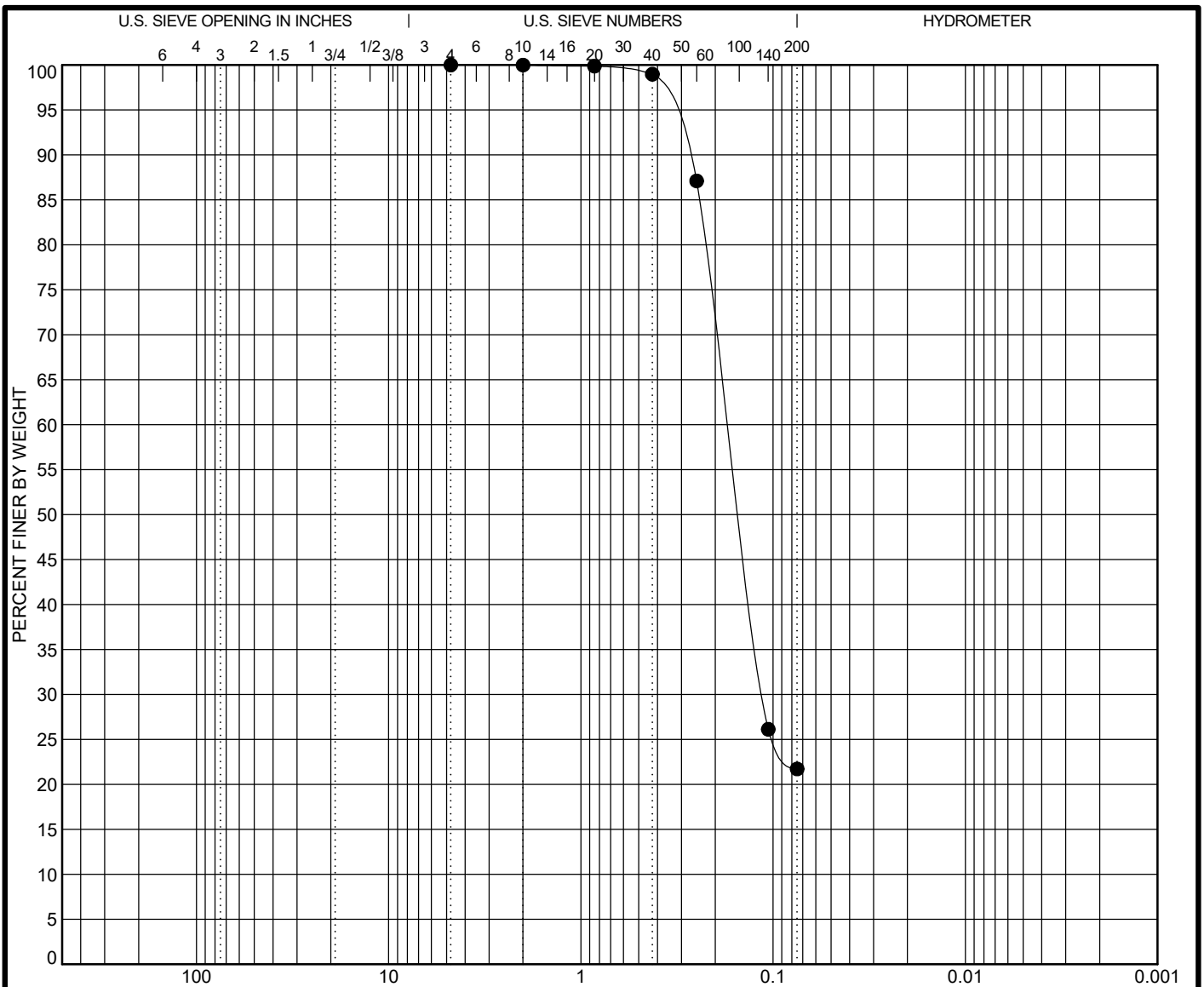
GRAIN SIZE DISTRIBUTION

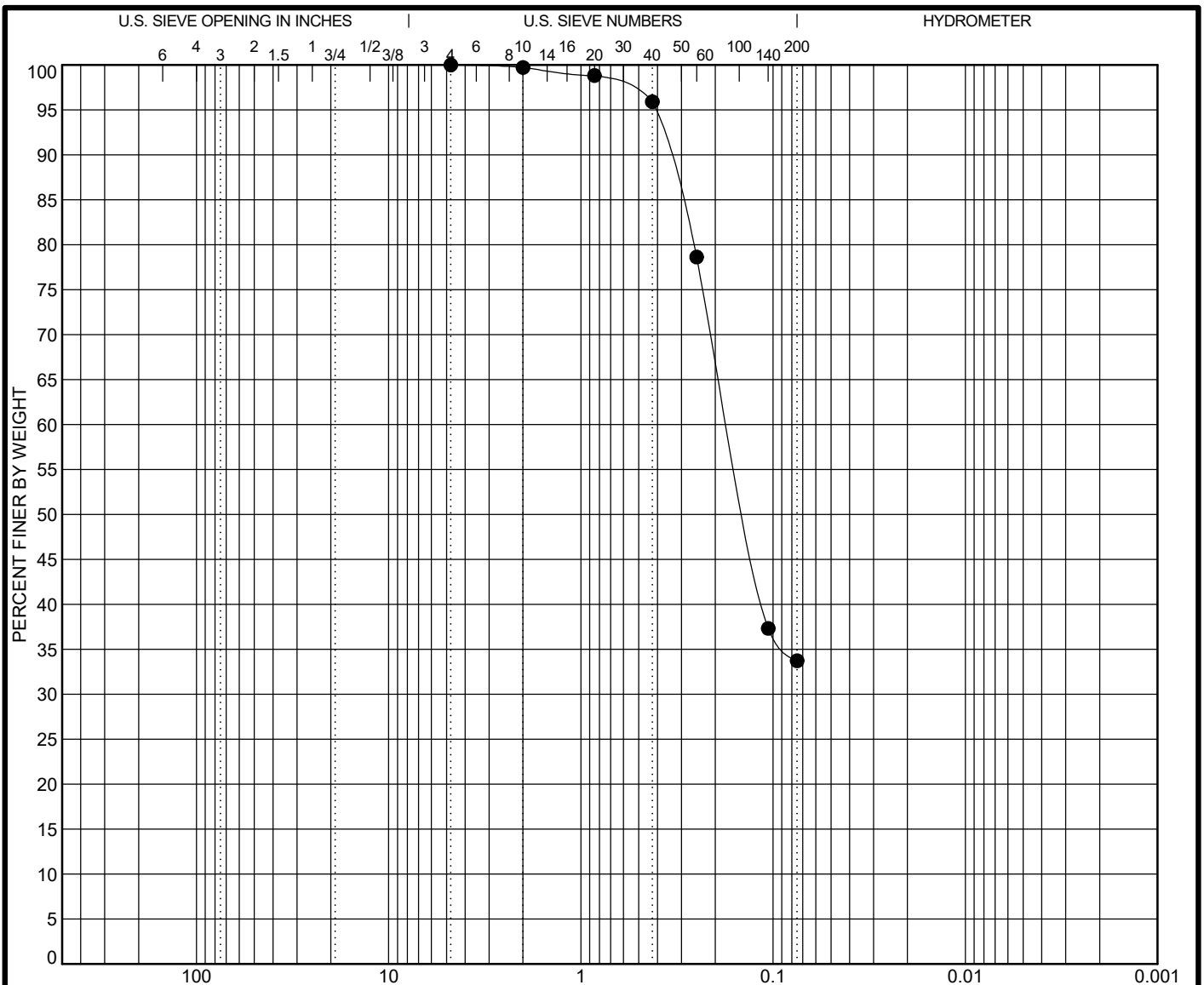
PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL

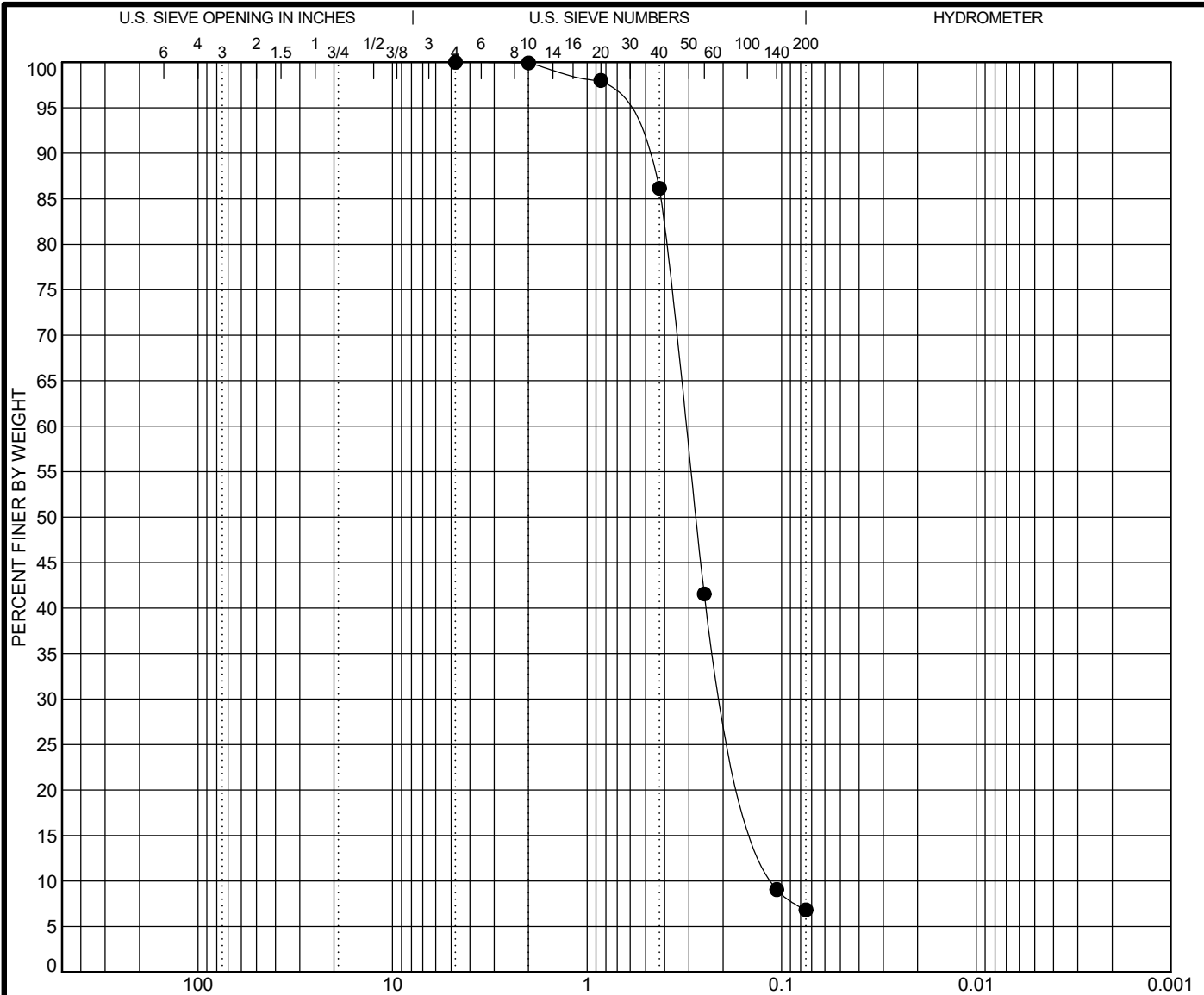


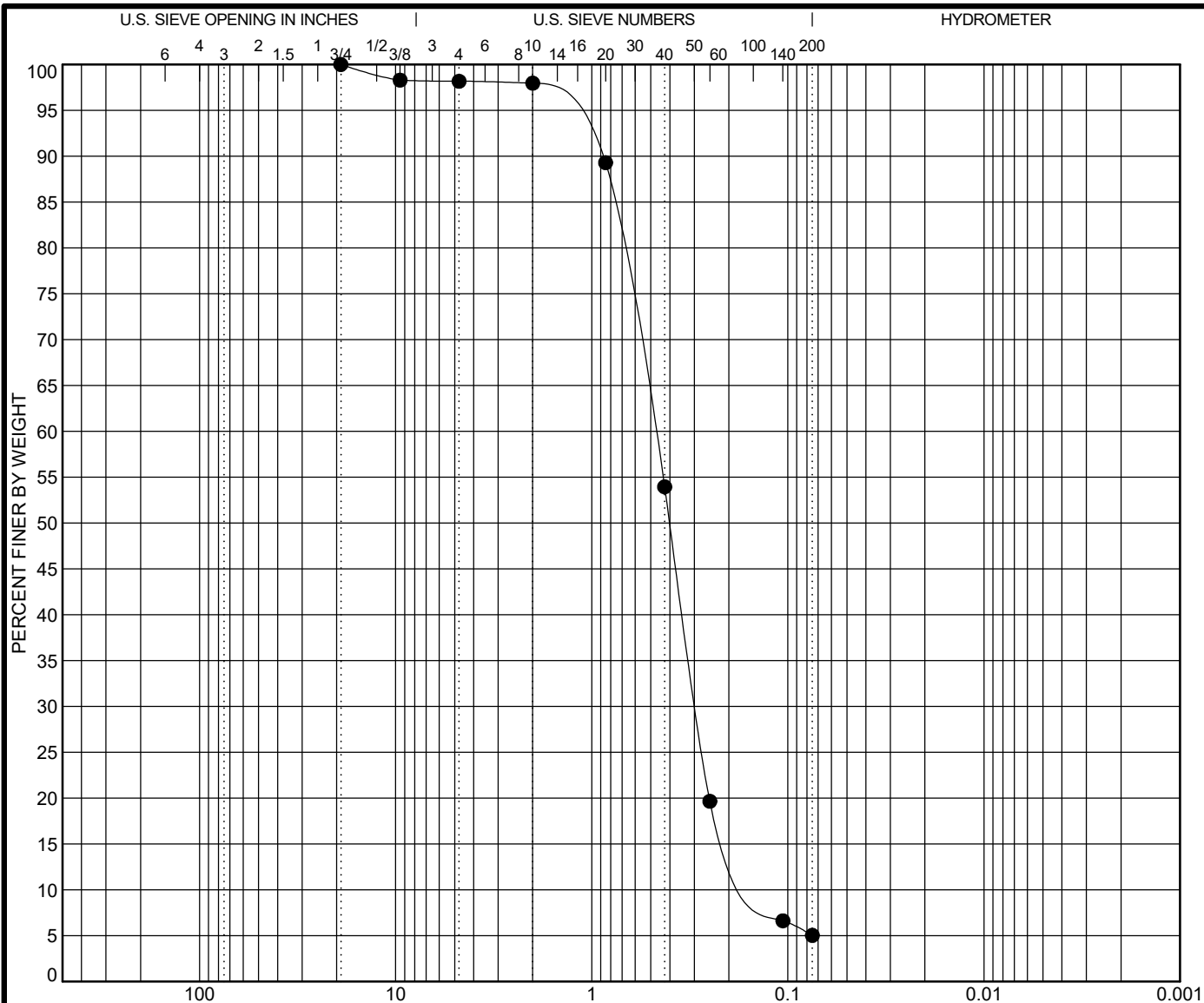












Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-17 S-12;39.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	1.36	3.61

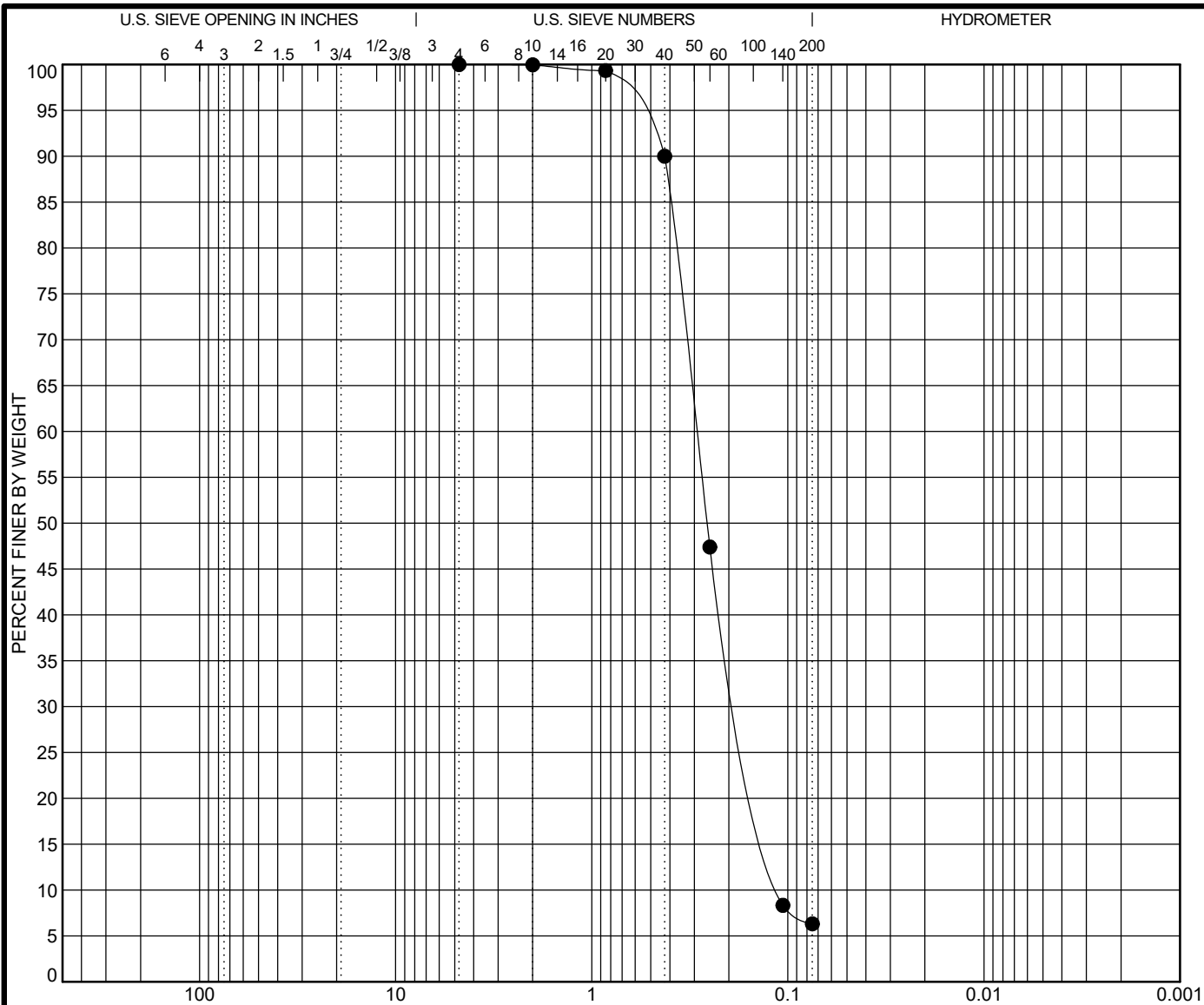
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-17 39.0 ft.	19.05	0.479	0.293	0.132	1.8	93.1	5.0	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-17 S-14;49.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	0.91	2.66

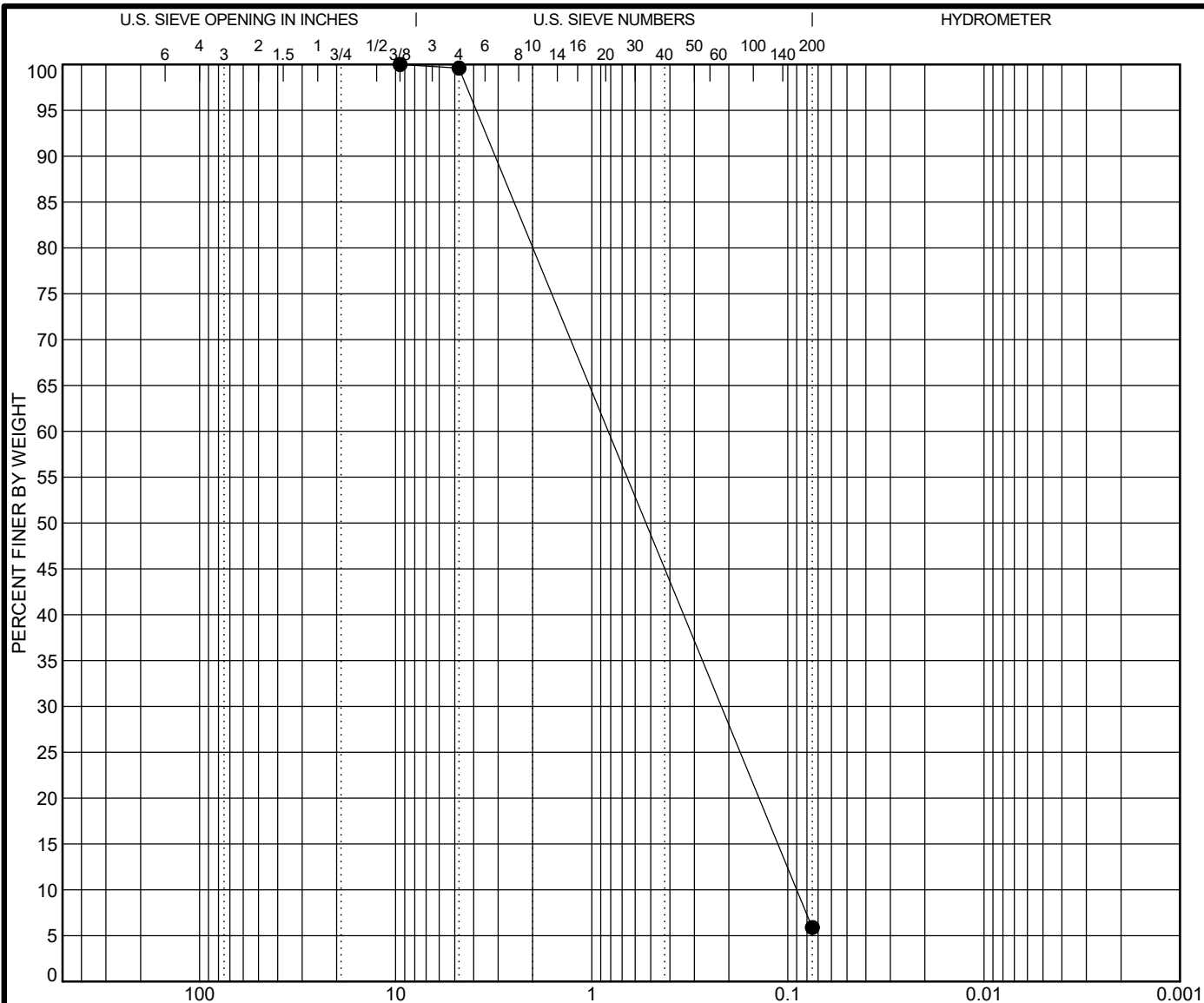
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-17 49.0 ft.	4.75	0.292	0.171	0.11	0.0	93.7	6.3	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● B-17 S-16;59.0 ft.	POORLY GRADED SAND with SILT (SP-SM)					NP	NP	NP	0.64	9.15

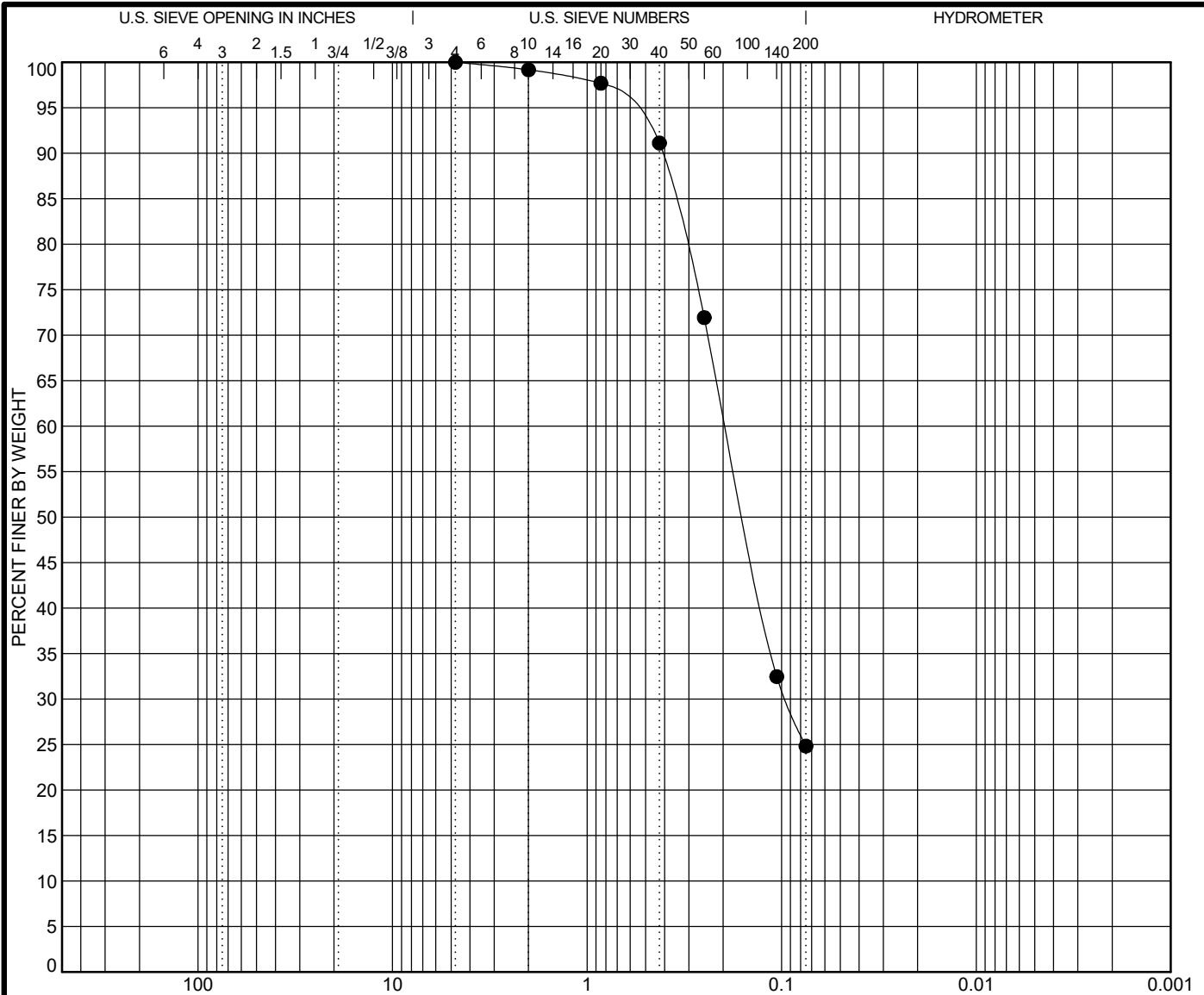
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
B-17 59.0 ft.	9.5	0.823	0.218	0.09	0.4	93.7	5.9	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● C-1 1;0.3 ft.	SILTY SAND (SM)	NP	16	NP		

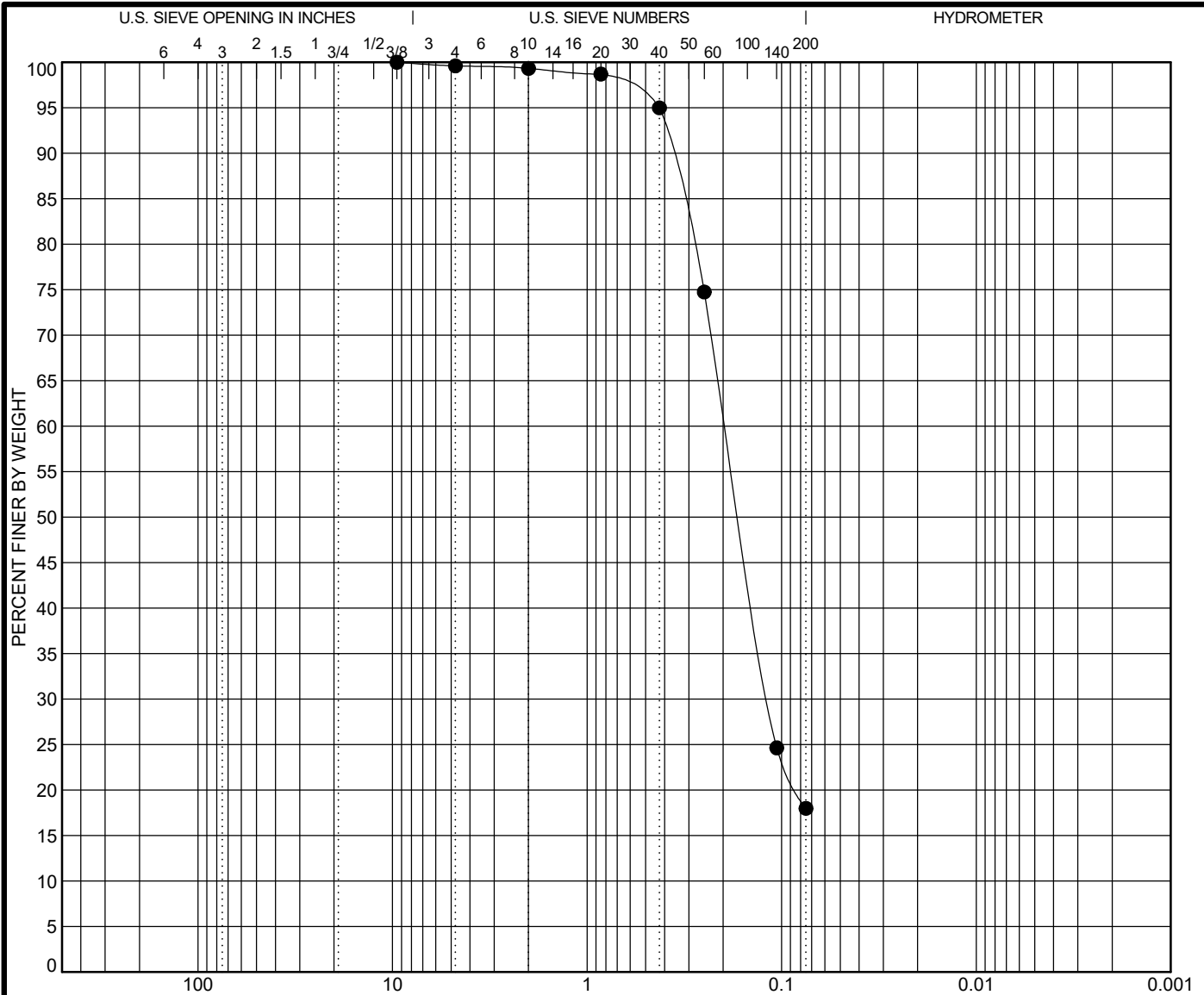
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
C-1 0.3 ft.	4.75	0.193	0.095		0.0	75.2	24.8	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● C-2 1;0.4 ft.	SILTY SAND (SM)	NP	NP	NP		

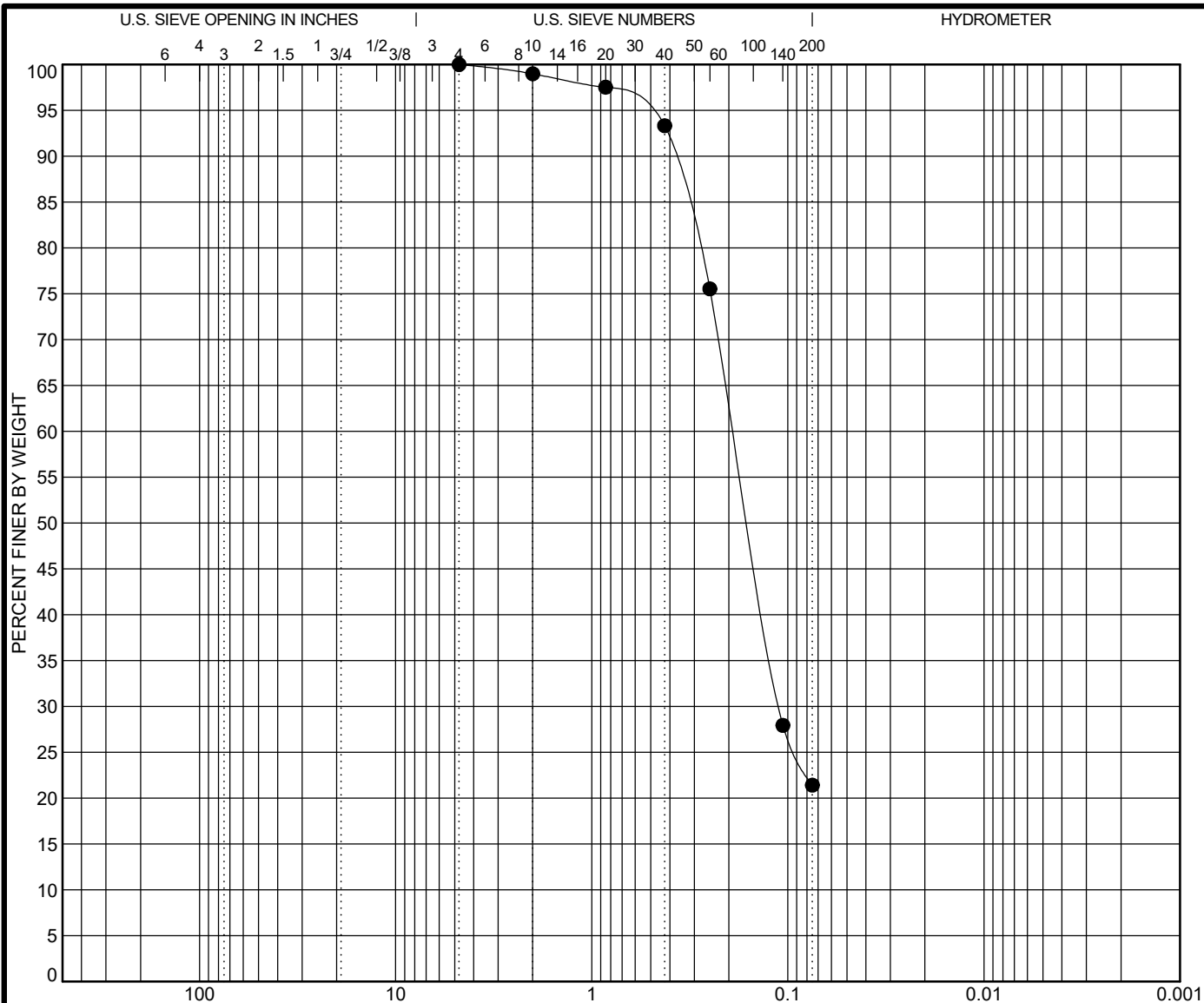
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
C-2 0.4 ft.	9.5	0.194	0.116		0.4	81.6	18.0	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● C-3 1;0.4 ft.	SILTY SAND (SM)	NP	14	NP		

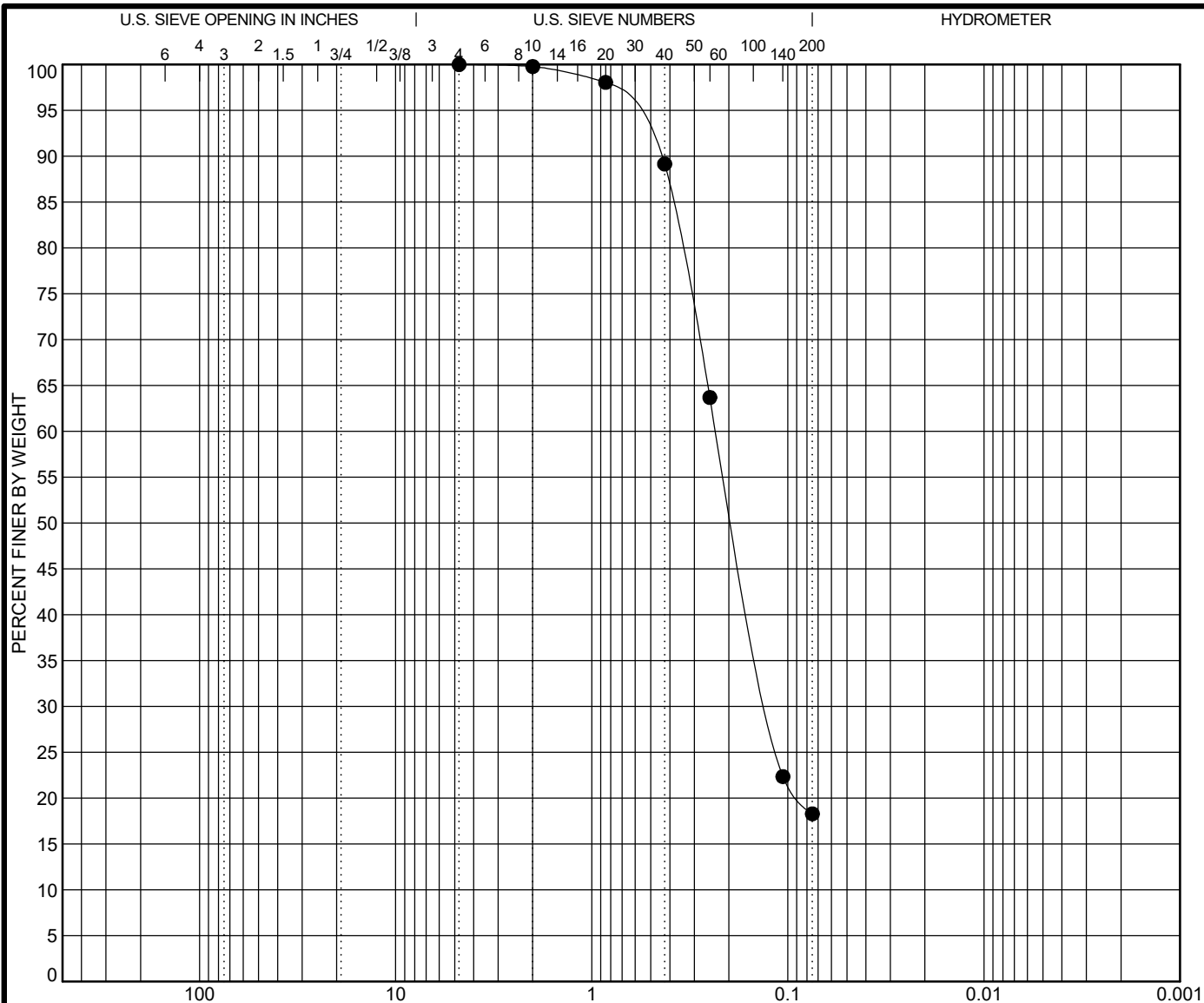
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
C-3 0.4 ft.	4.75	0.189	0.11		0.0	78.6	21.4	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● C-4 ; 0.4 ft.	SILTY SAND (SM)	NP	14	NP		

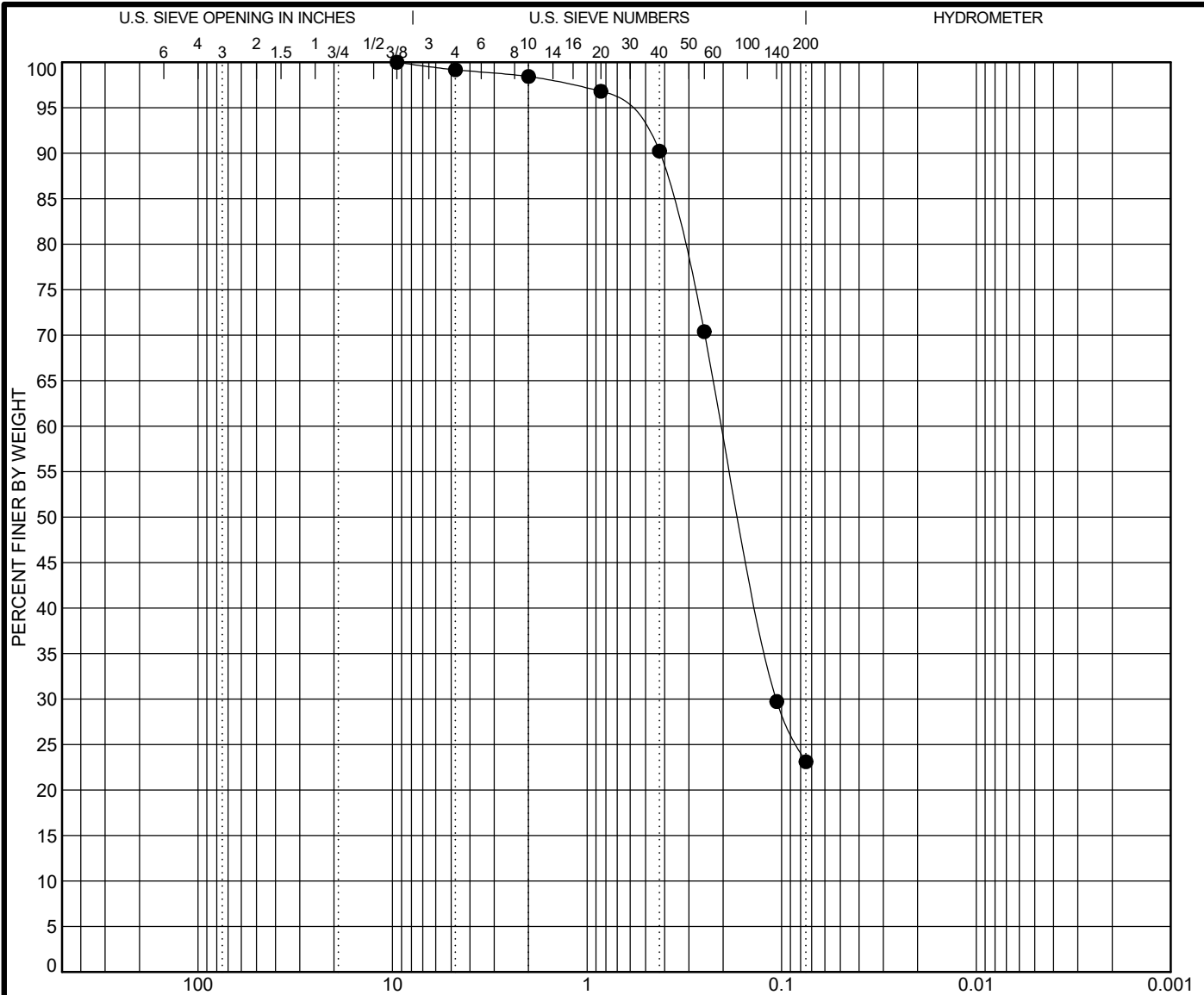
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
C-4 0.4 ft.	4.75	0.232	0.124		0.0	81.7	18.3	

REMARKS:



GRAIN SIZE DISTRIBUTION

PROJECT NAME: CITY OF MOBILE
CIVIC CENTER
G.E.T. PROJ. NUMBER: 24-120
PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● C-5 1;0.3 ft.	SILTY, CLAYEY SAND (SC-SM)	17	12	5		

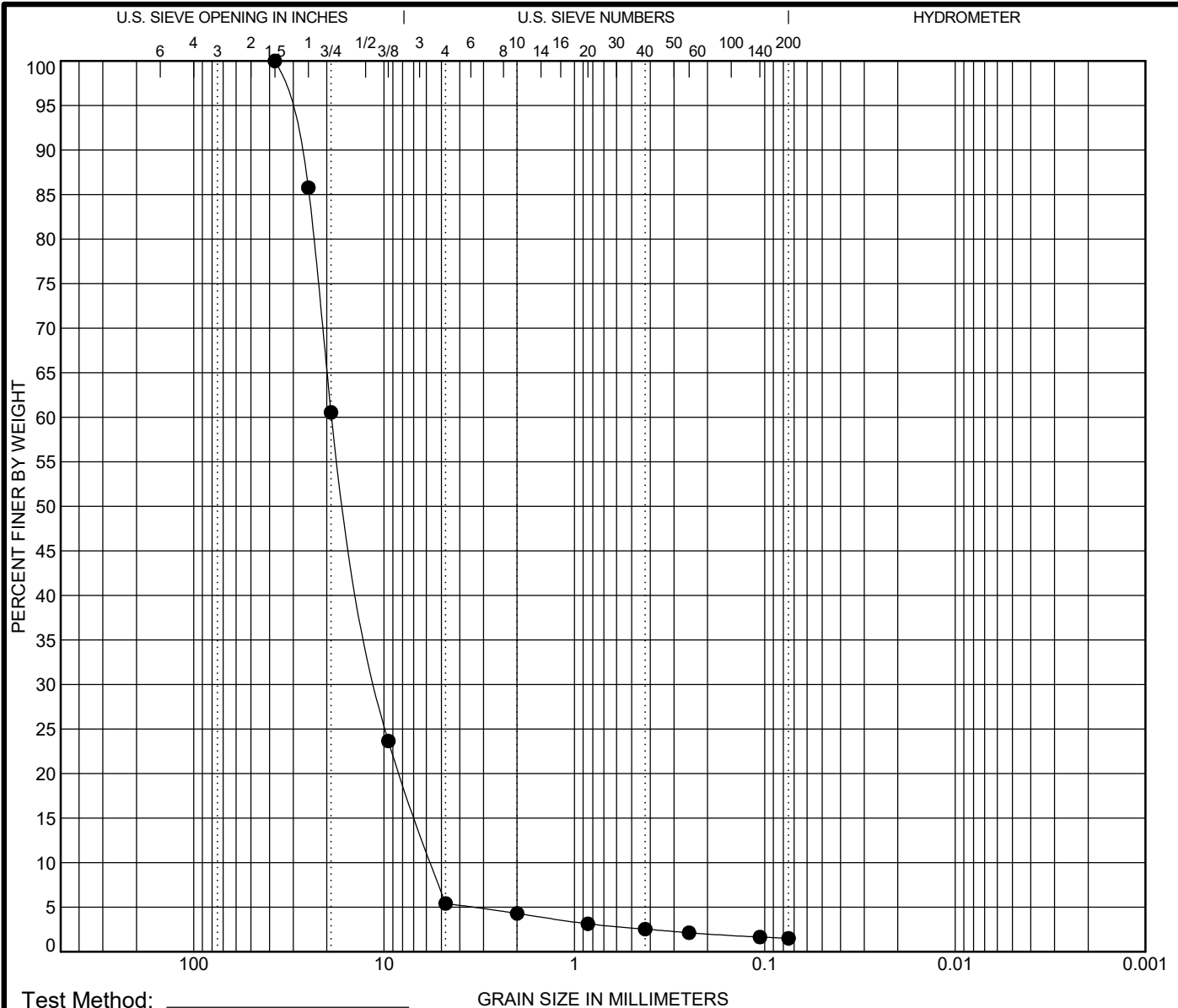
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
C-5 0.3 ft.	9.5	0.201	0.107		0.8	76.1	23.1	

REMARKS:



GRAIN SIZE DISTRIBUTION

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CIVIC CENTER
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PROJECT LOCATION: MOBILE, AL



Test Method: _____ GRAIN SIZE IN MILLIMETERS

COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification					LL	PL	PI	Cc	Cu
● C-6 1;0.2 ft.	POORLY GRADED GRAVEL (GP)					NP	NP	NP	1.08	3.33

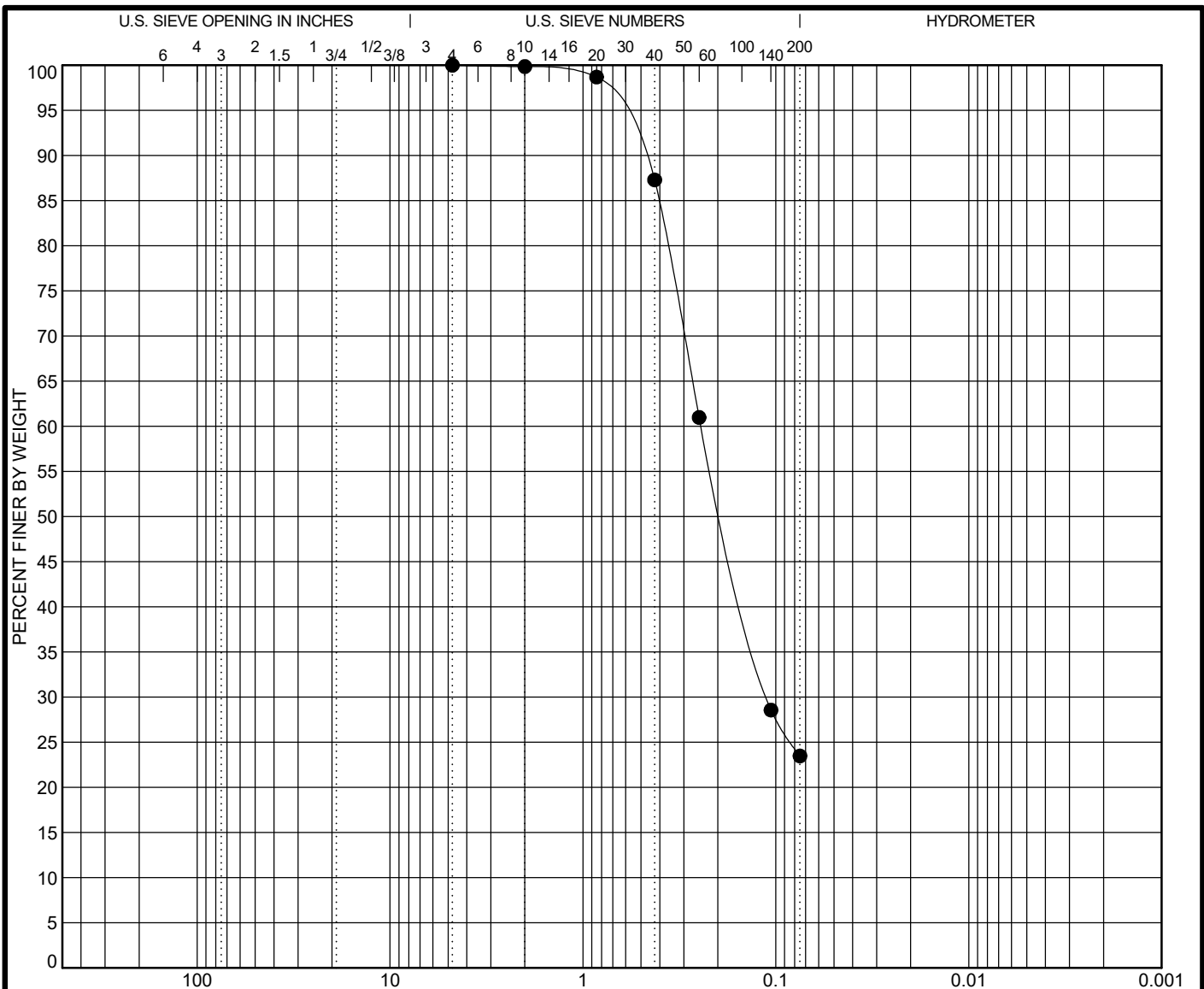
Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
C-6 0.2 ft.	37.5	18.807	10.703	5.653	94.6	3.9	1.5	

REMARKS:



GRAIN SIZE DISTRIBUTION

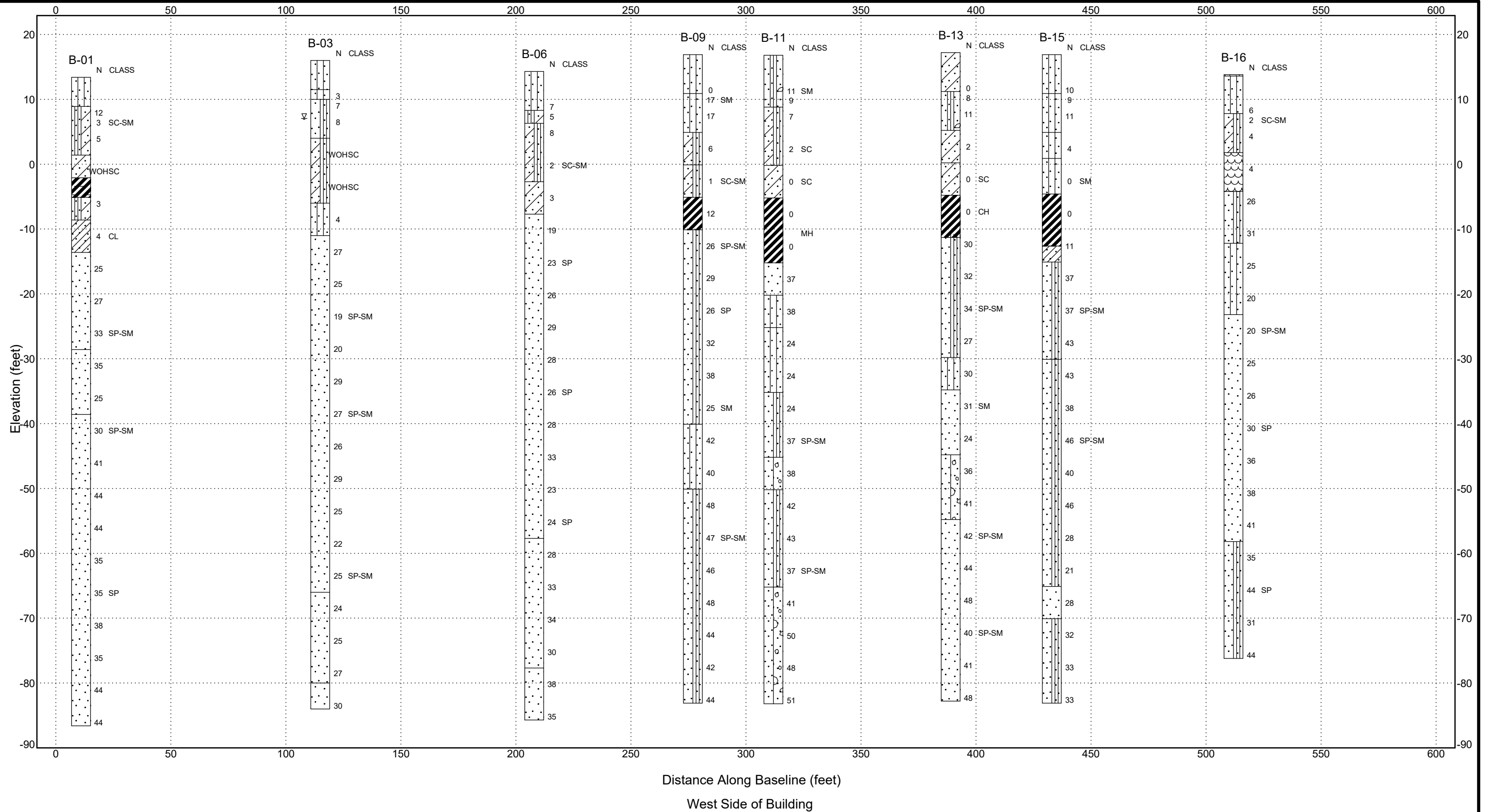
PROJECT NAME: CITY OF MOBILE
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PROJECT LOCATION: MOBILE, AL



APPENDIX D

SUBSURFACE DIAGRAMS

STRAT_DWG_#1 24-120 COM CIVIC CENTER.GPJ GETI_AL.GDT 7/5/24



LEGEND

	Topsoil		Sandy Clay		Sand with Clay
	Poorly-graded Sand		Lean Clay		Sand with Silt
	Silty Sand		Fat Clay		Water
	Clayey Sand		Silt		High Plasticity
					Organic silt or clay

N - SPT value determined in field ASTM D1586
%w - Percent Water
c - Cohesive Shear Strength, tsf
c* - Cohesive Shear Strength, tsf (Hand Penetrometer)
MV - Vane Shear Strength, tsf
Class - USCS Classification



SUBSURFACE DIAGRAM

PROJECT NAME: CITY OF MOBILE
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